STORMWATER MANAGEMENT CALCULATIONS

FOR

WHIPPLE DRIVE TOWNHOMES REZONING APPLICATION

MOUNT TABOR MAGISTERIAL DISTRICT TOWN OF BLACKSBURG, VIRGINIA

June 1, 2022



PREPARED BY:

BALZER AND ASSOCIATES, INC. 80 COLLEGE STREET, SUITE H CHRISTIANSBURG, VIRGINIA 24073 P-540-381-4290 F-540-381-4291

TABLE OF CONTENTS

SECTION I: PROJECT NARRATIVE	3
SECTION II: STORMWATER MANAGEMENT SUMMARY	5
PRE-DEVELOPMENT SUMMARY	5
POST-DEVELOPMENT SUMMARYError!	Bookmark not defined.
SECTION III: STORMWATER QUALITY SUMMARY	11
SECTION IV: DOWNSTREAM ANALYSIS	12
SECTION V: STORMWATER MANAGEMENT MAINTENANCE/INSPECT	<u>ION PLAN</u> 13
APPENDIX A: SOILS MAPS & SOIL DESCRIPTIONS	
APPENDIX R. DRAINAGE MAPS	

APPENDIX D: STORMWATER QUALITY CALCULATIONS

SECTION I: PROJECT NARRATIVE

Project Description

The purpose of this project is the rezoning of 3.08 acres of land for JJB Properties, LLC. The project area is currently made up of five (5) separate parcels. Four (4) of the parcels have existing houses located on them and one parcel is vacant. The applicant proposes to rezone the properties from R-5 (Transitional Residential) to a PRD (Planned Residential District) in order to construct a new townhome community.

Existing Site Conditions

The project site¹ is situated near the southeast corner of the intersection of Whipple Drive and Givens Lane. The site is bound by Ratcliff Corner Townhomes to the north, the property of Margaret Ann Evans to the east and south, and Whipple Drive to the west. Surrounding properties consist of multi-family residential, townhomes, single-family residential, commercial properties, and vacant land.

Existing soil conditions on site include the types listed below with slopes ranging from 2%-15%. There are currently no known environmental issues on site, however, prior to construction, the site will be fully investigated to determine if there are any jurisdictional waters on the property or within any of the areas of disturbance. If evidence is found, the property will be delineated, confirmed by the US Army Corps of Engineers, and all necessary permits will be filed.

Existing soil conditions on-site include the following types:

(See attached soils map for specific locations.)

11B - Duffield-Ernest Complex, 2 to 7 percent slopes

K-Factor: 0.28 Texture: Silt Loam

HSG: B

13B - Frederick and Vertrees Gravelly Silt Loams, 2 to 7 percent slopes

K-Factor: 0.24

Texture: Gravelly Silt Loam

HSG: B

13C - Frederick and Vertrees Gravelly Silt Loams, 7 to 15 percent slopes

K-Factor: 0.24

Texture: Gravelly Silt Loam

HSG: B

¹ For the purposes of the Project Narrative, "site" shall be defined as the area within the subject property boundary, 3.08 acres, Tax Map #'s 197-1 50A, 51C, 5*; 197-1 51B, 52B, 5*; 197-1 51A, 52A, 5*; 167-24 53A, and 197-1 46A.

Development Plans

The proposed development will consist of 34 new townhomes with 2- or 3-bedrooms each. The townhomes will each be two (2) stories and built over a crawl space. Each unit will include a covered front porch and either an elevated deck or patio on grade in the rear. Parking will be provided in a surface parking lot built in a loop around the development. Water main and sanitary sewer main extensions are proposed to serve the development. Stormwater quantity management will be handled by an underground detention system. Water quality requirements will be met by purchasing nutrient credits.

During Construction

Neighboring areas are primarily developed urban land consisting of single- and multi-family residential and commercial properties. Any runoff from the site shall be controlled with temporary measures such as a construction entrance, silt fence, inlet protection, construction road stabilization, seeding and other measures per Virginia Erosion and Sediment Control Handbook standards.

SECTION II: STORMWATER MANAGEMENT SUMMARY

PRE-DEVELOPMENT SUMMARY

Please see Sheet SW3 for drainage area map.

In the pre-development condition, the site contains several single-family homes as well as some grassed areas and a few trees. The majority of the site drains naturally to a low spot in the northwestern corner of the site, where runoff from this site converges with runoff from the properties to the north as well as the outflow from the underground detention system located at Ratcliff Townhomes. All of this flow is conveyed through an existing storm pipe to a curb inlet located in Whipple Drive. A portion of the property (approximately 0.94 acres) drains directly into the right of way, where it is conveyed to the same curb inlet. From this point, runoff is carried across Whipple Drive and continues through a series of pipes and conveyance channels. The point of analysis has been set at this curb inlet where the majority runoff from the site converges before crossing under Whipple Drive.

A small area of the site is shown as a Direct Runoff Area. Water from this portion of the site flows towards the southwest corner of site and into a different curb inlet, where it is conveyed through a pipe system and eventually combines with the remainder of the site runoff.

See the following pages and the enclosed HydroCAD report for the peak flow rates and runoff volumes in the pre-development condition. All flows in the HydroCAD model have been analyzed using the SCS/TR-55, weighted Q method. See the included drainage map and HydroCAD report for time of concentration calculations. Where a subwatershed is predominantly impervious, a minimum time of concentration of 6 minutes has been assumed.

Pre-Development Land Cover

Area (acres)	CN	Description (subcatchment numbers)
0.115	70	1/2 acre lots, 25% imp, HSG B (2S)
4.665	61	>75% Grass cover, Good, HSG B (1S, 2S, 3S)
0.881	98	Paved parking, HSG B (1S, 2S, 3S)
5.661	67	TOTAL AREA

Point of Analysis

Total Drainage Area= 5.09 acres

	Peak Flow	Runoff Volume
1-year	0.54 cfs	0.124 af
2-year	1.37 cfs	0.211 af
10-year	4.78 cfs	0.526 af
100-year	14.11 cfs	1.249 af

Direct Runoff Area

Total Drainage Area= 0.570 acres

Run	off	Vol	lume
Null	vII	VU	ullic

1-year 0.013 af2-year 0.022 af10-year 0.057 af100-year 0.138 af

POST-DEVELOPMENT SUMMARY

Please see Sheet SW4 for drainage area map.

In the post-development condition, the proposed site will be graded to capture runoff via sheet flow, roof drains, curb inlets, and stormwater piping. Runoff will be collected in an underground detention basin. Outflow from this system will be managed by multiple flow control devices. As shown in the enclosed HydroCAD calculations, the underground system has been designed to manage peak flows and meet water quantity requirements. The system outlet structure will be fitted with a DI-7 top to allow water from the 100-year storm event to escape.

Outflow from the system will converge with the flow from Ratcliff Corner and be conveyed to the point of analysis, the curb inlet in Whipple Drive. The following pages and the HydroCAD report demonstrate that the site will be contributing less flow to the inlet in Whipple Drive than in the pre-development condition.

A small portion of the site will remain as direct runoff and continue to flow to the southeast corner. As in the pre-development condition, this flow will travel through the storm pipe system in Whipple Drive and combine with the flow from the new stormwater system before traveling downstream.

See the following pages and the enclosed HydroCAD report for the peak flow rates and runoff volumes in the post-development condition. All flows in this model have been analyzed using the SCS/TR-55, weighted Q method. See the included drainage map and HydroCAD report for time of concentration calculations. Where a subwatershed is predominantly impervious, a minimum time of concentration of 6 minutes has been assumed.

Post-Development Land Cover

Area (acres)	CN	Description
0.115	70	1/2 acre lots, 25% imp, HSG B (2S)
3.200	61	>75% Grass cover, HSG C (1S, 2S, 3S)
2.345	98	Paved parking, HSG C (1S-A, 1S-B)
5.660	77	TOTAL AREA

Point of Analysis

Total Drainage Area = 5.306 acres

The following table summarizes the pre- and post-development peak flow rates for the point of analysis and the percent change for each storm (see HydroCAD report).

	Pre-Dev Peak Flow Rate	Post-Dev Peak Flow Rate	Reduction
1-year	0.54 cfs	0.26 cfs	-51.8%
2-year	1.37 cfs	0.95 cfs	-30.7%
10-year	4.78 cfs	4.08 cfs	-14.6%

As shown above, the post-development peak flow rates are less than the pre-development peak flow rates for the 1-year, 2-year, and 10-year, thus meeting Town of Blacksburg requirements for stormwater quantity.

Direct Runoff Area

Total Drainage Area = 0.203 acres

The following table summarizes the pre- and post-development runoff volumes for Direct Runoff Area #1 and the percent change for each storm (See HydroCAD report).

	Pre-Dev Runoff Volume	Post-Dev Runoff Volume	Reduction
1-year	0.013 af	0.008 af	-38.5%
2-year	0.022 af	0.012 af	-45.5%
10-year	0.057 af	0.027 af	-52.6%
100-year	0.138 af	0.059 af	-57.2%

As shown above, the post-development runoff volumes for the direct runoff areas are less than the pre-development runoff volumes for the 1-year, 2-year, and 10-year, thus meeting the requirements of 9VAC25-870-66 subsection D.

Channel Protection

In accordance with 9VAC25-870-66 (B), concentrated stormwater flows will be discharged directly to a stormwater conveyance system. Runoff from this system will be discharged through a pipe into a channel. From this point, runoff will flow through a series of manmade and natural conveyance systems to the 1% analysis point of the site² (323 acres). No erosion of either the natural or the manmade system should be expected from stormwater flows. Per subdivision (3)(a), the maximum post-development peak flow rate from the 1-year 24-hour storm shall be calculated per the equations below to prevent erosion of the natural conveyance systems. Additionally, all analyzed storms will produce a post-development flow rate lower than the pre-development flow rate, therefore no erosion of the manmade system should be expected.

R_v Calculation

Pre-developed = 0.048 acre*ft – See HydroCAD "RV Calculation" Report Developed = 0.209 acre*ft – See HydroCAD "RV Calculation" Report

$$\begin{split} Q_{Developed} &\leq I.F. \times \left(Q_{Pr\,e-developed} \times RV_{Pr\,e-Developed}\right) / RV_{Developed} \\ Q_{Developed} &\leq 0.8 \times \left(Q_{Pr\,e-developed} \times 0.048\right) / 0.209 \\ Q_{Developed} &\leq 0.18 \times Q_{Pr\,e-developed} \end{split}$$

The resulting maximum allowable peak flow rate for the one-year 24-hour storm at the Point of Analysis is 0.29 cfs. The actual post-development peak flow rate achieved is 0.26 cfs.

Flood Protection

In accordance with 9VAC25-870-66 (C), concentrated stormwater flows have been discharged to a stormwater conveyance system. The downstream conveyance systems are made up of a series of natural and manmade conveyance systems. As shown on the attached HydroCAD calculations, the point of discharge releases a post-development peak flow rate for the 10-year 24-hour storm event that is less than the pre-development peak flow rate from the 10-year 24-hour storm event, satisfying subdivision 2(b). Per subdivision (3) of these regulations, no further analysis of the downstream stormwater conveyance system is required.

² In the context of channel and flood protection, "site" shall be defined as the area where work is being performed, including any offsite disturbance (approximately 3.23 acres). See Sheets SW3-SW4.

SECTION III: STORMWATER QUALITY SUMMARY

Water quality compliance will be achieved through the purchase of nutrient credits in accordance with the criteria set forth in the Code of Virginia. Per §62.1-44.15:35(C)(2), the VSMP shall allow the use of nutrient credits when the area of disturbance is less than 5 acres or the water quality reduction requirement is less than 10 pounds per year. This site qualifies for nutrient credit purchase with a total disturbed area of approximately 3.23 acres and a reduction requirement of 2.85 pounds per year.

The existing site³ has an impervious land cover of 0.36 acres (11%). The post-development site will have an impervious land cover of 1.82 acres (56%) resulting in a runoff coefficient (R_v) of 0.62. The required pollutant removal rate is 2.85 lb/year, all of which will be handled with nutrient credits.

³ In the context of channel and flood protection, "site" shall be defined as the area where work is being performed, including any offsite disturbance (approximately 3.23 acres). See Sheets SW3-SW4.

SECTION IV: DOWNSTREAM ANALYSIS

Runoff from the proposed development is discharged directly into to a series of natural and manmade conveyance systems. These conveyance systems carry flows from the site downstream to the 1% analysis point (323 acres). The post-development peak runoff has been mitigated via underground detention facilities to prevent adverse impacts from this site to downstream properties in the form of channel erosion and flooding.

Per 9VAC25-870-66 subsection A, compliance with Minimum Standard 19 of the Virginia Erosion and Sediment Control Regulations has been satisfied by meeting the requirements of the for channel protection and flood protection as shown in the Post Development Summary. No adverse impacts to downstream properties are expected as a result of this development.

SECTION V: STORMWATER MANAGEMENT MAINTENANCE/INSPECTION PLAN

Generally

- 1. The owner is responsible for providing or coordinating all facility inspections and any required maintenance that may result from such inspections.
- 2. Requirements listed here are to be taken as a minimum and do not represent the limit of responsibility.
- 3. Any standing water pumped during the maintenance operation must be disposed of per the VESCH, 1992 edition and any local requirements.

Required Action

Underground Detention Facilities:

- 1. Every (12) months the responsible party shall complete and document a visual inspection of the underground facility and its components and make any repairs necessary to areas of failure or concern discovered during inspection. Typical maintenance tasks include:
 - a. Cleanout of any debris or sediment accumulated in the structure that reduces the storage volume or otherwise hinders the performance of the facility.
 - b. Visual inspection for structural deterioration, spalling, or cracking of the structural components.
- 2. The flow control manholes shall be inspected after each runoff producing storm event to check for debris and/or sediment accumulation that may compromise the performance of the structure. Such debris and sediments shall be removed immediately.

Per the Town of Blacksburg stormwater ordinance, a formal maintenance agreement shall be provided to the Town for review and ultimately recorded at the Montgomery County Courthouse legally binding the identified party to the maintenance/inspection responsibilities listed above.

APPENDIX A: SOIL MAPS & SOIL DESCRIPTIONS



NRCS

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Montgomery County, Virginia



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
How Soil Surveys Are Made	
Soil Map	
Soil Map	
Legend	
Map Unit Legend	
Map Unit Descriptions	
Montgomery County, Virginia	
11B—Duffield-Ernest complex, 2 to 7 percent slopes	
13B—Frederick and Vertrees gravelly silt loams, 2 to 7 percent slopes	. 14
13C—Frederick and Vertrees gravelly silt loams, 7 to 15 percent slopes	16
Soil Information for All Uses	19
Soil Properties and Qualities	. 19
Soil Erosion Factors	19
K Factor, Whole Soil	19
Soil Physical Properties	. 22
Surface Texture	
Soil Qualities and Features	25
Hydrologic Soil Group	. 25
References	30

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons

Soil Map Unit Lines

Soil Map Unit Points

Special Point Features

(o)

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water Rock Outcrop

Saline Spot

Sandy Spot

Severely Eroded Spot

Sinkhole

Sodic Spot

Slide or Slip



Spoil Area Stony Spot



Very Stony Spot



Wet Spot Other



Special Line Features

Water Features

Streams and Canals

Transportation

Rails

Interstate Highways

US Routes

00

Major Roads Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15.800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Montgomery County, Virginia Survey Area Data: Version 14, Sep 14, 2021

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 29, 2019—Oct 4, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
11B	Duffield-Ernest complex, 2 to 7 percent slopes	1.4	37.0%
13B	Frederick and Vertrees gravelly silt loams, 2 to 7 percent slopes	0.3	8.3%
13C	Frederick and Vertrees gravelly silt loams, 7 to 15 percent slopes	2.0	54.7%
Totals for Area of Interest		3.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate

pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Montgomery County, Virginia

11B—Duffield-Ernest complex, 2 to 7 percent slopes

Map Unit Setting

National map unit symbol: kc1q Elevation: 1,300 to 3,000 feet

Mean annual precipitation: 30 to 45 inches Mean annual air temperature: 50 to 57 degrees F

Frost-free period: 117 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Duffield and similar soils: 45 percent Ernest and similar soils: 35 percent Minor components: 3 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Duffield

Setting

Landform: Drainageways

Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope

Down-slope shape: Concave Across-slope shape: Convex

Parent material: Loamy colluvial, alluvial, eolian sediments underlain by loamy and

clayey residuum of limestone and shale

Typical profile

H1 - 0 to 7 inches: silt loam
H2 - 7 to 37 inches: silty clay loam

H3 - 37 to 79 inches: clay

Properties and qualities

Slope: 2 to 7 percent

Depth to restrictive feature: 48 to 99 inches to lithic bedrock

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: High (about 10.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B Hydric soil rating: No

Description of Ernest

Setting

Landform: Drainageways

Landform position (two-dimensional): Footslope Landform position (three-dimensional): Base slope

Down-slope shape: Concave Across-slope shape: Convex

Parent material: Interbedded limestone and shale residuum

Typical profile

H1 - 0 to 6 inches: silt loam
H2 - 6 to 26 inches: silty clay loam
H3 - 26 to 50 inches: silty clay loam
H4 - 50 to 79 inches: silty clay loam

Properties and qualities

Slope: 2 to 7 percent

Depth to restrictive feature: 20 to 35 inches to fragipan

Drainage class: Moderately well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.57 in/hr)

Depth to water table: About 18 to 36 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 3.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C Hydric soil rating: No

Minor Components

Purdv

Percent of map unit: 3 percent

Landform: Stream terraces, depressions
Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: Yes

13B—Frederick and Vertrees gravelly silt loams, 2 to 7 percent slopes

Map Unit Setting

National map unit symbol: kc1v Elevation: 1,700 to 3,000 feet

Mean annual precipitation: 30 to 45 inches Mean annual air temperature: 50 to 57 degrees F

Frost-free period: 117 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Frederick and similar soils: 40 percent Vertrees and similar soils: 35 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Frederick

Setting

Landform: Hills

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Limestone interbedded with siltstone and shale residuum

Typical profile

H1 - 0 to 10 inches: gravelly silt loam

H2 - 10 to 22 inches: clay H3 - 22 to 79 inches: clay

Properties and qualities

Slope: 2 to 7 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: B

Forage suitability group: Moist, Fertile Soils (G128XB001VA)
Other vegetative classification: Moist, Fertile Soils (G128XB001VA)

Hydric soil rating: No

Description of Vertrees

Settina

Landform: Hills

Landform position (two-dimensional): Summit Landform position (three-dimensional): Interfluve

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Limestone, siltstone, and shale residuum

Typical profile

H1 - 0 to 10 inches: gravelly silt loam

H2 - 10 to 25 inches: silty clay H3 - 25 to 50 inches: clay H4 - 50 to 79 inches: clay

Properties and qualities

Slope: 2 to 7 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.57 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 8.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C

Forage suitability group: Moist, Fertile Soils (G128XB001VA)
Other vegetative classification: Moist, Fertile Soils (G128XB001VA)

Hydric soil rating: No

13C—Frederick and Vertrees gravelly silt loams, 7 to 15 percent slopes

Map Unit Setting

National map unit symbol: kc1w Elevation: 1,700 to 3,000 feet

Mean annual precipitation: 30 to 45 inches
Mean annual air temperature: 50 to 57 degrees F

Frost-free period: 117 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Frederick and similar soils: 40 percent Vertrees and similar soils: 35 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Frederick

Setting

Landform: Hills

Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Limestone interbedded with siltstone and shale residuum

Typical profile

H1 - 0 to 10 inches: gravelly silt loam

H2 - 10 to 22 inches: clay H3 - 22 to 79 inches: clay

Properties and qualities

Slope: 7 to 15 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.57 to 1.98 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 8.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Forage suitability group: Moist, Fertile Soils (G128XB001VA)
Other vegetative classification: Moist, Fertile Soils (G128XB001VA)

Hydric soil rating: No

Description of Vertrees

Setting

Landform: Hills

Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Interfluve, side slope

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Limestone, siltstone, and shale residuum

Typical profile

H1 - 0 to 10 inches: gravelly silt loam H2 - 10 to 25 inches: silty clay H3 - 25 to 50 inches: clay H4 - 50 to 79 inches: clay

Properties and qualities

Slope: 7 to 15 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.57 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 8.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: C

Forage suitability group: Moist, Fertile Soils (G128XB001VA)
Other vegetative classification: Moist, Fertile Soils (G128XB001VA)

Hydric soil rating: No

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Erosion Factors

Soil Erosion Factors are soil properties and interpretations used in evaluating the soil for potential erosion. Example soil erosion factors can include K factor for the whole soil or on a rock free basis, T factor, wind erodibility group and wind erodibility index.

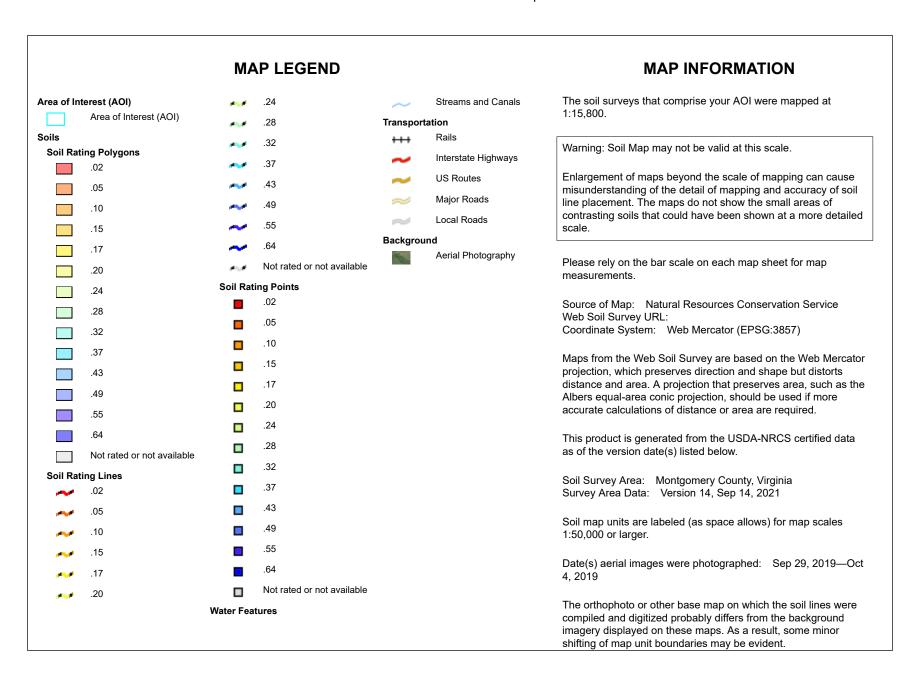
K Factor, Whole Soil

Erosion factor K indicates the susceptibility of a soil to sheet and rill erosion by water. Factor K is one of six factors used in the Universal Soil Loss Equation (USLE) and the Revised Universal Soil Loss Equation (RUSLE) to predict the average annual rate of soil loss by sheet and rill erosion in tons per acre per year. The estimates are based primarily on percentage of silt, sand, and organic matter and on soil structure and saturated hydraulic conductivity (Ksat). Values of K range from 0.02 to 0.69. Other factors being equal, the higher the value, the more susceptible the soil is to sheet and rill erosion by water.

"Erosion factor Kw (whole soil)" indicates the erodibility of the whole soil. The estimates are modified by the presence of rock fragments.

Factor K does not apply to organic horizons and is not reported for those layers.





Table—K Factor, Whole Soil

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
11B	Duffield-Ernest complex, 2 to 7 percent slopes	.28	1.4	37.0%
13B	Frederick and Vertrees gravelly silt loams, 2 to 7 percent slopes	.24	0.3	8.3%
13C	Frederick and Vertrees gravelly silt loams, 7 to 15 percent slopes	.24	2.0	54.7%
Totals for Area of Intere	st	1	3.7	100.0%

Rating Options—K Factor, Whole Soil

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Layer Options (Horizon Aggregation Method): Surface Layer (Not applicable)

Soil Physical Properties

Soil Physical Properties are measured or inferred from direct observations in the field or laboratory. Examples of soil physical properties include percent clay, organic matter, saturated hydraulic conductivity, available water capacity, and bulk density.

Surface Texture

This displays the representative texture class and modifier of the surface horizon.

Texture is given in the standard terms used by the U.S. Department of Agriculture. These terms are defined according to percentages of sand, silt, and clay in the fraction of the soil that is less than 2 millimeters in diameter. "Loam," for example, is soil that is 7 to 27 percent clay, 28 to 50 percent silt, and less than 52 percent sand. If the content of particles coarser than sand is 15 percent or more, an appropriate modifier is added, for example, "gravelly."



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

Gravelly silt loam

Silt loam

Not rated or not available

Soil Rating Lines

Gravelly silt loam

Silt loam

Not rated or not available

Soil Rating Points

Gravelly silt loam

Silt loam

Not rated or not available

Water Features

Streams and Canals

Transportation

Rails Interstate Highways

US Routes

Major Roads

Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15.800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service

Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Montgomery County, Virginia Survey Area Data: Version 14, Sep 14, 2021

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Sep 29, 2019—Oct 4, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Surface Texture

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
11B	Duffield-Ernest complex, 2 to 7 percent slopes	Silt loam	1.4	37.0%
13B	Frederick and Vertrees gravelly silt loams, 2 to 7 percent slopes	Gravelly silt loam	0.3	8.3%
13C Frederick and Vertrees gravelly silt loams, 7 to 15 percent slopes		Gravelly silt loam	2.0	54.7%
Totals for Area of Intere	est	3.7	100.0%	

Rating Options—Surface Texture

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Lower

Layer Options (Horizon Aggregation Method): Surface Layer (Not applicable)

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

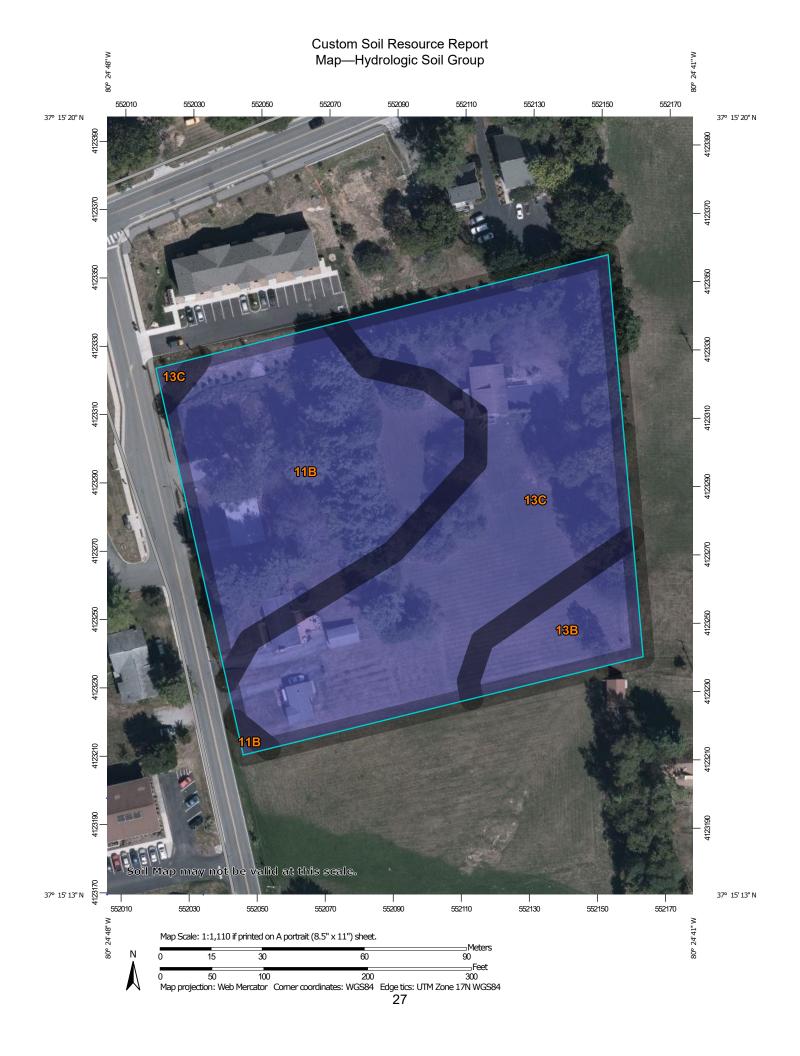
Custom Soil Resource Report

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



MAP LEGEND MAP INFORMATION Area of Interest (AOI) The soil surveys that comprise your AOI were mapped at С 1:15.800. Area of Interest (AOI) C/D Soils D Warning: Soil Map may not be valid at this scale. Soil Rating Polygons Not rated or not available Α Enlargement of maps beyond the scale of mapping can cause **Water Features** A/D misunderstanding of the detail of mapping and accuracy of soil Streams and Canals line placement. The maps do not show the small areas of В contrasting soils that could have been shown at a more detailed Transportation scale. B/D Rails ---Interstate Highways Please rely on the bar scale on each map sheet for map C/D **US Routes** measurements. Major Roads Source of Map: Natural Resources Conservation Service Not rated or not available Local Roads Web Soil Survey URL: -Coordinate System: Web Mercator (EPSG:3857) Soil Rating Lines Background Aerial Photography Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: Montgomery County, Virginia Not rated or not available Survey Area Data: Version 14, Sep 14, 2021 Soil Rating Points Soil map units are labeled (as space allows) for map scales Α 1:50.000 or larger. A/D Date(s) aerial images were photographed: Sep 29, 2019—Oct 4, 2019 B/D The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
11B	Duffield-Ernest complex, 2 to 7 percent slopes	В	1.4	37.0%
13B	Frederick and Vertrees gravelly silt loams, 2 to 7 percent slopes	В	0.3	8.3%
13C	Frederick and Vertrees gravelly silt loams, 7 to 15 percent slopes		2.0	54.7%
Totals for Area of Inter	est	3.7	100.0%	

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2 053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084

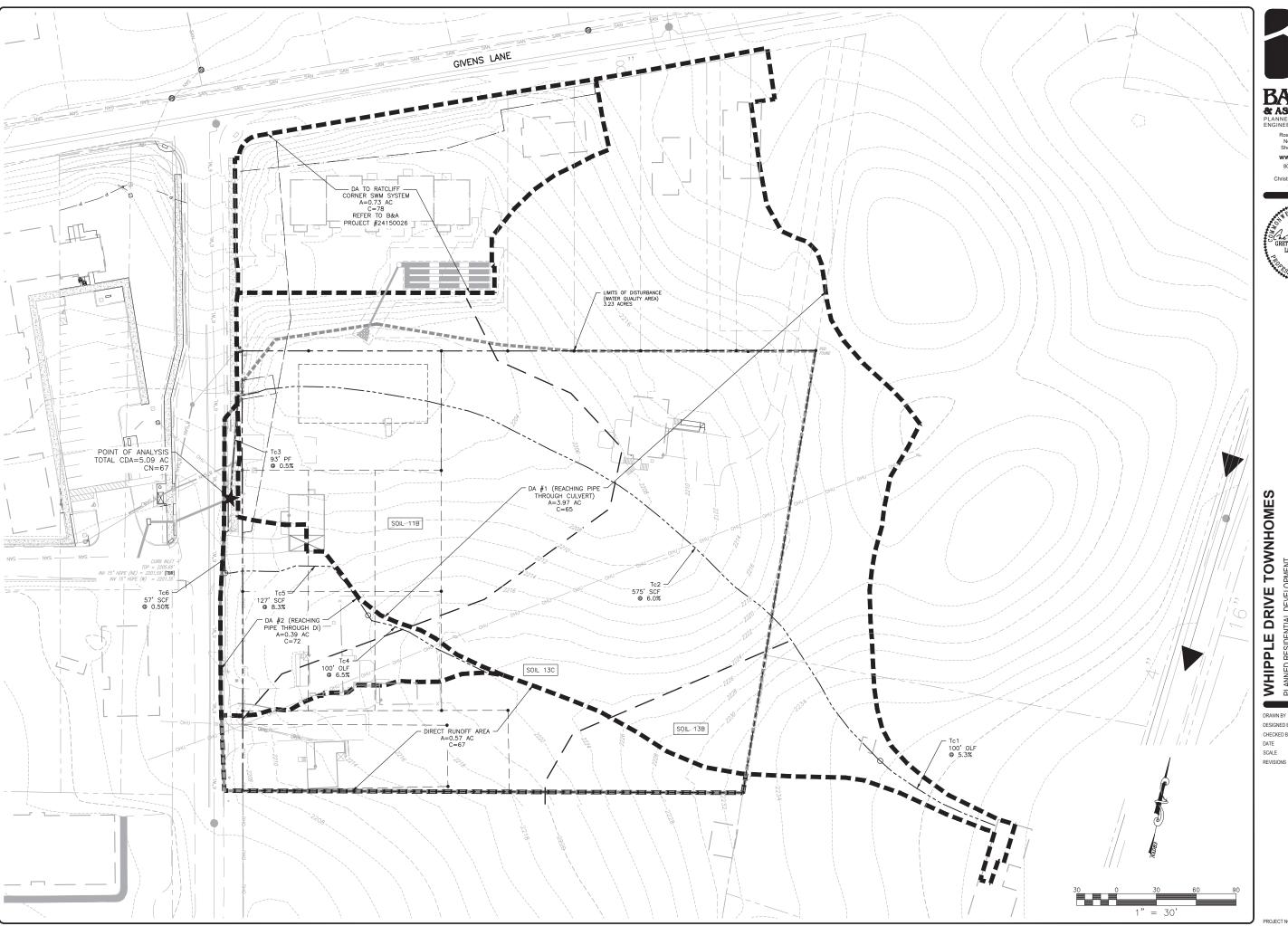
Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

APPENDIX B: DRAINAGE MAPS





Roanoke / Richmond New River Valley Shenandoah Valley

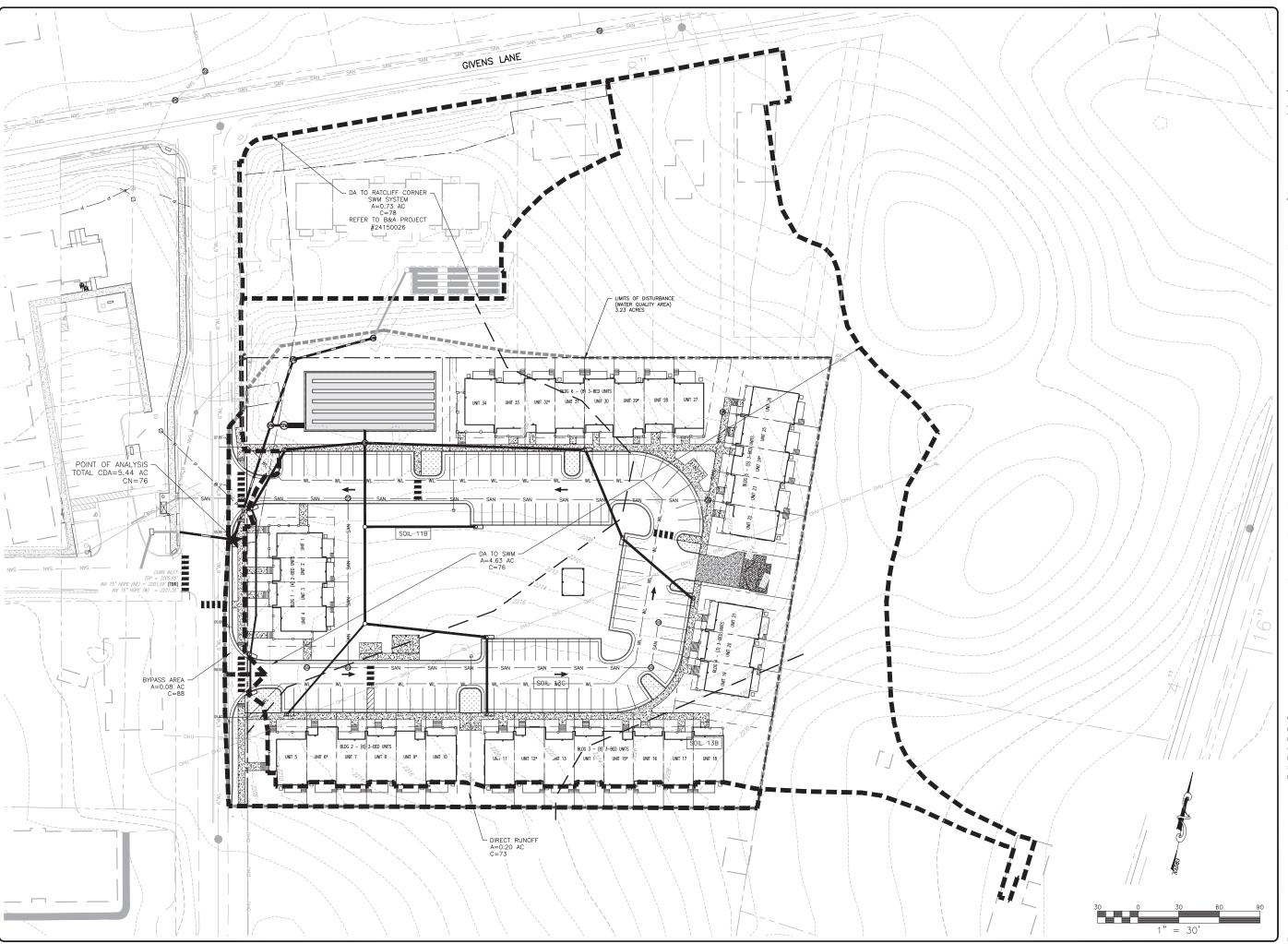
www.balzer.cc 80 College Street Suite H Christiansburg, VA 24073 540.381.4290



WHIPPLE DRIVE TOWNHOMES PLANNED RESIDENTIAL DEVELOPMENT PRE-DEVELOPMENT DRAINAGE AREA MAP

GLM GLM DESIGNED BY SMS CHECKED BY

6/1/22 1" = 30'





Roanoke / Richmond New River Valley Shenandoah Valley

www.balzer.cc 80 College Street Suite H Christiansburg, VA 24073 540.381.4290

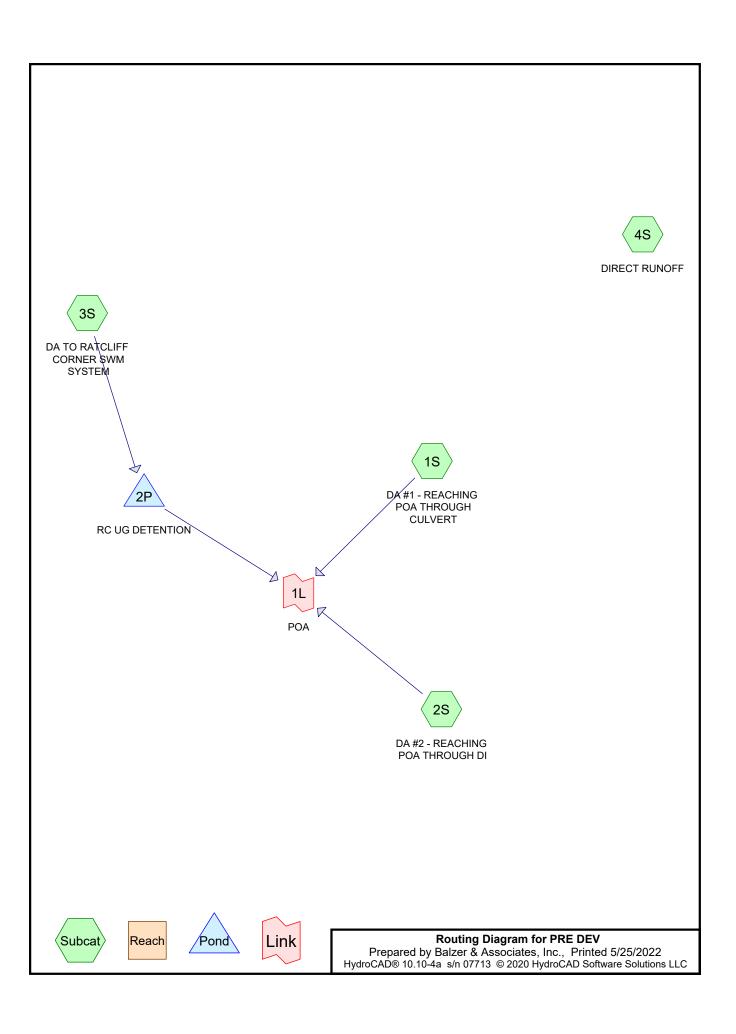


WHIPPLE DRIVE TOWNHOMES PLANNED RESIDENTIAL DEVELOPMENT POST-DEVELOPMENT DRAINAGE AREA MAP

DRAWN BY

GLM GLM DESIGNED BY SMS CHECKED BY DATE 6/1/22 1" = 30'

APPENDIX C: STORMWATER QUANTITY CALCULATIONS



PRE DEV

Prepared by Balzer & Associates, Inc.

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022 Page 2

Area Listing (all nodes)

Area	CN.	Description	
(acres))	(subcatchment-numbers)	
0.115	70	1/2 acre lots, 25% imp, HSG B (3S)	
4.665	61	>75% Grass cover, Good, HSG B (1S, 2S, 3S, 4S)	
0.881	98	Paved parking, HSG B (1S, 2S, 3S, 4S)	
5.661	67	TOTAL AREA	

PRE DEV

VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022

Page 3

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA #1 - REACHING POA Runoff Area=3.973 ac 9.46% Impervious Runoff Depth=0.22"

Flow Length=768' Tc=9.9 min CN=65 Runoff=0.40 cfs 0.072 af

Subcatchment 2S: DA #2 - REACHING POA Runoff Area=0.393 ac 30.53% Impervious Runoff Depth=0.41"

Flow Length=284' Tc=7.8 min CN=72 Runoff=0.15 cfs 0.014 af

Subcatchment3S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=0.64"

Tc=6.0 min CN=78 Runoff=0.60 cfs 0.039 af

Subcatchment 4S: DIRECT RUNOFF Runoff Area=0.570 ac 14.91% Impervious Runoff Depth=0.27"

Tc=0.0 min CN=67 Runoff=0.16 cfs 0.013 af

Pond 2P: RC UG DETENTION Peak Elev=2,205.39' Storage=0.018 af Inflow=0.60 cfs 0.039 af

Outflow=0.04 cfs 0.039 af

Link 1L: POA Inflow=0.54 cfs 0.124 af Primary=0.54 cfs 0.124 af

Total Runoff Area = 5.661 ac Runoff Volume = 0.137 af Average Runoff Depth = 0.29" 83.93% Pervious = 4.751 ac 16.07% Impervious = 0.910 ac

Page 4

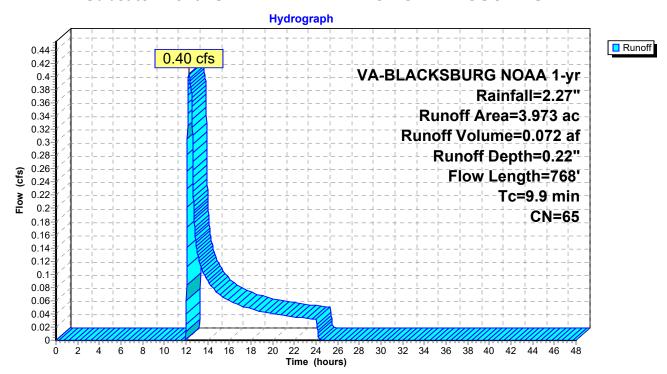
Summary for Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT

Runoff = 0.40 cfs @ 12.26 hrs, Volume= 0.072 af, Depth= 0.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area	(ac) C	N Des	cription		
3.597 61 >75% Grass cove			% Grass co	over, Good,	HSG B
0	.376	98 Pave	ed parking	, HSG B	
3.	.973	65 Wei	ghted Aver	age	
3.	.597	90.5	4% Pervio	us Area	
0.	.376	9.46	% Impervi	ous Area	
_				_	
Тс	Length	•	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.1	100	0.0530	0.23		Sheet Flow, Tc1
					Grass: Short n= 0.150 P2= 2.76"
2.4	575	0.0600	3.94		Shallow Concentrated Flow, Tc2
					Unpaved Kv= 16.1 fps
0.4	93	0.0053	3.83	4.70	Pipe Channel, Tc3
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Corrugated PE, smooth interior
9.9	768	Total			

Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT



Page 5

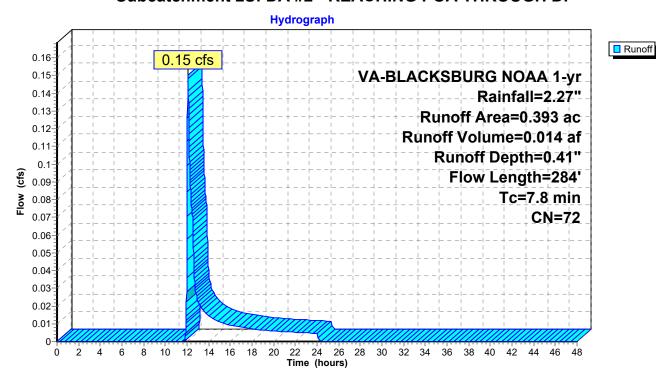
Summary for Subcatchment 2S: DA #2 - REACHING POA THROUGH DI

Runoff = 0.15 cfs @ 12.08 hrs, Volume= 0.014 af, Depth= 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

_	Area	(ac) C	N Desc	cription			
	_				over, Good	, HSG B	
_	0.	120 S	98 Pave	ed parking,	, HSG B		
	0.	393 7	⁷ 2 Weig	ghted Aver	age		
	0.	273	69.4	7% Pervio	us Area		
	0.	120	30.5	3% Imperv	∕ious Area		
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•	
	6.6	100	0.0650	0.25		Sheet Flow, Tc4	
						Grass: Short n= 0.150 P2= 2.76"	
	0.5	127	0.0830	4.64		Shallow Concentrated Flow, Tc5	
						Unpaved Kv= 16.1 fps	
	0.7	57	0.0050	1.44		Shallow Concentrated Flow, Tc6	
	J. .	•				Paved Kv= 20.3 fps	
_	7.8	284	Total				

Subcatchment 2S: DA #2 - REACHING POA THROUGH DI



Page 6

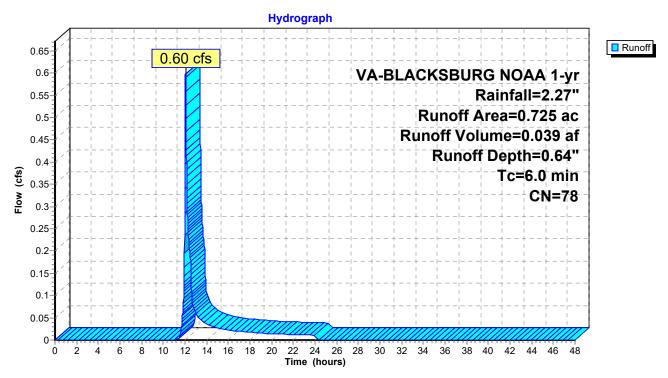
Summary for Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 0.60 cfs @ 12.05 hrs, Volume= 0.039 af, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

	Area	(ac)	CN	Desc	Description						
	0.	0.310 61 >75% Grass cover, Good, HSG B									
	0.	300	98	Pave	ed parking,	HSG B					
	0.	115	70	1/2 a	cre lots, 2	5% imp, H	SG B				
	0.725 78 Weighted Average										
	0.396 54.66% Pervious Area					us Area					
	0.329 45.34% Impervious Area					ious Area					
	Тс	Leng		Slope	Velocity	Capacity	Description				
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, TR-55				

Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM



Page 7

Summary for Subcatchment 4S: DIRECT RUNOFF

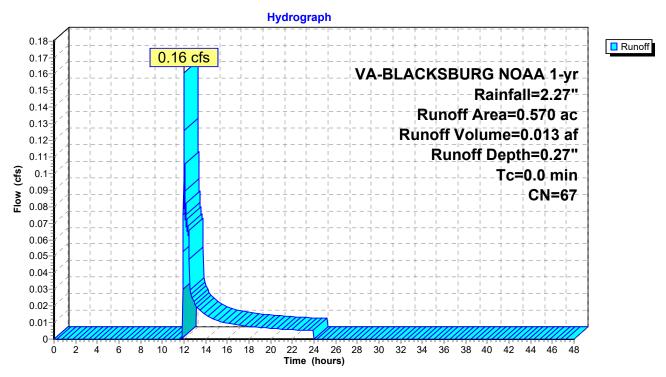
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.16 cfs @ 11.99 hrs, Volume= 0.013 af, Depth= 0.27"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area ((ac)	CN	Description					
0.4	485	61	>75% Grass cover, Good, HSG B					
0.0	085	98	Paved parking, HSG B					
0.:	570	67	Weighted Average					
0.4	485		85.09% Pervious Area					
0.0	085		14.91% Impervious Area					

Subcatchment 4S: DIRECT RUNOFF



PRE DEV

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 8

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 0.64" for 1-yr event

Inflow = 0.60 cfs @ 12.05 hrs, Volume= 0.039 af

Outflow = 0.04 cfs @ 13.96 hrs, Volume= 0.039 af, Atten= 94%, Lag= 114.9 min

Primary = 0.04 cfs @ 13.96 hrs, Volume= 0.039 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,205.39' @ 13.96 hrs Surf.Area= 0.032 ac Storage= 0.018 af

Plug-Flow detention time= 252.5 min calculated for 0.039 af (100% of inflow)

Center-of-Mass det. time= 252.5 min (1,125.6 - 873.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
		0.0E7.af	Total Available Ctarage

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.96'	15.0" Round 15" Culvert
	•	·	L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#2	Device 1	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.04 cfs @ 13.96 hrs HW=2,205.39' (Free Discharge)

1=15" Culvert (Passes 0.04 cfs of 4.43 cfs potential flow)

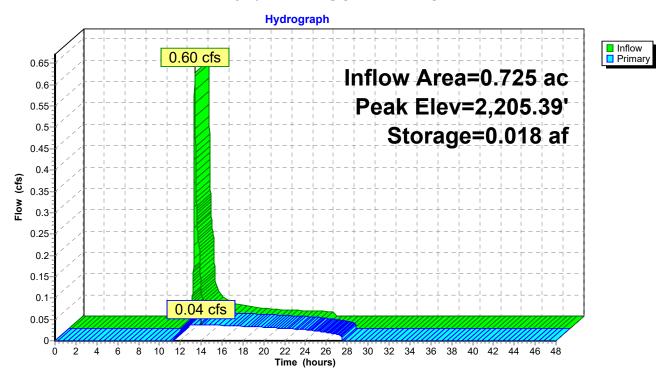
2=Orifice/Grate (Orifice Controls 0.04 cfs @ 5.57 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

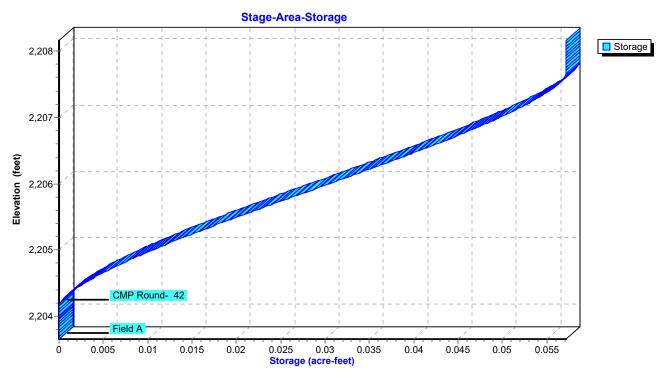
-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 9

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Page 10

Summary for Link 1L: POA

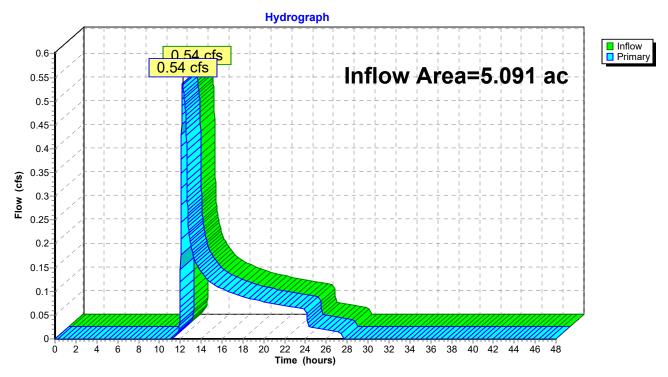
5.091 ac, 16.20% Impervious, Inflow Depth = 0.29" for 1-yr event Inflow Area =

0.54 cfs @ 12.22 hrs, Volume= Inflow 0.124 af

Primary 0.54 cfs @ 12.22 hrs, Volume= 0.124 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



PRE DEV

VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022

Page 11

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA #1 - REACHING POA Runoff Area=3.973 ac 9.46% Impervious Runoff Depth=0.40"

Flow Length=768' Tc=9.9 min CN=65 Runoff=1.10 cfs 0.131 af

Subcatchment 2S: DA #2 - REACHING POA Runoff Area=0.393 ac 30.53% Impervious Runoff Depth=0.66"

Flow Length=284' Tc=7.8 min CN=72 Runoff=0.28 cfs 0.022 af

Subcatchment3S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=0.95"

Tc=6.0 min CN=78 Runoff=0.94 cfs 0.058 af

Subcatchment 4S: DIRECT RUNOFF Runoff Area = 0.570 ac 14.91% Impervious Runoff Depth = 0.47"

Tc=0.0 min CN=67 Runoff=0.38 cfs 0.022 af

Pond 2P: RC UG DETENTION Peak Elev=2,205.97' Storage=0.030 af Inflow=0.94 cfs 0.058 af

Outflow=0.04 cfs 0.058 af

Link 1L: POA Inflow=1.37 cfs 0.211 af
Primary=1.37 cfs 0.211 af

Total Runoff Area = 5.661 ac Runoff Volume = 0.233 af Average Runoff Depth = 0.49" 83.93% Pervious = 4.751 ac 16.07% Impervious = 0.910 ac

Page 12

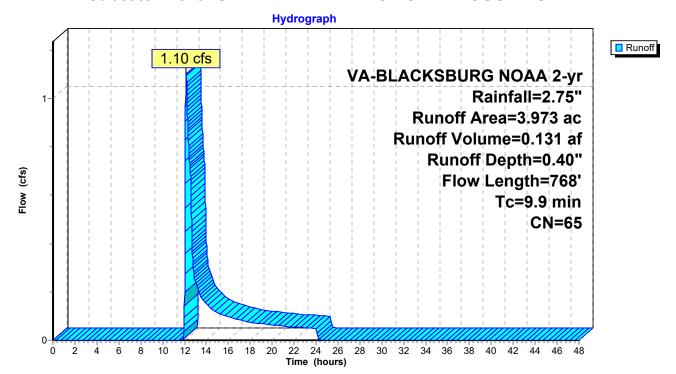
Summary for Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT

Runoff = 1.10 cfs @ 12.15 hrs, Volume= 0.131 af, Depth= 0.40"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

Area	(ac) C	N Des	cription		
3.597 61 >75% Grass cove			% Grass co	over, Good,	HSG B
0	.376	98 Pave	ed parking	, HSG B	
3.	.973	65 Wei	ghted Aver	age	
3.	.597	90.5	4% Pervio	us Area	
0.	.376	9.46	% Impervi	ous Area	
_				_	
Тс	Length	•	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.1	100	0.0530	0.23		Sheet Flow, Tc1
					Grass: Short n= 0.150 P2= 2.76"
2.4	575	0.0600	3.94		Shallow Concentrated Flow, Tc2
					Unpaved Kv= 16.1 fps
0.4	93	0.0053	3.83	4.70	Pipe Channel, Tc3
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Corrugated PE, smooth interior
9.9	768	Total			

Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT



Page 13

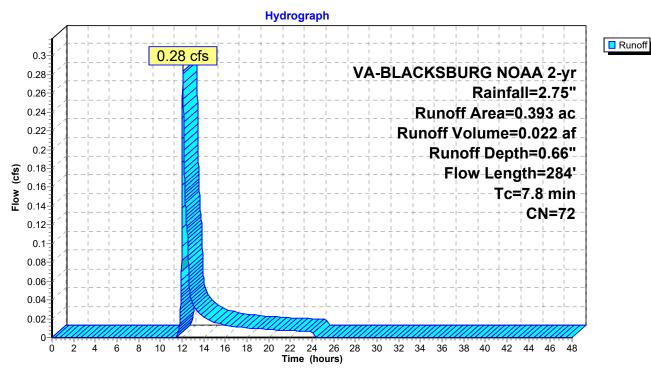
Summary for Subcatchment 2S: DA #2 - REACHING POA THROUGH DI

Runoff = 0.28 cfs @ 12.07 hrs, Volume= 0.022 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

_	Area	(ac) C	N Desc	cription		
	_				over, Good	, HSG B
_	0.	120 9	<u>8 Pave</u>	ed parking,	HSG B	
	0.	393 7	'2 Weig	ghted Aver	age	
	0.	273	69.4	7% Pervio	us Area	
	0.	120	30.5	3% Imperv	ious Area	
				-		
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	6.6	100	0.0650	0.25		Sheet Flow, Tc4
						Grass: Short n= 0.150 P2= 2.76"
	0.5	127	0.0830	4.64		Shallow Concentrated Flow, Tc5
						Unpaved Kv= 16.1 fps
	0.7	57	0.0050	1.44		Shallow Concentrated Flow, Tc6
						Paved Kv= 20.3 fps
_	7.8	284	Total			•

Subcatchment 2S: DA #2 - REACHING POA THROUGH DI



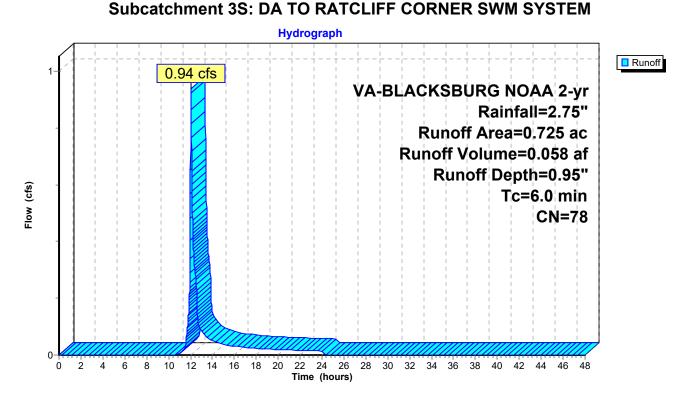
Page 14

Summary for Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 0.94 cfs @ 12.04 hrs, Volume= 0.058 af, Depth= 0.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

Area	(ac)	CN	Desc	Description						
0.310 61 >75% Grass cover, Good, HSG B										
0.	.300	98	Pave	ed parking,	HSG B					
0.	.115	70	1/2 a	cre lots, 2	5% imp, H	SG B				
0.	0.725 78 Weighted Average									
0.	.396		54.6	6% Pervio	us Area					
0.	.329		45.34	4% Imperv	ious Area					
_			21		0 :	D				
Tc	Lengt		Slope	Velocity	Capacity	Description				
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
6.0						Direct Entry, TR-55				



Summary for Subcatchment 4S: DIRECT RUNOFF

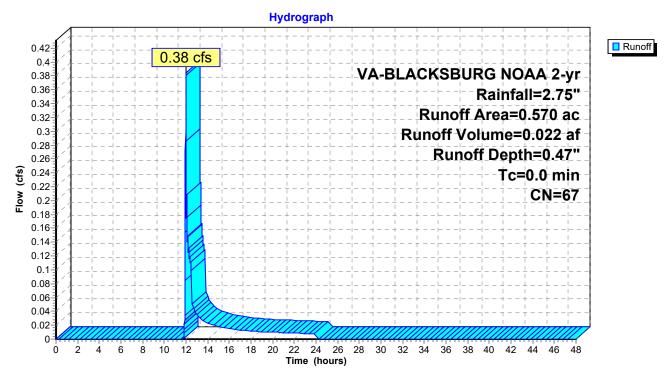
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.38 cfs @ 11.99 hrs, Volume= 0.022 af, Depth= 0.47"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

A	rea (ac)	CN	Description
	0.485	61	>75% Grass cover, Good, HSG B
	0.085	98	Paved parking, HSG B
	0.570	67	Weighted Average
	0.485		85.09% Pervious Area
	0.085		14.91% Impervious Area

Subcatchment 4S: DIRECT RUNOFF



Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 16

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 0.95" for 2-yr event

Inflow = 0.94 cfs @ 12.04 hrs, Volume= 0.058 af

Outflow = 0.04 cfs @ 14.42 hrs, Volume= 0.058 af, Atten= 95%, Lag= 142.4 min

Primary = 0.04 cfs @ 14.42 hrs, Volume= 0.058 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 2,205.97' @ 14.42 hrs Surf.Area= 0.032 ac Storage= 0.030 af

Plug-Flow detention time= 357.8 min calculated for 0.058 af (100% of inflow)

Center-of-Mass det. time= 357.8 min (1,217.8 - 859.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
		0.055 (T () A ()) O(

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.96'	15.0" Round 15" Culvert
	•	·	L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#2	Device 1	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.04 cfs @ 14.42 hrs HW=2,205.97' (Free Discharge)

-1=15" Culvert (Passes 0.04 cfs of 6.15 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.04 cfs @ 6.67 fps)

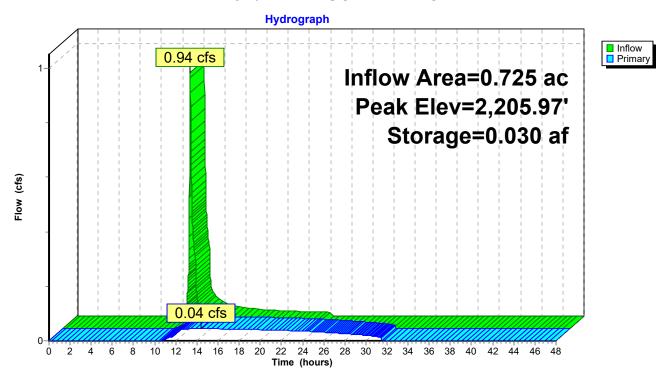
-3=Orifice/Grate (Controls 0.00 cfs)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

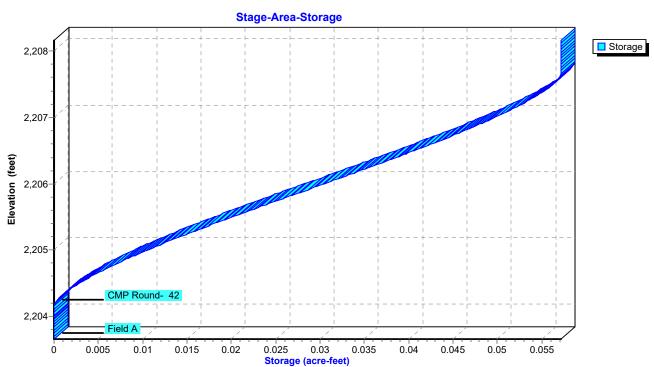
Printed 5/25/2022

Page 17

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Page 18

Summary for Link 1L: POA

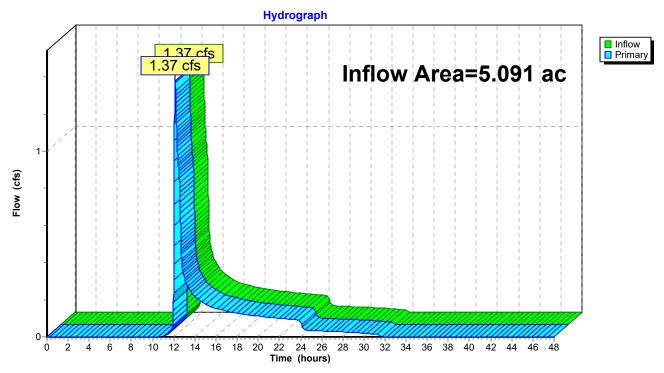
Inflow Area = 5.091 ac, 16.20% Impervious, Inflow Depth = 0.50" for 2-yr event

Inflow = 1.37 cfs @ 12.14 hrs, Volume= 0.211 af

Primary = 1.37 cfs @ 12.14 hrs, Volume= 0.211 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



PRE DEV

VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 19

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA #1 - REACHING POA Runoff Area=3.973 ac 9.46% Impervious Runoff Depth=1.08"

Flow Length=768' Tc=9.9 min CN=65 Runoff=4.10 cfs 0.358 af

Subcatchment 2S: DA #2 - REACHING POA Runoff Area=0.393 ac 30.53% Impervious Runoff Depth=1.52"

Flow Length=284' Tc=7.8 min CN=72 Runoff=0.69 cfs 0.050 af

Subcatchment3S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=1.96"

Tc=6.0 min CN=78 Runoff=1.86 cfs 0.118 af

Subcatchment 4S: DIRECT RUNOFF Runoff Area=0.570 ac 14.91% Impervious Runoff Depth=1.20"

Tc=0.0 min CN=67 Runoff=1.07 cfs 0.057 af

Pond 2P: RC UG DETENTION Peak Elev=2,207.01' Storage=0.050 af Inflow=1.86 cfs 0.118 af

Outflow=0.45 cfs 0.118 af

Link 1L: POA Inflow=4.78 cfs 0.526 af Primary=4.78 cfs 0.526 af

Total Runoff Area = 5.661 ac Runoff Volume = 0.583 af Average Runoff Depth = 1.24" 83.93% Pervious = 4.751 ac 16.07% Impervious = 0.910 ac

Page 20

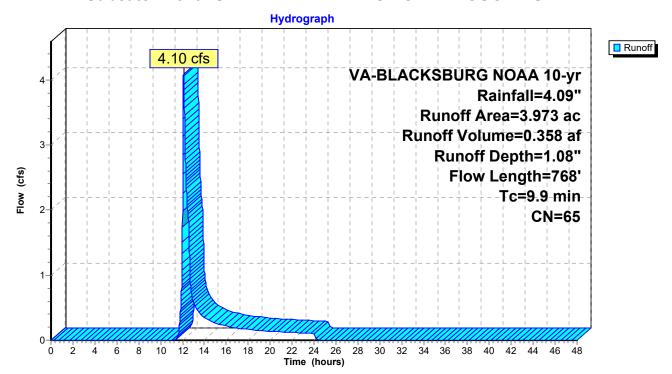
Summary for Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT

Runoff = 4.10 cfs @ 12.11 hrs, Volume= 0.358 af, Depth= 1.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Area	(ac) C	N Desc	cription		
3.597 6		31 >75°	% Grass co	over, Good,	HSG B
0.	.376	8 Pave	ed parking	, HSG B	
3.	.973 6	55 Weig	ghted Aver	age	
3.	.597	90.5	4% Pervio	us Area	
0.	.376	9.46	% Impervi	ous Area	
				_	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.1	100	0.0530	0.23		Sheet Flow, Tc1
					Grass: Short n= 0.150 P2= 2.76"
2.4	575	0.0600	3.94		Shallow Concentrated Flow, Tc2
					Unpaved Kv= 16.1 fps
0.4	93	0.0053	3.83	4.70	Pipe Channel, Tc3
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Corrugated PE, smooth interior
9.9	768	Total			

Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT



Page 21

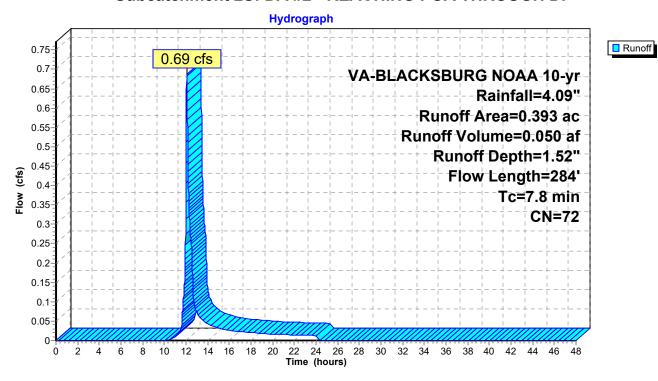
Summary for Subcatchment 2S: DA #2 - REACHING POA THROUGH DI

Runoff = 0.69 cfs @ 12.07 hrs, Volume= 0.050 af, Depth= 1.52"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

_	Area	(ac) C	N Desc	cription				
Ī	0.	273 6	31 >75°	>75% Grass cover, Good, HSG B				
	0.	120		ed parking.		,		
0.393 72 Weighted Average								
0.273 69.47% Pervious Area								
	0.	120	30.5	3% Imperv	ious Area			
				'				
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	6.6	100	0.0650	0.25		Sheet Flow, Tc4		
						Grass: Short n= 0.150 P2= 2.76"		
	0.5	127	0.0830	4.64		Shallow Concentrated Flow, Tc5		
						Unpaved Kv= 16.1 fps		
	0.7	57	0.0050	1.44		Shallow Concentrated Flow, Tc6		
_						Paved Kv= 20.3 fps		
	7.8	284	Total					

Subcatchment 2S: DA #2 - REACHING POA THROUGH DI



Page 22

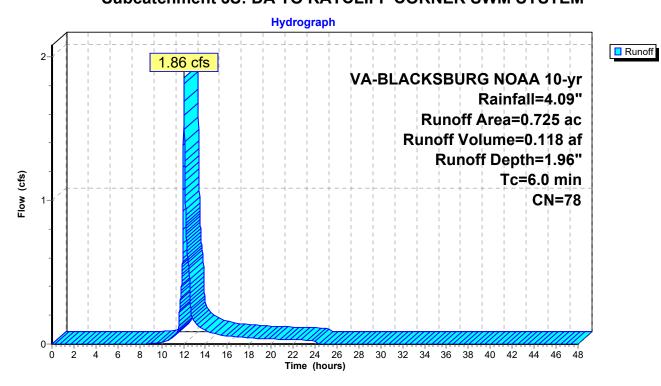
Summary for Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 1.86 cfs @ 12.04 hrs, Volume= 0.118 af, Depth= 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Area	(ac)	CN	Desc	Description						
0.	0.310 61 >75% Grass cover, Good, HSG B									
0.	.300	98	Pave	d parking,						
0.	.115	70	1/2 a	cre lots, 2	5% imp, H	SG B				
0.	725	78	Weig	hted Aver	age					
0.	0.396			54.66% Pervious Area						
0.329			45.34% Impervious Area							
т.	1	u. <i>1</i>	3 1	V/. I !4	0	D				
Tc	Lengt		Slope	Velocity	Capacity	Description				
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
6.0						Direct Entry, TR-55				

Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM



Printed 5/25/2022

Page 23

Summary for Subcatchment 4S: DIRECT RUNOFF

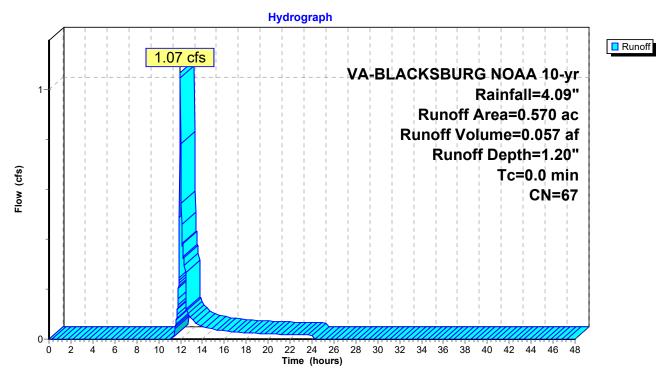
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 1.07 cfs @ 11.99 hrs, Volume= 0.057 af, Depth= 1.20"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Area ((ac)	CN	Description
0.4	485	61	>75% Grass cover, Good, HSG B
0.0	085	98	Paved parking, HSG B
0.:	570	67	Weighted Average
0.4	485		85.09% Pervious Area
0.0	085		14.91% Impervious Area

Subcatchment 4S: DIRECT RUNOFF



Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 24

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 1.96" for 10-yr event

Inflow = 1.86 cfs @ 12.04 hrs, Volume= 0.118 af

Outflow = 0.45 cfs @ 12.56 hrs, Volume= 0.118 af, Atten= 76%, Lag= 31.0 min

Primary = 0.45 cfs @ 12.56 hrs, Volume= 0.118 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,207.01' @ 12.56 hrs Surf.Area= 0.032 ac Storage= 0.050 af

Plug-Flow detention time= 325.9 min calculated for 0.118 af (100% of inflow)

Center-of-Mass det. time= 326.0 min (1,166.2 - 840.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
	·	0.055 (T () A ()) O (

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices			
#1	Primary	2,203.96'	15.0" Round 15" Culvert			
	•	·	L= 54.0' CPP, square edge headwall, Ke= 0.500			
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900			
			n= 0.011, Flow Area= 1.23 sf			
#2	Device 1	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
#3	Device 1	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads			
#4	Device 1	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir			
			Head (feet) 0.20 0.40 0.60 0.80 1.00			
			Coef. (English) 2.80 2.92 3.08 3.30 3.32			

Primary OutFlow Max=0.45 cfs @ 12.56 hrs HW=2,207.01' (Free Discharge)

-1=15" Culvert (Passes 0.45 cfs of 8.95 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.05 cfs @ 8.29 fps)

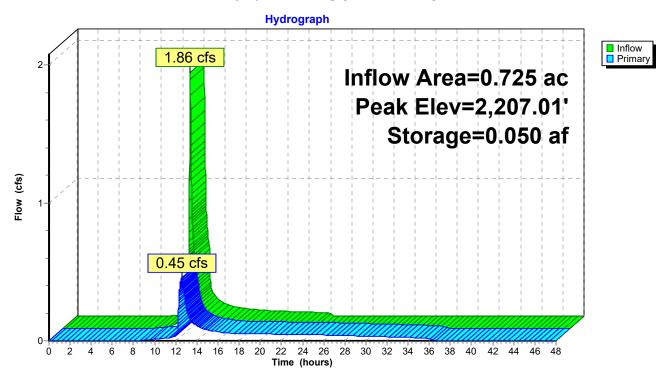
—3=Orifice/Grate (Orifice Controls 0.40 cfs @ 2.91 fps)

-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

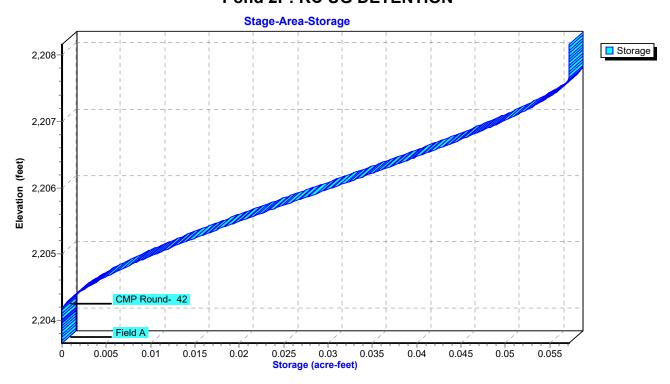
Page 25

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Summary for Link 1L: POA

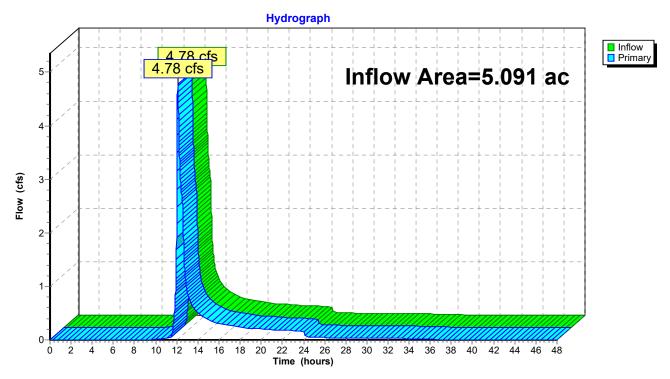
Inflow Area = 5.091 ac, 16.20% Impervious, Inflow Depth = 1.24" for 10-yr event

Inflow = 4.78 cfs @ 12.10 hrs, Volume= 0.526 af

Primary = 4.78 cfs @ 12.10 hrs, Volume= 0.526 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



PRE DEV

VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022

Page 27

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA #1 - REACHING POA Runoff Area=3.973 ac 9.46% Impervious Runoff Depth=2.71"

Flow Length=768' Tc=9.9 min CN=65 Runoff=9.95 cfs 0.896 af

Subcatchment 2S: DA #2 - REACHING POA Runoff Area=0.393 ac 30.53% Impervious Runoff Depth=3.39"

Flow Length=284' Tc=7.8 min CN=72 Runoff=1.37 cfs 0.111 af

Subcatchment3S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=4.01"

Tc=6.0 min CN=78 Runoff=3.25 cfs 0.242 af

Subcatchment 4S: DIRECT RUNOFF Runoff Area=0.570 ac 14.91% Impervious Runoff Depth=2.90"

Tc=0.0 min CN=67 Runoff=2.23 cfs 0.138 af

Pond 2P: RC UG DETENTION Peak Elev=2,207.47' Storage=0.056 af Inflow=3.25 cfs 0.242 af

Outflow=3.16 cfs 0.242 af

Link 1L: POA Inflow=14.11 cfs 1.249 af
Primary=14.11 cfs 1.249 af

Total Runoff Area = 5.661 ac Runoff Volume = 1.387 af Average Runoff Depth = 2.94" 83.93% Pervious = 4.751 ac 16.07% Impervious = 0.910 ac

Page 28

Summary for Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT

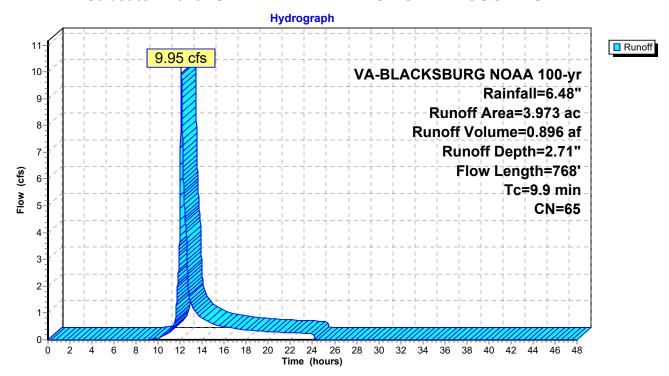
[47] Hint: Peak is 212% of capacity of segment #3

Runoff = 9.95 cfs @ 12.09 hrs, Volume= 0.896 af, Depth= 2.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Area	(ac) C	N Desc	cription		
3.	597 6	31 >75°	% Grass co	over, Good	, HSG B
0.	376	98 Pave	ed parking	, HSG B	
3.	973 6	55 Weig	ghted Aver	age	
3.	597	90.5	4% Pervio	us Area	
0.	376	9.46	% Impervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.1	100	0.0530	0.23		Sheet Flow, Tc1
					Grass: Short n= 0.150 P2= 2.76"
2.4	575	0.0600	3.94		Shallow Concentrated Flow, Tc2
					Unpaved Kv= 16.1 fps
0.4	93	0.0053	3.83	4.70	Pipe Channel, Tc3
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Corrugated PE, smooth interior
9.9	768	Total			

Subcatchment 1S: DA #1 - REACHING POA THROUGH CULVERT



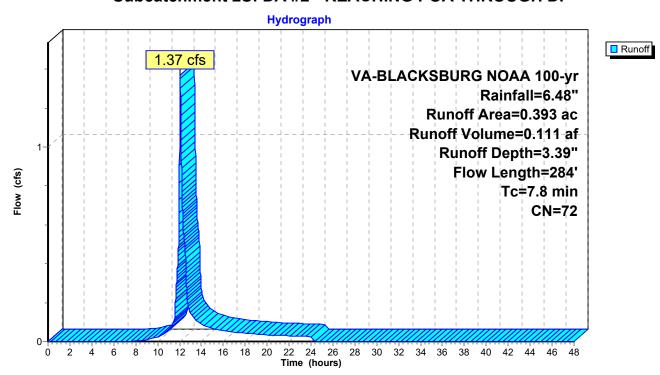
Summary for Subcatchment 2S: DA #2 - REACHING POA THROUGH DI

Runoff = 1.37 cfs @ 12.06 hrs, Volume= 0.111 af, Depth= 3.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Ar	ea ((ac) C	N Desc	cription					
	• • •				over, Good	, HSG B			
	0.120 98 Paved parking, HSG B								
	0.	393 7	'2 Weig	hted Aver	age				
	0.273 69.47% Pervious Area								
	0	120	30.5	3% Imperv	ious Area				
	٠.	0	00.0	o / opo	1040 / 1104				
-	Гс	Length	Slope	Velocity	Capacity	Description			
(mi	n)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'			
6	.6	100	0.0650	0.25		Sheet Flow, Tc4			
_	-					Grass: Short n= 0.150 P2= 2.76"			
0	.5	127	0.0830	4.64		Shallow Concentrated Flow, Tc5			
U	.0	121	0.0000	4.04		Unpaved Kv= 16.1 fps			
0	.7	57	0.0050	1 11		·			
U	. /	57	0.0050	1.44		Shallow Concentrated Flow, Tc6			
						Paved Kv= 20.3 fps			
7	.8	284	Total						

Subcatchment 2S: DA #2 - REACHING POA THROUGH DI



Page 30

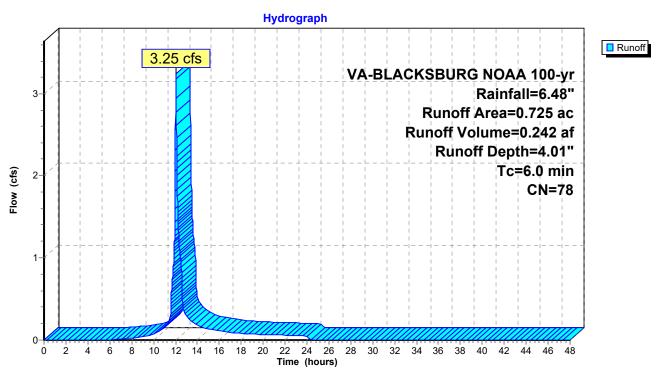
Summary for Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff 3.25 cfs @ 12.04 hrs, Volume= 0.242 af, Depth= 4.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Area	(ac)	CN	Desc	Description					
0	.310	61	>75%	75% Grass cover, Good, HSG B					
0	.300	98	Pave	Paved parking, HSG B					
0	.115	70	1/2 a	cre lots, 2	5% imp, H	SG B			
0	.725	78	Weig	hted Aver	age				
0	.396		54.60	6% Pervio	us Area				
0	.329		45.34	4% Imperv	ious Area				
Tc	Leng		Slope	Velocity	Capacity	Description			
<u>(min)</u>	(fee	et)	(ft/ft)	(ft/sec)	(cfs)				
6.0						Direct Entry, TR-55			

Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM



Page 31

Page 31

Summary for Subcatchment 4S: DIRECT RUNOFF

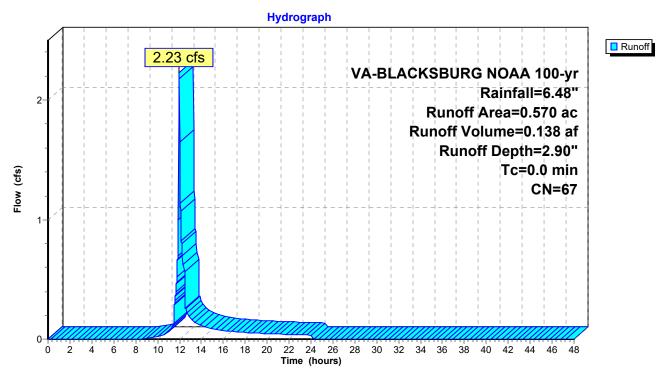
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 2.23 cfs @ 11.99 hrs, Volume= 0.138 af, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Area ((ac)	CN	Description				
0.4	485	61	>75% Grass cover, Good, HSG B				
0.0	085	98	Paved parking, HSG B				
0.:	570	67	Weighted Average				
0.4	485		85.09% Pervious Area				
0.0	085		14.91% Impervious Area				

Subcatchment 4S: DIRECT RUNOFF



PRE DEV

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 32

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 4.01" for 100-yr event

Inflow = 3.25 cfs @ 12.04 hrs, Volume= 0.242 af

Outflow = 3.16 cfs @ 12.06 hrs, Volume= 0.242 af, Atten= 3%, Lag= 1.0 min

Primary = 3.16 cfs @ 12.06 hrs, Volume= 0.242 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,207.47' @ 12.06 hrs Surf.Area= 0.032 ac Storage= 0.056 af

Plug-Flow detention time= 195.8 min calculated for 0.242 af (100% of inflow)

Center-of-Mass det. time= 195.9 min (1,021.2 - 825.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
		0.057 (T (A " O(

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices				
#1	Primary	2,203.96'	15.0" Round 15" Culvert				
	•		L= 54.0' CPP, square edge headwall, Ke= 0.500				
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900				
			n= 0.011, Flow Area= 1.23 sf				
#2	Device 1	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
#3	Device 1	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads				
#4	Device 1	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir				
			Head (feet) 0.20 0.40 0.60 0.80 1.00				
			Coef. (English) 2.80 2.92 3.08 3.30 3.32				

Primary OutFlow Max=3.15 cfs @ 12.06 hrs HW=2,207.47' (Free Discharge)

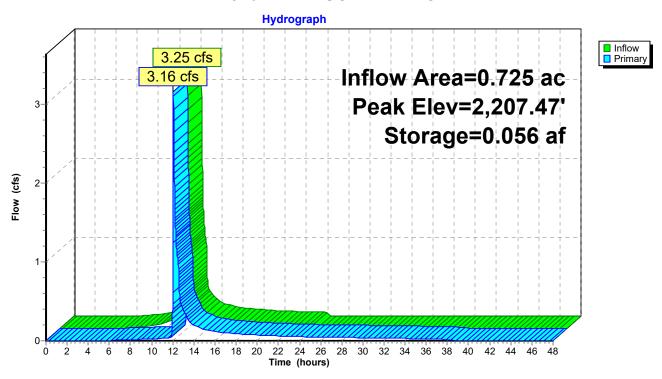
-1=15" Culvert (Passes 3.15 cfs of 9.92 cfs potential flow)

2=Orifice/Grate (Orifice Controls 0.06 cfs @ 8.91 fps)

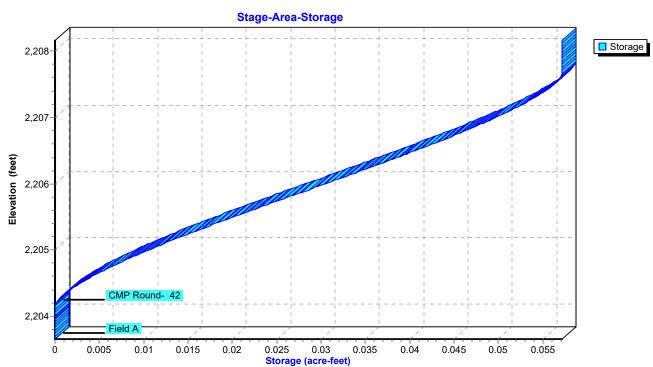
-3=Orifice/Grate (Orifice Controls 0.59 cfs @ 4.36 fps)

-4=Broad-Crested Rectangular Weir (Weir Controls 2.49 cfs @ 1.74 fps)

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Page 34

Summary for Link 1L: POA

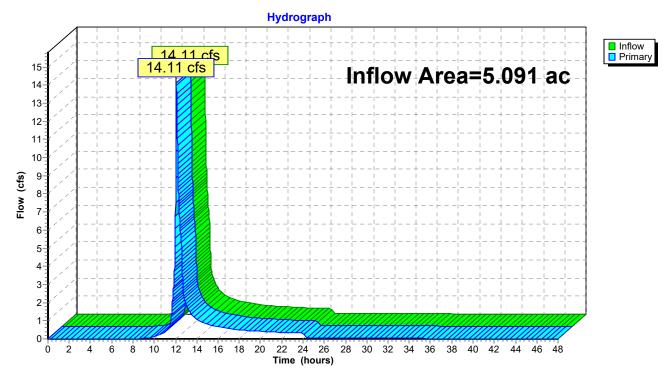
Inflow Area = 5.091 ac, 16.20% Impervious, Inflow Depth = 2.94" for 100-yr event

Inflow = 14.11 cfs @ 12.08 hrs, Volume= 1.249 af

Primary = 14.11 cfs @ 12.08 hrs, Volume= 1.249 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA







POST DEV









RV CALC

VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022

Page 2

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: PRE DEV Runoff Area = 2.662 ac 10.29% Impervious Runoff Depth = 0.22"

Tc=0.0 min CN=65 Runoff=0.43 cfs 0.048 af

Subcatchment 2S: POST DEV Runoff Area=3.014 ac 57.83% Impervious Runoff Depth=0.83"

Tc=0.0 min CN=82 Runoff=4.51 cfs 0.209 af

Summary for Subcatchment 1S: PRE DEV

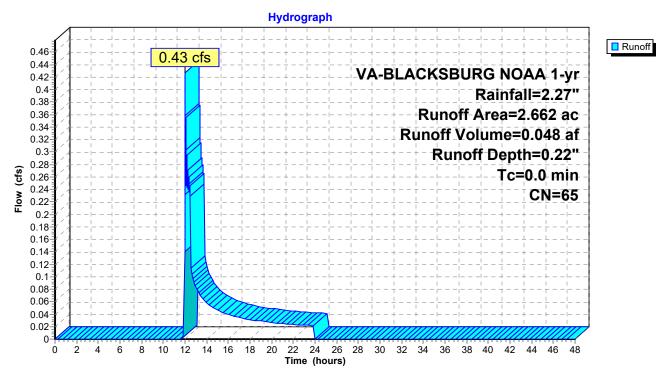
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.43 cfs @ 11.99 hrs, Volume= 0.048 af, Depth= 0.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area	ı (ac)	CN	Description				
2	2.388	61	>75% Grass cover, Good, HSG B				
C).274	98	Paved parking, HSG B				
2	2.662	65	Weighted Average				
2	2.388		89.71% Pervious Area				
C).274		10.29% Impervious Area				

Subcatchment 1S: PRE DEV



Summary for Subcatchment 2S: POST DEV

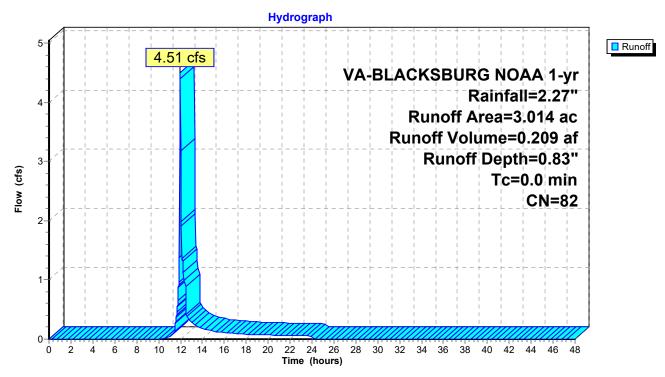
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

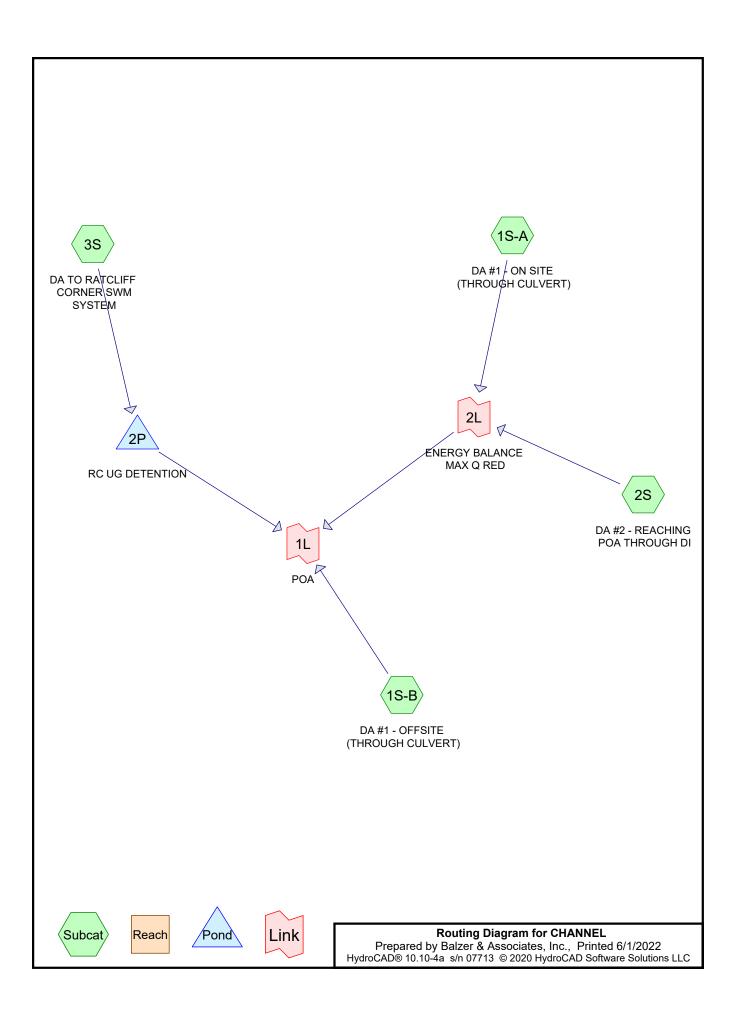
Runoff = 4.51 cfs @ 11.99 hrs, Volume= 0.209 af, Depth= 0.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area (ac)	CN	Description				
1.271	61	>75% Grass cover, Good, HSG B				
1.743	98	Paved parking, HSG B				
3.014	82	Weighted Average				
1.271		42.17% Pervious Area				
1.743	}	57.83% Impervious Area				

Subcatchment 2S: POST DEV





CHANNEL

VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 6/1/2022

Page 2

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S-A: DA #1 - ON SITE Runoff Area=2.269 ac 6.79% Impervious Runoff Depth=0.19"

Flow Length=768' Tc=9.9 min CN=64 Runoff=0.19 cfs 0.037 af

Subcatchment 1S-B: DA #1 - OFFSITE Runoff Area=1.704 ac 13.03% Impervious Runoff Depth=0.24"

Flow Length=768' Tc=9.9 min CN=66 Runoff=0.21 cfs 0.034 af

Subcatchment 2S: DA #2 - REACHING POA Runoff Area=0.393 ac 30.53% Impervious Runoff Depth=0.41"

Flow Length=284' Tc=7.8 min CN=72 Runoff=0.15 cfs 0.014 af

Subcatchment3S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=0.64"

Tc=6.0 min CN=78 Runoff=0.60 cfs 0.039 af

Pond 2P: RC UG DETENTION Peak Elev=2,205.39' Storage=0.018 af Inflow=0.60 cfs 0.039 af

Outflow=0.04 cfs 0.039 af

Link 1L: POA Inflow=0.29 cfs 0.082 af Primary=0.29 cfs 0.082 af

Link 2L: ENERGY BALANCE MAX Q RED x 0.18 Inflow=0.28 cfs 0.050 af

Primary=0.05 cfs 0.009 af Secondary=0.23 cfs 0.041 af

Page 3

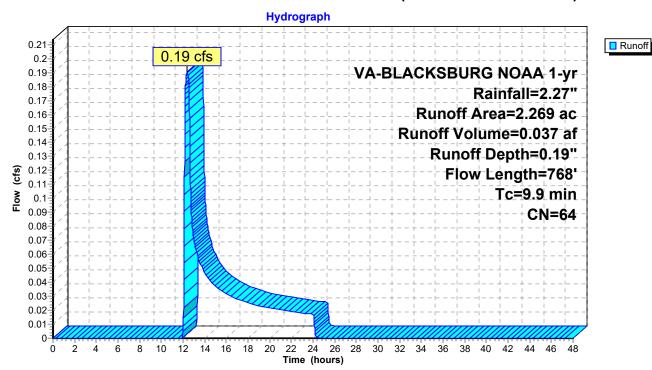
Summary for Subcatchment 1S-A: DA #1 - ON SITE (THROUGH CULVERT)

Runoff = 0.19 cfs @ 12.37 hrs, Volume= 0.037 af, Depth= 0.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area	(ac) C	N Desc	cription		
2.	.115 6	61 >75°	% Grass co	over, Good	, HSG B
0.	.154	98 Pave	ed parking	, HSG B	
2.	.269 6	64 Weig	ghted Aver	age	
2.	.115	93.2	1% Pervio	us Area	
0.	.154	6.79	% Impervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
7.1	100	0.0530	0.23		Sheet Flow, Tc1
					Grass: Short n= 0.150 P2= 2.76"
2.4	575	0.0600	3.94		Shallow Concentrated Flow, Tc2
					Unpaved Kv= 16.1 fps
0.4	93	0.0053	3.83	4.70	Pipe Channel, Tc3
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.013 Corrugated PE, smooth interior
9.9	768	Total			

Subcatchment 1S-A: DA #1 - ON SITE (THROUGH CULVERT)



Page 4

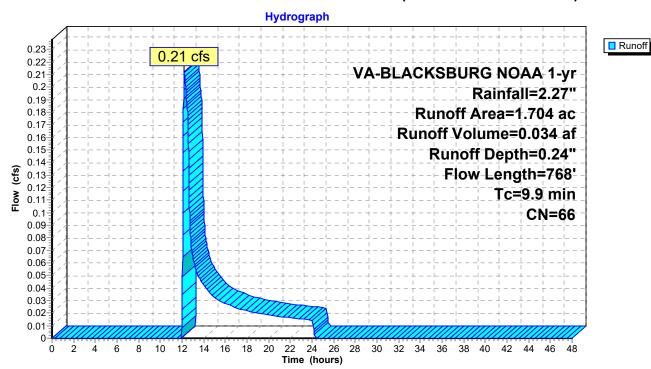
Summary for Subcatchment 1S-B: DA #1 - OFFSITE (THROUGH CULVERT)

Runoff = 0.21 cfs @ 12.22 hrs, Volume= 0.034 af, Depth= 0.24"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

	Area	(ac) C	N Des	cription		
	1.	.482 (31 >75°	% Grass co	over, Good	, HSG B
	0.	.222	98 Pave	ed parking	, HSG B	
	1.	.704	66 Weig	ghted Aver	age	
	1.	.482	86.9	7% Pervio	us Area	
	0.	.222	13.0	3% Imper\	∕ious Area	
	т.	1 41.	01	V/-1	0	Describe the co
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	7.1	100	0.0530	0.23		Sheet Flow, Tc1
						Grass: Short n= 0.150 P2= 2.76"
	2.4	575	0.0600	3.94		Shallow Concentrated Flow, Tc2
						Unpaved Kv= 16.1 fps
	0.4	93	0.0053	3.83	4.70	Pipe Channel, Tc3
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.013 Corrugated PE, smooth interior
	9.9	768	Total			

Subcatchment 1S-B: DA #1 - OFFSITE (THROUGH CULVERT)



Page 5

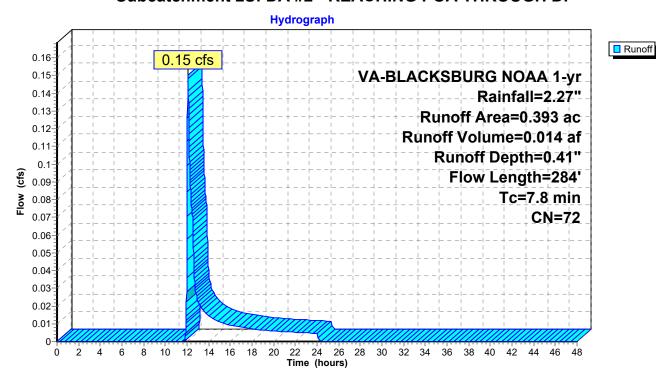
Summary for Subcatchment 2S: DA #2 - REACHING POA THROUGH DI

Runoff = 0.15 cfs @ 12.08 hrs, Volume= 0.014 af, Depth= 0.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

	Area	(ac) C	N Desc	cription					
	0.273 61			>75% Grass cover, Good, HSG B					
_	0.	120 9	8 Pave	ed parking,	, HSG B				
	0.	393 7	'2 Weig	ghted Aver	age				
	0.	273	69.4	7% Pervio	us Area				
	0.	120	30.5	3% Imperv	ious Area				
				•					
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	'			
	6.6	100	0.0650	0.25		Sheet Flow, Tc4			
						Grass: Short n= 0.150 P2= 2.76"			
	0.5	127	0.0830	4.64		Shallow Concentrated Flow, Tc5			
	0.0		0.000			Unpaved Kv= 16.1 fps			
	0.7	57	0.0050	1.44		Shallow Concentrated Flow, Tc6			
	5.1	01	0.0000	1		Paved Kv= 20.3 fps			
_	7.0	004	T ()			1 4 VOU 1 1 V - 20.0 1 PO			
	7.8	284	Total						

Subcatchment 2S: DA #2 - REACHING POA THROUGH DI



Page 6

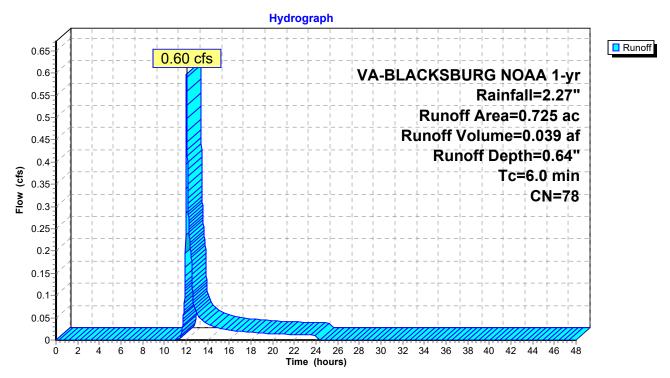
Summary for Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 0.60 cfs @ 12.05 hrs, Volume= 0.039 af, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

_	Area	(ac)	CN	Desc	Description							
	0.	310 61 >75% Grass cover, Good, HSG B										
	0.	300	98	Pave	ed parking,	HSG B						
	0.	115	70	1/2 a	cre lots, 2	5% imp, H	SG B					
	0.	725	78	Weig	hted Aver	age						
	0.396				54.66% Pervious Area							
	0.329			45.3	45.34% Impervious Area							
	Тс	Leng	th	Slope	Velocity	Capacity	Description					
_	(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)						
	6.0						Direct Entry, 1	TR-55				

Subcatchment 3S: DA TO RATCLIFF CORNER SWM SYSTEM



CHANNEL

Prepared by Balzer & Associates, Inc.

Printed 6/1/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 7

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 0.64" for 1-yr event

Inflow = 0.60 cfs @ 12.05 hrs, Volume= 0.039 af

Outflow = 0.04 cfs @ 13.96 hrs, Volume= 0.039 af, Atten= 94%, Lag= 114.9 min

Primary = 0.04 cfs @ 13.96 hrs, Volume= 0.039 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,205.39' @ 13.96 hrs Surf.Area= 0.032 ac Storage= 0.018 af

Plug-Flow detention time= 252.5 min calculated for 0.039 af (100% of inflow)

Center-of-Mass det. time= 252.5 min (1,125.6 - 873.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
		0.057 of	Total Available Storage

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.96'	15.0" Round 15" Culvert
	•		L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#2	Device 1	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#3	Device 1	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 1	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.04 cfs @ 13.96 hrs HW=2,205.39' (Free Discharge)

-1=15" Culvert (Passes 0.04 cfs of 4.43 cfs potential flow)

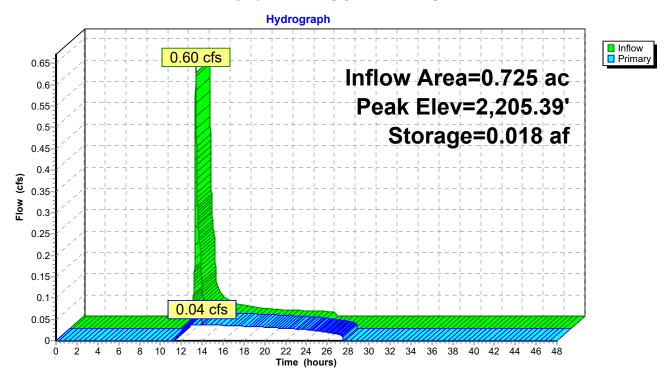
2=Orifice/Grate (Orifice Controls 0.04 cfs @ 5.57 fps)

-3=Orifice/Grate (Controls 0.00 cfs)

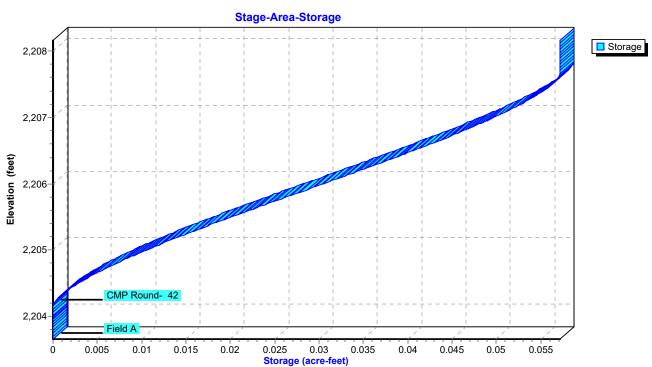
-4=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Page 8

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Page 9

Summary for Link 1L: POA

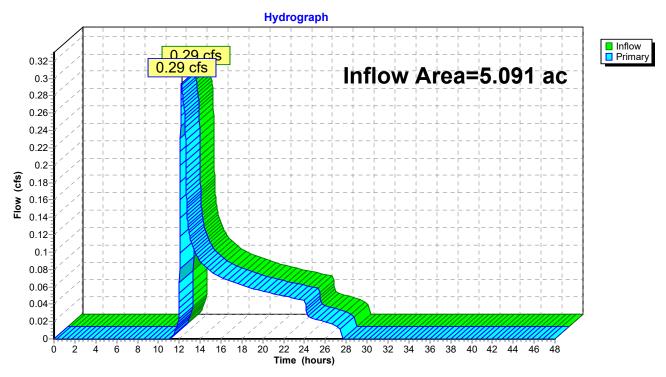
Inflow Area = 5.091 ac, 16.20% Impervious, Inflow Depth = 0.19" for 1-yr event

Inflow = 0.29 cfs @ 12.22 hrs, Volume= 0.082 af

Primary = 0.29 cfs @ 12.22 hrs, Volume= 0.082 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



Printed 6/1/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 10

Summary for Link 2L: ENERGY BALANCE MAX Q RED

Inflow Area = 2.662 ac, 10.29% Impervious, Inflow Depth = 0.23" for 1-yr event

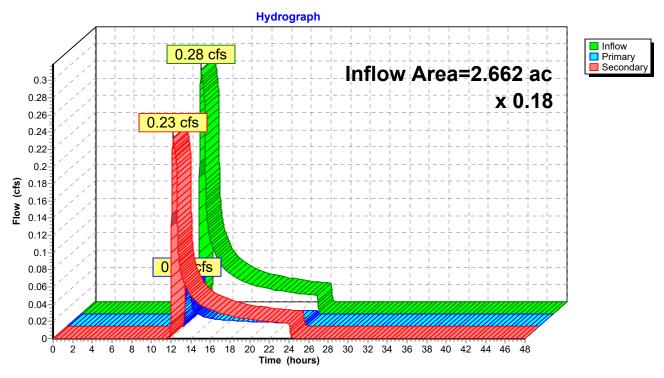
Inflow = 0.28 cfs @ 12.22 hrs, Volume= 0.050 af

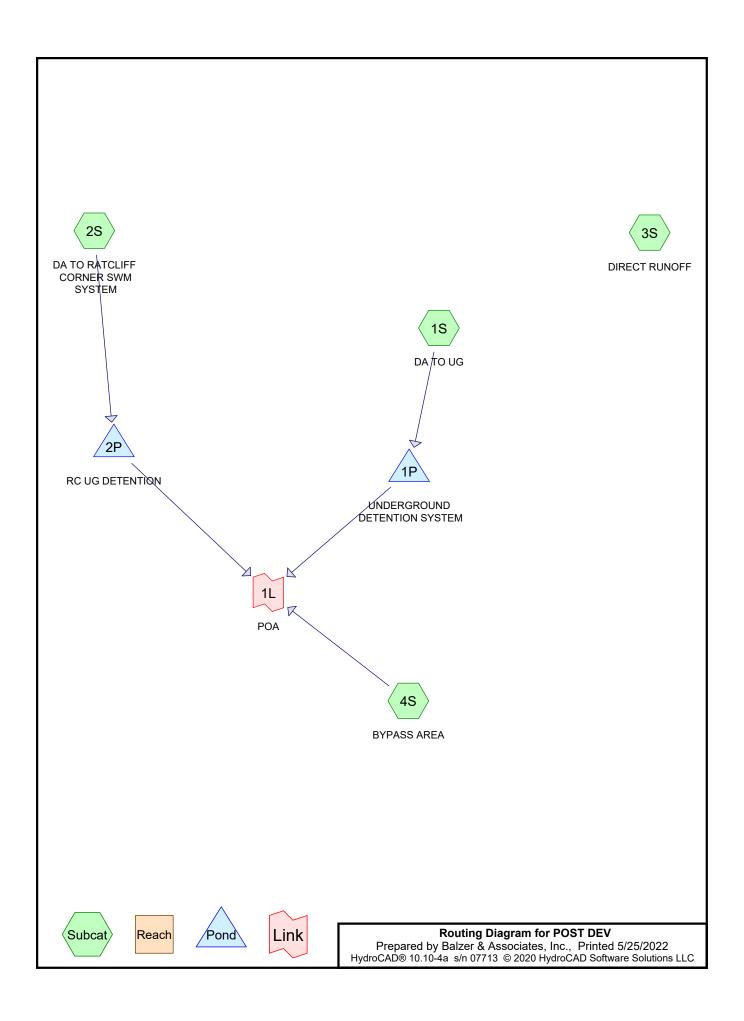
Primary = 0.05 cfs @ 12.22 hrs, Volume= 0.009 af, Atten= 82%, Lag= 0.0 min

Secondary = 0.23 cfs @ 12.22 hrs, Volume= 0.041 af

Primary outflow = Inflow x 0.18, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 2L: ENERGY BALANCE MAX Q RED





POST DEV

Prepared by Balzer & Associates, Inc.

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022 Page 2

Area Listing (all nodes)

A	Area (CN	Description
(ac	res)		(subcatchment-numbers)
0.	.115	70	1/2 acre lots, 25% imp, HSG B (2S)
3.	.200	61	>75% Grass cover, Good, HSG B (1S, 2S, 3S, 4S)
2.	.345	98	Paved parking, HSG B (1S, 2S, 3S, 4S)
5	.660	77	TOTAL AREA

POST DEV

VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 3

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA TO UG Runoff Area=4.657 ac 41.34% Impervious Runoff Depth=0.56"

Tc=6.0 min CN=76 Runoff=3.18 cfs 0.217 af

Subcatchment 2S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=0.64"

Tc=6.0 min CN=78 Runoff=0.60 cfs 0.039 af

Subcatchment 3S: DIRECT RUNOFF Runoff Area=0.203 ac 32.51% Impervious Runoff Depth=0.45"

Tc=0.0 min CN=73 Runoff=0.14 cfs 0.008 af

Subcatchment 4S: BYPASS AREA Runoff Area=0.075 ac 72.00% Impervious Runoff Depth=1.19"

Tc=6.0 min CN=88 Runoff=0.13 cfs 0.007 af

Pond 1P: UNDERGROUND DETENTION | Peak Elev=2,204.91' | Storage=0.102 af | Inflow=3.18 cfs 0.217 af

Outflow=0.18 cfs 0.217 af

Pond 2P: RC UG DETENTION Peak Elev=2,205.39' Storage=0.018 af Inflow=0.60 cfs 0.039 af

Outflow=0.04 cfs 0.039 af

Link 1L: POA Inflow=0.26 cfs 0.263 af

Primary=0.26 cfs 0.263 af

Total Runoff Area = 5.660 ac Runoff Volume = 0.271 af Average Runoff Depth = 0.57" 58.06% Pervious = 3.286 ac 41.94% Impervious = 2.374 ac

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 4

Summary for Subcatchment 1S: DA TO UG

Runoff = 3.18 cfs @ 12.05 hrs, Volume= 0.217 af, Depth= 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area	(ac) CN Description								
2	.732	61	>75%	√ Grass co	over, Good,	, HSG B			
1	.925	98	Pave	ed parking,	HSG B				
4	4.657 76 Weighted Average				age				
2	2.732 58				58.66% Pervious Area				
1	1.925			4% Imperv	rious Area				
Tc	Lengt	th :	Slope	Velocity	Capacity	Description			
(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	'			
6.0	•				·	Direct Entry, DIRECT			

Subcatchment 1S: DA TO UG

Hydrograph Runoff 3.18 cfs VA-BLACKSBURG NOAA 1-yr 3 Rainfall=2.27" Runoff Area=4.657 ac Runoff Volume=0.217 af Runoff Depth=0.56" Flow (cfs) Tc=6.0 min CN=76 20 22 24 26 10 12 14 16 18 28 30 32 34 36 38 40 42 44 46 48 Time (hours)

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 5

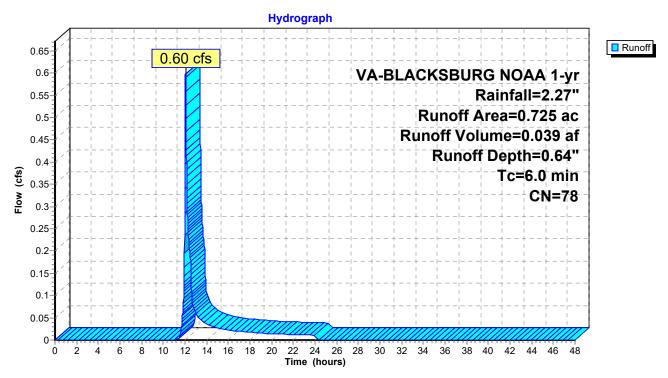
Summary for Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 0.60 cfs @ 12.05 hrs, Volume= 0.039 af, Depth= 0.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

	Area ((ac)	CN	Desc	Description						
	0.	310	61	>75%	√ Grass co	ver, Good	I, HSG B				
	0.	300	98	Pave	ed parking,	HSG B					
	0.	115	70	1/2 a	cre lots, 2	5% imp, H	SG B	_			
	0.	725	78	Weig	hted Aver	age					
	0.396 54.66%				66% Pervious Area						
	0.329			45.3	45.34% Impervious Area						
	Тс	Leng		Slope	Velocity	Capacity	Description				
<u>(r</u>	min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, TR-55				

Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM



Page 6

Summary for Subcatchment 3S: DIRECT RUNOFF

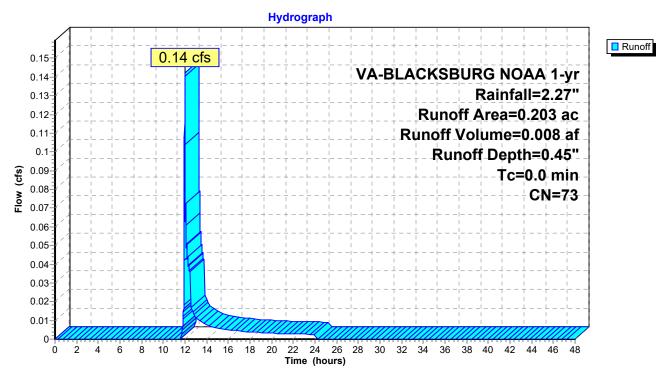
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.14 cfs @ 11.99 hrs, Volume= 0.008 af, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

	Area (ac)	CN	Description
	0.137	61	>75% Grass cover, Good, HSG B
*	0.066	98	Paved parking, HSG B
	0.203	73	Weighted Average
	0.137		67.49% Pervious Area
	0.066		32.51% Impervious Area

Subcatchment 3S: DIRECT RUNOFF



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 7

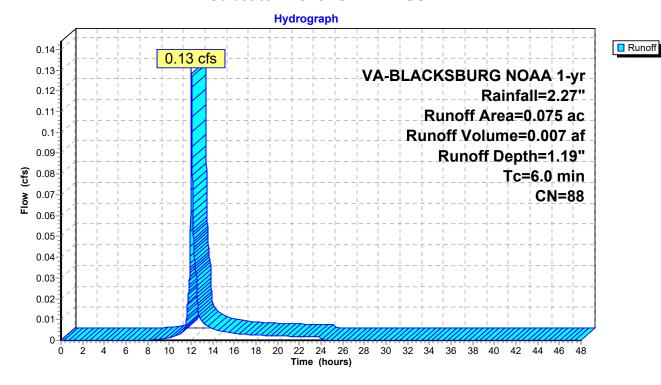
Summary for Subcatchment 4S: BYPASS AREA

Runoff = 0.13 cfs @ 12.04 hrs, Volume= 0.007 af, Depth= 1.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 1-yr Rainfall=2.27"

Area	a (ac) CN Description					
0.	.021	61	>75%	√ Grass co	over, Good,	d, HSG B
0	.054	98	Pave	ed parking,	HSG B	
0.	0.075 88 Weighted Average				age	
0.	.021		28.00	0% Pervio	us Area	
0.	0.054			0% Imperv	rious Area	
Тс	Lengt	h S	Slope	Velocity	Capacity	Description
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)	
6.0						Direct Entry, DIRECT

Subcatchment 4S: BYPASS AREA



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 8

Summary for Pond 1P: UNDERGROUND DETENTION SYSTEM

Inflow Area = 4.657 ac, 41.34% Impervious, Inflow Depth = 0.56" for 1-yr event

Inflow = 3.18 cfs @ 12.05 hrs, Volume= 0.217 af

Outflow = 0.18 cfs @ 14.48 hrs, Volume= 0.217 af, Atten= 94%, Lag= 145.7 min

Primary = 0.18 cfs @ 14.48 hrs, Volume= 0.217 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 2,204.91' @ 14.48 hrs Surf.Area= 0.099 ac Storage= 0.102 af

Plug-Flow detention time= 305.1 min calculated for 0.217 af (100% of inflow)

Center-of-Mass det. time= 305.1 min (1,186.8 - 881.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,202.35'	0.000 af	44.50'W x 97.00'L x 6.00'H Field A
			0.595 af Overall - 0.263 af Embedded = 0.332 af x 0.0% Voids
#2A	2,202.85'	0.263 af	CMP Round 60 x 30 Inside #1
			Effective Size= 60.0"W x 60.0"H => 19.63 sf x 20.00'L = 392.7 cf
			Overall Size= 60.0"W x 60.0"H x 20.00'L
			Row Length Adjustment= -10.00' x 19.63 sf x 6 rows
			42.50' Header x 19.63 sf x 1 = 834.5 cf Inside

0.263 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,202.65'	15.0" Round 15" HDPE Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,202.65' / 2,202.45' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,202.75'	2.2" Vert. 2.25" Dia. Orifice C= 0.600
			Limited to weir flow at low heads
#3	Device 1	2,204.95'	12.0" W x 3.0" H Vert. 10" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#4	Device 1	2,205.45'	16.0" W x 3.0" H Vert. 18" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#5	Device 1	2,206.85'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.18 cfs @ 14.48 hrs HW=2,204.91' (Free Discharge)

-1=15" HDPE Culvert (Passes 0.18 cfs of 7.56 cfs potential flow)

-2=2.25" Dia. Orifice (Orifice Controls 0.18 cfs @ 6.92 fps)

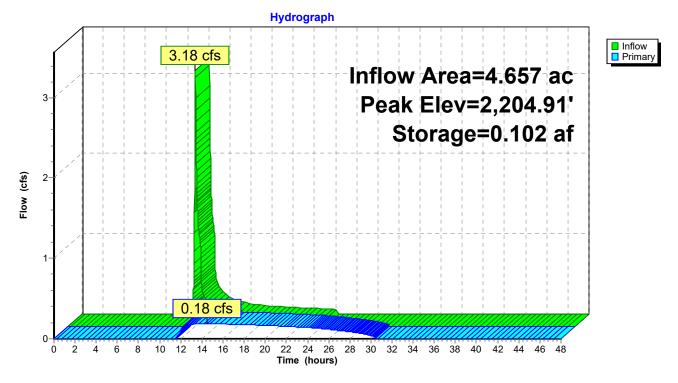
—3=10" x 3" Rect. Orifice (Controls 0.00 cfs)

-4=18" x 3" Rect. Orifice (Controls 0.00 cfs)

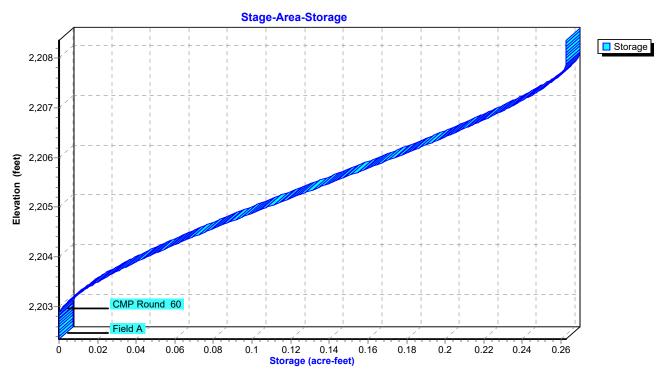
-5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Pond 1P: UNDERGROUND DETENTION SYSTEM



Pond 1P: UNDERGROUND DETENTION SYSTEM



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 10

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 0.64" for 1-yr event

Inflow = 0.60 cfs @ 12.05 hrs, Volume= 0.039 af

Outflow = 0.04 cfs (a) 13.96 hrs, Volume= 0.039 af, Atten= 94%, Lag= 114.9 min

Primary = 0.04 cfs @ 13.96 hrs, Volume= 0.039 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,205.39' @ 13.96 hrs Surf.Area= 0.032 ac Storage= 0.018 af

Plug-Flow detention time= 252.5 min calculated for 0.039 af (100% of inflow)

Center-of-Mass det. time= 252.5 min (1,125.6 - 873.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.78'	15.0" Round 15" Culvert Extension L= 133.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.78' / 2,202.67' S= 0.0083 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,203.96'	15.0" Round 15" Culvert
			L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#3	Device 2	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.04 cfs @ 13.96 hrs HW=2,205.39' (Free Discharge)

—1=15" Culvert Extension (Passes 0.04 cfs of 5.85 cfs potential flow)

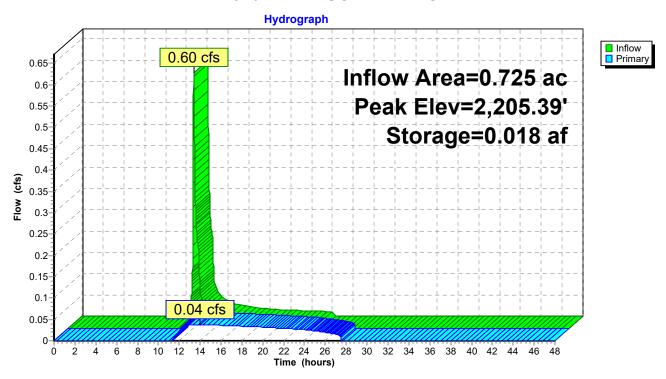
-2=15" Culvert (Passes 0.04 cfs of 4.43 cfs potential flow)

3=Orifice/Grate (Orifice Controls 0.04 cfs @ 5.57 fps)

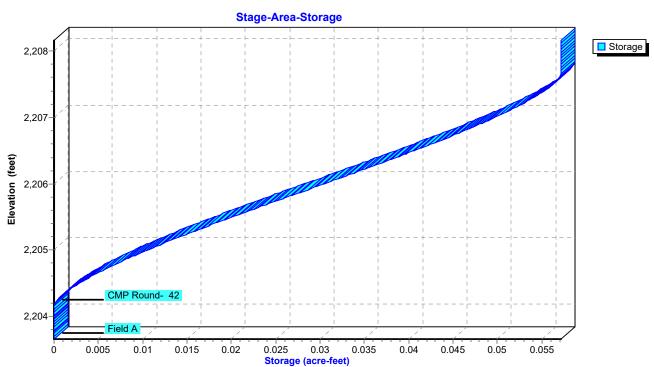
-4=Orifice/Grate (Controls 0.00 cfs)

-5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Summary for Link 1L: POA

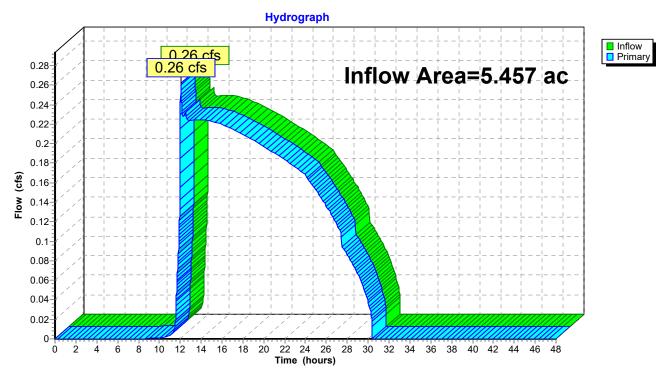
Inflow Area = 5.457 ac, 42.29% Impervious, Inflow Depth = 0.58" for 1-yr event

Inflow = 0.26 cfs @ 12.05 hrs, Volume= 0.263 af

Primary = 0.26 cfs @ 12.05 hrs, Volume= 0.263 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



POST DEV

VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 13

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA TO UG Runoff Area=4.657 ac 41.34% Impervious Runoff Depth=0.85"

Tc=6.0 min CN=76 Runoff=5.22 cfs 0.330 af

Subcatchment 2S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=0.95"

Tc=6.0 min CN=78 Runoff=0.94 cfs 0.058 af

Subcatchment 3S: DIRECT RUNOFF Runoff Area=0.203 ac 32.51% Impervious Runoff Depth=0.71"

Tc=0.0 min CN=73 Runoff=0.24 cfs 0.012 af

Subcatchment 4S: BYPASS AREA Runoff Area=0.075 ac 72.00% Impervious Runoff Depth=1.60"

Tc=6.0 min CN=88 Runoff=0.17 cfs 0.010 af

Pond 1P: UNDERGROUND DETENTION | Peak Elev=2,205.40' | Storage=0.135 af | Inflow=5.22 cfs 0.330 af

Outflow=0.88 cfs 0.330 af

Pond 2P: RC UG DETENTION Peak Elev=2,205.97' Storage=0.030 af Inflow=0.94 cfs 0.058 af

Outflow=0.04 cfs 0.058 af

Link 1L: POA Inflow=0.95 cfs 0.398 af

Primary=0.95 cfs 0.398 af

Total Runoff Area = 5.660 ac Runoff Volume = 0.410 af Average Runoff Depth = 0.87" 58.06% Pervious = 3.286 ac 41.94% Impervious = 2.374 ac

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 14

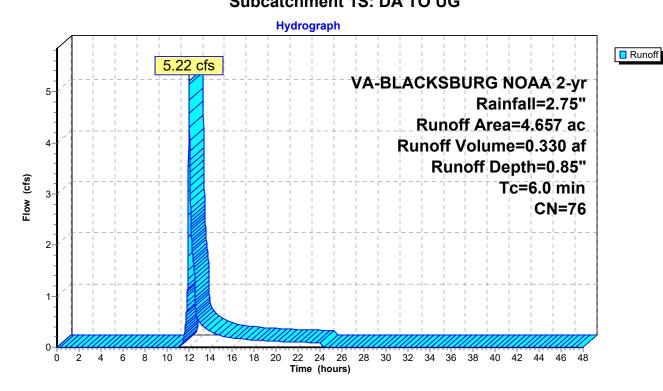
Summary for Subcatchment 1S: DA TO UG

Runoff = 5.22 cfs @ 12.05 hrs, Volume= 0.330 af, Depth= 0.85"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

Area	(ac)	CN	Desc	Description					
2	.732	61	>75%	√ Grass co	over, Good,	, HSG B			
1	.925	98	Pave	ed parking,	HSG B				
4	4.657 76 Weighted Average				age				
2	2.732 58.66% Pervious Area				us Area				
1	1.925			4% Imperv	rious Area				
Tc	Lengt	th :	Slope	Velocity	Capacity	Description			
(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	'			
6.0	•				·	Direct Entry, DIRECT			

Subcatchment 1S: DA TO UG



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 15

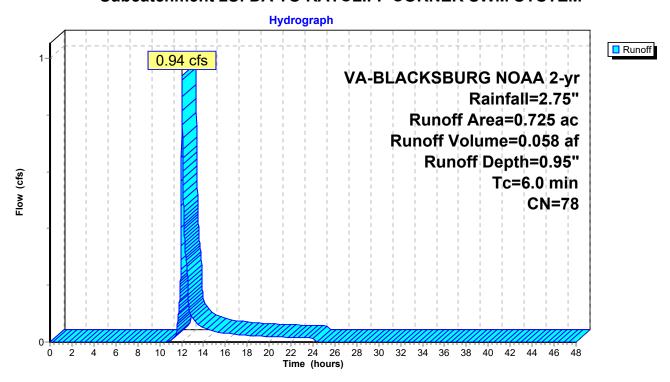
Summary for Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 0.94 cfs @ 12.04 hrs, Volume= 0.058 af, Depth= 0.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

Area	(ac)	CN	Desc	Description						
0	.310	61	>75%	6 Grass co	ver, Good	, HSG B				
0	.300	98	Pave	ed parking,	HSG B					
0	.115	70	1/2 a	cre lots, 2	5% imp, H	SG B				
0	.725	78	Weig	hted Aver	age					
0	0.396 54.66% Pervious Area				us Area					
0	.329		45.3	4% Imperv	ious Area					
Тс	Lengt		Slope	Velocity	Capacity	Description				
<u>(min)</u>	(fee	t)	(ft/ft)	(ft/sec)	(cfs)					
6.0						Direct Entry, TR-55				

Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM



Summary for Subcatchment 3S: DIRECT RUNOFF

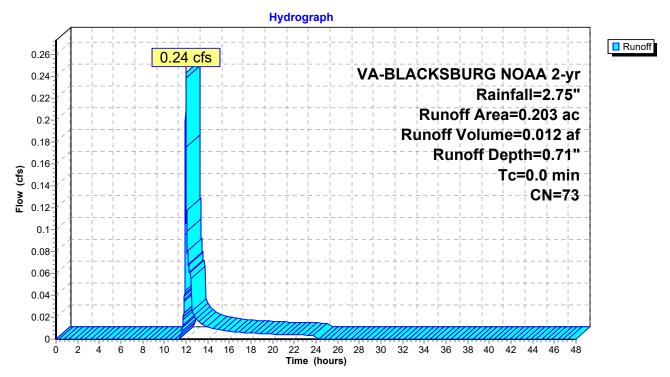
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.24 cfs @ 11.99 hrs, Volume= 0.012 af, Depth= 0.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

	Area (ac)	CN	Description
	0.137	61	>75% Grass cover, Good, HSG B
*	0.066	98	Paved parking, HSG B
	0.203	73	Weighted Average
	0.137		67.49% Pervious Area
	0.066		32.51% Impervious Area

Subcatchment 3S: DIRECT RUNOFF



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 17

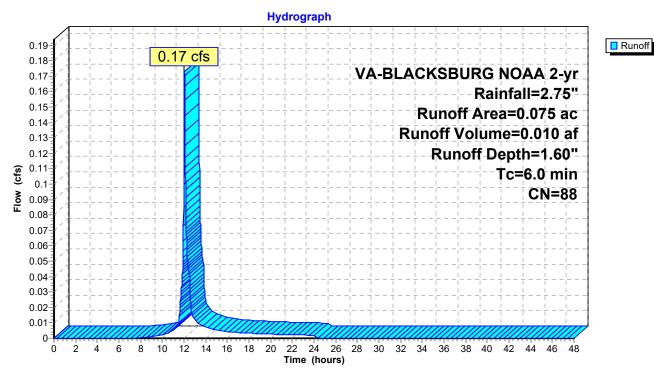
Summary for Subcatchment 4S: BYPASS AREA

Runoff = 0.17 cfs @ 12.04 hrs, Volume= 0.010 af, Depth= 1.60"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 2-yr Rainfall=2.75"

A	rea ((ac)	CN	Desc	Description						
	0.0	021	61	>75%	6 Grass co	over, Good,	I, HSG B				
	0.0	054	98	Pave	d parking,	HSG B					
	0.075 88 Weighted Average					age					
	0.021 28.00% Pervious Area					us Area					
	0.054			72.00	0% Imperv	ious Area					
	т.	المصطا	h (Slana	Valacity	Canacity	Description				
/~		Lengtl		Slope	Velocity	Capacity	Description				
	nin)	(feet	.)	(ft/ft)	(ft/sec)	(cfs)					
	6.0						Direct Entry, DIRECT				

Subcatchment 4S: BYPASS AREA



POST DEV

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 18

Summary for Pond 1P: UNDERGROUND DETENTION SYSTEM

Inflow Area = 4.657 ac, 41.34% Impervious, Inflow Depth = 0.85" for 2-yr event

Inflow = 5.22 cfs @ 12.05 hrs, Volume= 0.330 af

Outflow = 0.88 cfs @ 12.62 hrs, Volume= 0.330 af, Atten= 83%, Lag= 34.2 min

Primary = 0.88 cfs @ 12.62 hrs, Volume= 0.330 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,205.40' @ 12.62 hrs Surf.Area= 0.099 ac Storage= 0.135 af

Plug-Flow detention time= 259.0 min calculated for 0.330 af (100% of inflow)

Center-of-Mass det. time= 259.0 min (1,126.1 - 867.1)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,202.35'	0.000 af	44.50'W x 97.00'L x 6.00'H Field A
			0.595 af Overall - 0.263 af Embedded = 0.332 af x 0.0% Voids
#2A	2,202.85'	0.263 af	CMP Round 60 x 30 Inside #1
			Effective Size= 60.0"W x 60.0"H => 19.63 sf x 20.00'L = 392.7 cf
			Overall Size= 60.0"W x 60.0"H x 20.00'L
			Row Length Adjustment= -10.00' x 19.63 sf x 6 rows
			42.50' Header x 19.63 sf x 1 = 834.5 cf Inside

0.263 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,202.65'	15.0" Round 15" HDPE Culvert
	-		L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,202.65' / 2,202.45' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,202.75'	2.2" Vert. 2.25" Dia. Orifice C= 0.600
			Limited to weir flow at low heads
#3	Device 1	2,204.95'	12.0" W x 3.0" H Vert. 10" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#4	Device 1	2,205.45'	16.0" W x 3.0" H Vert. 18" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#5	Device 1	2,206.85'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.88 cfs @ 12.62 hrs HW=2,205.40' (Free Discharge)

-1=15" HDPE Culvert (Passes 0.88 cfs of 8.61 cfs potential flow)

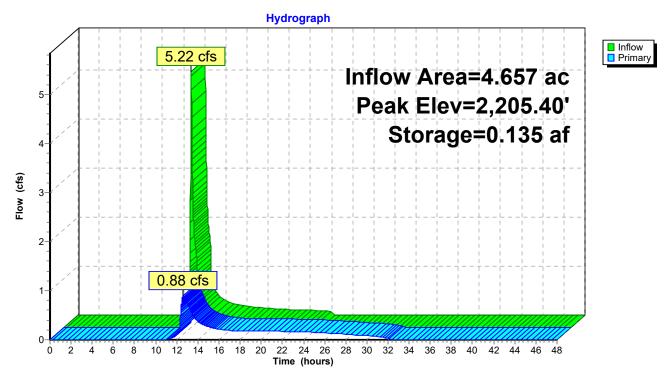
-2=2.25" Dia. Orifice (Orifice Controls 0.20 cfs @ 7.70 fps)

—3=10" x 3" Rect. Orifice (Orifice Controls 0.68 cfs @ 2.73 fps)

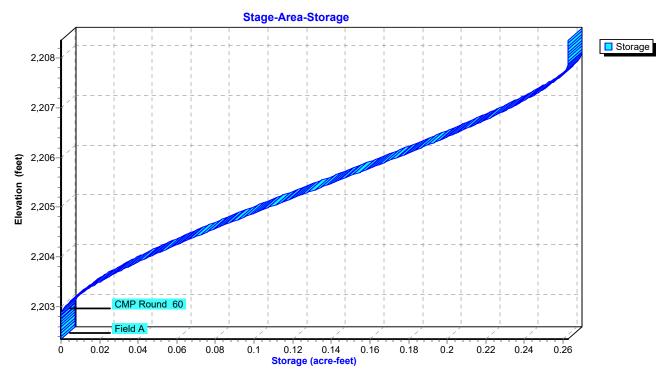
-4=18" x 3" Rect. Orifice (Controls 0.00 cfs)

-5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 1P: UNDERGROUND DETENTION SYSTEM



Pond 1P: UNDERGROUND DETENTION SYSTEM



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 20

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 0.95" for 2-yr event

Inflow = 0.94 cfs @ 12.04 hrs, Volume= 0.058 af

Outflow = 0.04 cfs @ 14.42 hrs, Volume= 0.058 af, Atten= 95%, Lag= 142.4 min

Primary = 0.04 cfs @ 14.42 hrs, Volume= 0.058 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 2,205.97' @ 14.42 hrs Surf.Area= 0.032 ac Storage= 0.030 af

Plug-Flow detention time= 357.8 min calculated for 0.058 af (100% of inflow)

Center-of-Mass det. time= 357.8 min (1,217.8 - 859.9)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.78'	15.0" Round 15" Culvert Extension L= 133.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.78' / 2,202.67' S= 0.0083 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,203.96'	15.0" Round 15" Culvert
			L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#3	Device 2	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.04 cfs @ 14.42 hrs HW=2,205.97' (Free Discharge)

—1=15" Culvert Extension (Passes 0.04 cfs of 6.56 cfs potential flow)

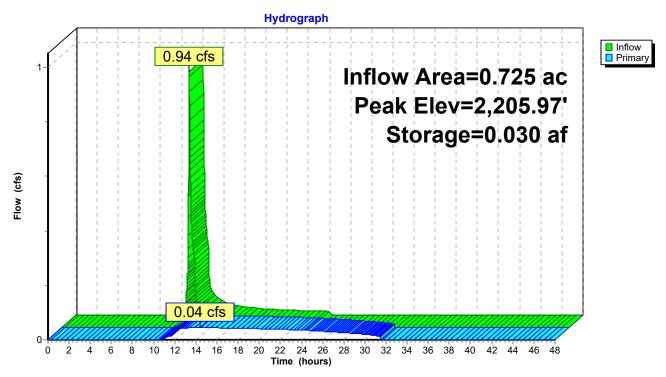
2=15" Culvert (Passes 0.04 cfs of 6.15 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.04 cfs @ 6.67 fps)

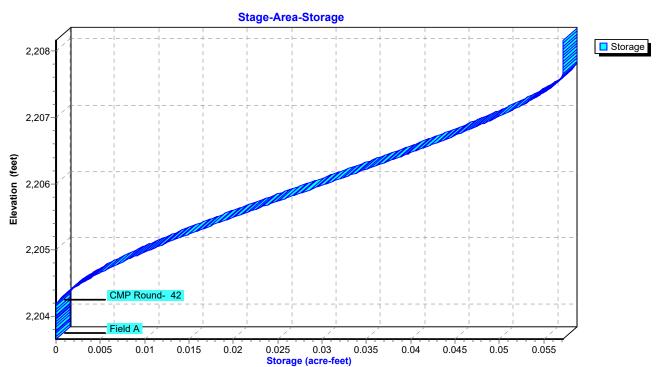
-4=Orifice/Grate (Controls 0.00 cfs)

-5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 22

Summary for Link 1L: POA

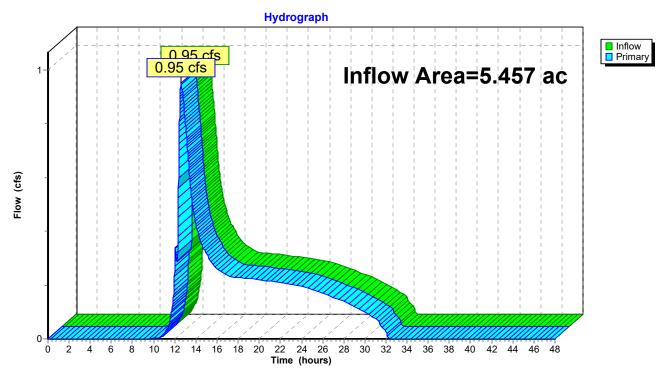
Inflow Area = 5.457 ac, 42.29% Impervious, Inflow Depth = 0.87" for 2-yr event

Inflow = 0.95 cfs @ 12.59 hrs, Volume= 0.398 af

Primary = 0.95 cfs @ 12.59 hrs, Volume= 0.398 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



POST DEV

VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022

Page 23

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA TO UG Runoff Area=4.657 ac 41.34% Impervious Runoff Depth=1.81"

Tc=6.0 min CN=76 Runoff=10.90 cfs 0.702 af

Subcatchment 2S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=1.96"

Tc=6.0 min CN=78 Runoff=1.86 cfs 0.118 af

Subcatchment 3S: DIRECT RUNOFF Runoff Area=0.203 ac 32.51% Impervious Runoff Depth=1.59"

Tc=0.0 min CN=73 Runoff=0.52 cfs 0.027 af

Subcatchment 4S: BYPASS AREA Runoff Area=0.075 ac 72.00% Impervious Runoff Depth=2.81"

Tc=6.0 min CN=88 Runoff=0.28 cfs 0.018 af

Pond 1P: UNDERGROUND DETENTION | Peak Elev=2,206.79' | Storage=0.222 af | Inflow=10.90 cfs 0.702 af

Outflow=3.59 cfs 0.702 af

Pond 2P: RC UG DETENTION Peak Elev=2,207.01' Storage=0.050 af Inflow=1.86 cfs 0.118 af

Outflow=0.45 cfs 0.118 af

Link 1L: POA Inflow=4.08 cfs 0.837 af

Primary=4.08 cfs 0.837 af

Total Runoff Area = 5.660 ac Runoff Volume = 0.864 af Average Runoff Depth = 1.83" 58.06% Pervious = 3.286 ac 41.94% Impervious = 2.374 ac

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 24

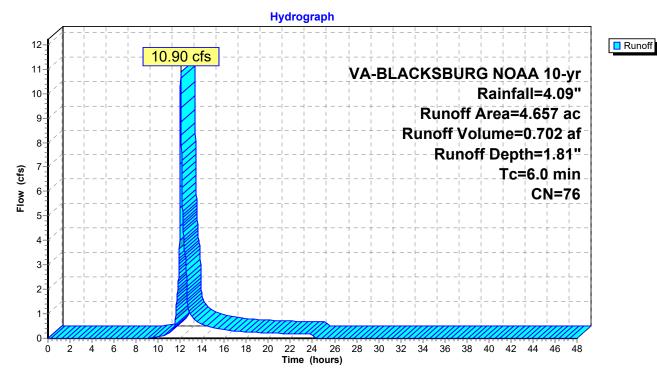
Summary for Subcatchment 1S: DA TO UG

Runoff = 10.90 cfs @ 12.04 hrs, Volume= 0.702 af, Depth= 1.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Area	(ac)	CN	Desc	Description					
2	.732	61	>75%	√ Grass co	over, Good,	, HSG B			
1	.925	98	Pave	ed parking,	HSG B				
4	4.657 76 Weighted Average				age				
2	2.732 58.66% Pervious Area				us Area				
1	1.925			4% Imperv	rious Area				
Tc	Lengt	th :	Slope	Velocity	Capacity	Description			
(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	'			
6.0	•				·	Direct Entry, DIRECT			

Subcatchment 1S: DA TO UG



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 25

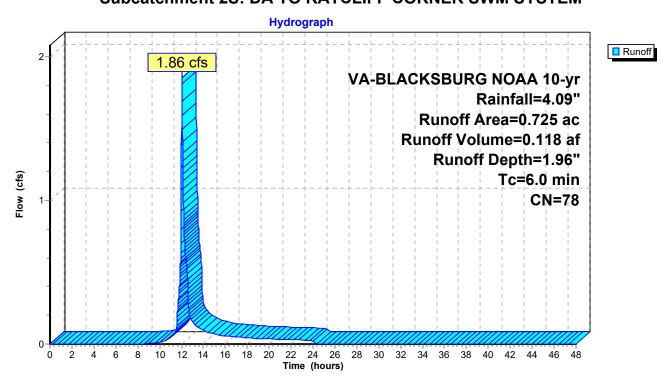
Summary for Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 1.86 cfs @ 12.04 hrs, Volume= 0.118 af, Depth= 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Area	(ac)	CN	Desc	Description						
0.	0.310 61 >75% Grass cover, Good, HSG B									
0.	.300	98	Pave	ed parking,	HSG B					
0.	.115	70	1/2 a	cre lots, 2	5% imp, H	SG B				
0.	725	78	Weig	hted Aver	age					
0.	.396		54.6	6% Pervio	us Area					
0.	.329		45.34	4% Imperv	ious Area					
_			21		0 :	D				
Tc	Lengt		Slope	Velocity	Capacity	Description				
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)					
6.0						Direct Entry, TR-55				

Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM



Summary for Subcatchment 3S: DIRECT RUNOFF

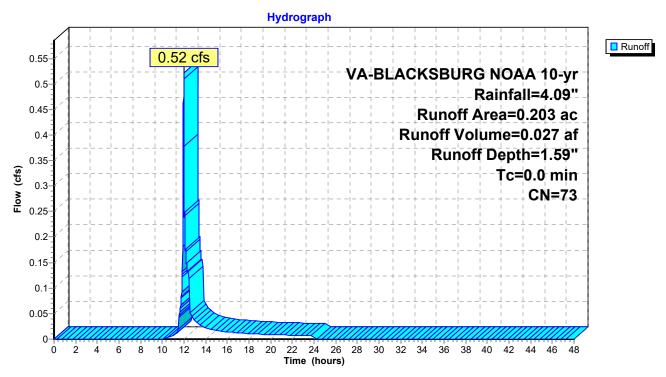
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff 0.52 cfs @ 11.99 hrs, Volume= 0.027 af, Depth= 1.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

	Area (ac)	CN	Description
	0.137	61	>75% Grass cover, Good, HSG B
*	0.066	98	Paved parking, HSG B
	0.203	73	Weighted Average
	0.137		67.49% Pervious Area
	0.066		32.51% Impervious Area

Subcatchment 3S: DIRECT RUNOFF



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 27

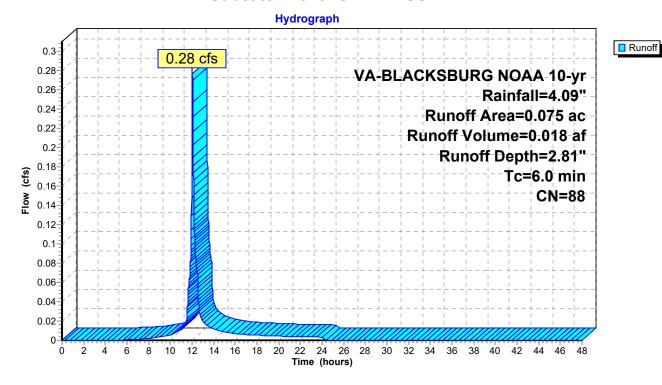
Summary for Subcatchment 4S: BYPASS AREA

Runoff = 0.28 cfs @ 12.04 hrs, Volume= 0.018 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 10-yr Rainfall=4.09"

Area	(ac)	CN	Desc	Description					
0.	.021	61	>75%	√ Grass co	over, Good,	d, HSG B			
0	.054	98	Pave	ed parking,	HSG B				
0.	0.075 88 Weighted Average				age				
0.	0.021 28.00% Pervious Area								
0.	0.054			0% Imperv	rious Area				
Тс	Lengt	h S	Slope	Velocity	Capacity	Description			
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)				
6.0						Direct Entry, DIRECT			

Subcatchment 4S: BYPASS AREA



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 28

Summary for Pond 1P: UNDERGROUND DETENTION SYSTEM

Inflow Area = 4.657 ac, 41.34% Impervious, Inflow Depth = 1.81" for 10-yr event

Inflow = 10.90 cfs @ 12.04 hrs, Volume= 0.702 af

Outflow = 3.59 cfs @ 12.39 hrs, Volume= 0.702 af, Atten= 67%, Lag= 21.0 min

Primary = 3.59 cfs @ 12.39 hrs, Volume= 0.702 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 2,206.79' @ 12.39 hrs Surf.Area= 0.099 ac Storage= 0.222 af

Plug-Flow detention time= 163.1 min calculated for 0.701 af (100% of inflow)

Center-of-Mass det. time= 163.2 min (1,009.1 - 846.0)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,202.35'	0.000 af	44.50'W x 97.00'L x 6.00'H Field A
			0.595 af Overall - 0.263 af Embedded = 0.332 af x 0.0% Voids
#2A	2,202.85'	0.263 af	CMP Round 60 x 30 Inside #1
			Effective Size= 60.0"W x 60.0"H => 19.63 sf x 20.00'L = 392.7 cf
			Overall Size= 60.0"W x 60.0"H x 20.00'L
			Row Length Adjustment= -10.00' x 19.63 sf x 6 rows
			42.50' Header x 19.63 sf x 1 = 834.5 cf Inside

0.263 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,202.65'	15.0" Round 15" HDPE Culvert L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,202.65' / 2,202.45' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,202.75'	2.2" Vert. 2.25" Dia. Orifice C= 0.600
			Limited to weir flow at low heads
#3	Device 1	2,204.95'	12.0" W x 3.0" H Vert. 10" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#4	Device 1	2,205.45'	16.0" W x 3.0" H Vert. 18" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#5	Device 1	2,206.85'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=3.59 cfs @ 12.39 hrs HW=2,206.79' (Free Discharge)

1=15" HDPE Culvert (Passes 3.59 cfs of 11.07 cfs potential flow)

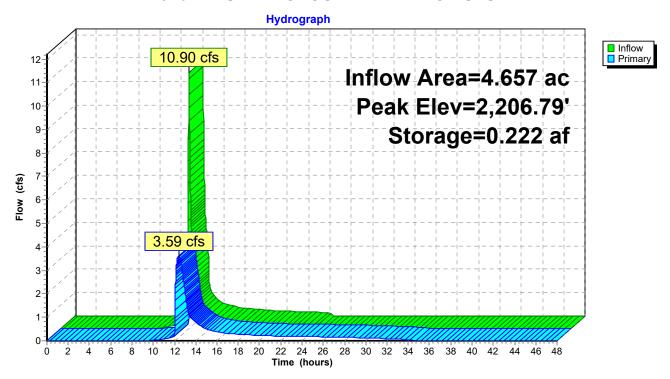
2=2.25" Dia. Orifice (Orifice Controls 0.25 cfs @ 9.56 fps)

—3=10" x 3" Rect. Orifice (Orifice Controls 1.57 cfs @ 6.30 fps)

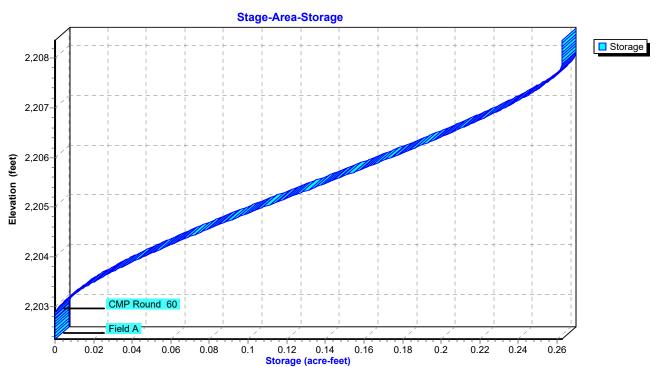
-4=18" x 3" Rect. Orifice (Orifice Controls 1.77 cfs @ 5.30 fps)

-5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 1P: UNDERGROUND DETENTION SYSTEM



Pond 1P: UNDERGROUND DETENTION SYSTEM



Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 30

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 1.96" for 10-yr event

Inflow = 1.86 cfs @ 12.04 hrs, Volume= 0.118 af

Outflow = 0.45 cfs @ 12.56 hrs, Volume= 0.118 af, Atten= 76%, Lag= 31.0 min

Primary = 0.45 cfs @ 12.56 hrs, Volume= 0.118 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 2,207.01' @ 12.56 hrs Surf.Area= 0.032 ac Storage= 0.050 af

Plug-Flow detention time= 325.9 min calculated for 0.118 af (100% of inflow)

Center-of-Mass det. time= 326.0 min (1,166.2 - 840.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
	·	0.055 (T () A ()) O (

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.78'	15.0" Round 15" Culvert Extension L= 133.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.78' / 2,202.67' S= 0.0083 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,203.96'	15.0" Round 15" Culvert
			L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#3	Device 2	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	2,206.44'	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=0.45 cfs @ 12.56 hrs HW=2,207.01' (Free Discharge)

—1=15" Culvert Extension (Passes 0.45 cfs of 8.06 cfs potential flow)

2=15" Culvert (Passes 0.45 cfs of 8.95 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.05 cfs @ 8.29 fps)

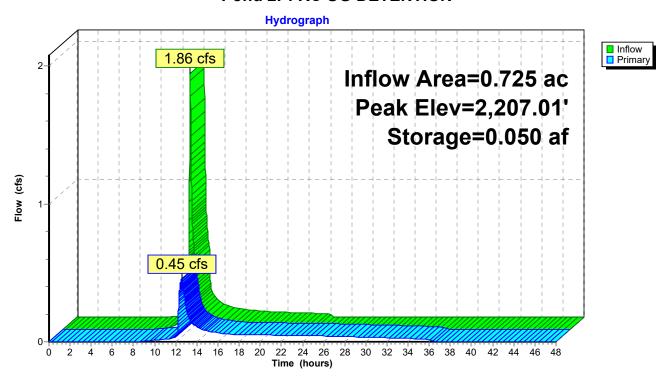
-4=Orifice/Grate (Orifice Controls 0.40 cfs @ 2.91 fps)

-5=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

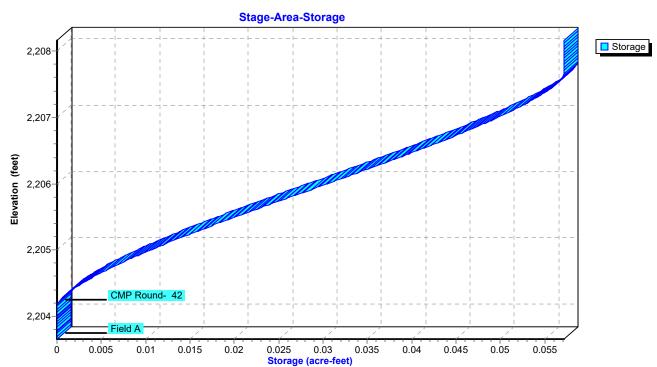
Page 31

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



Page 32

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Summary for Link 1L: POA

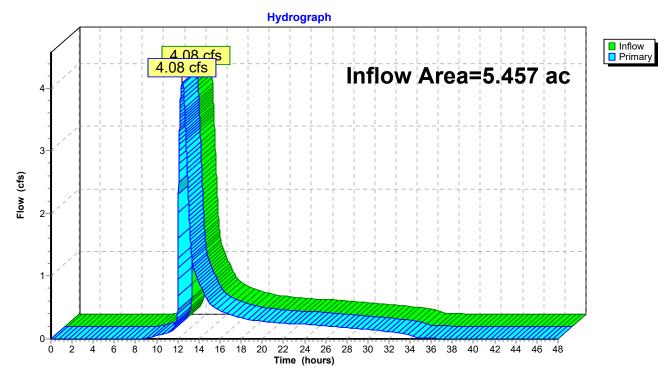
Inflow Area = 5.457 ac, 42.29% Impervious, Inflow Depth = 1.84" for 10-yr event

Inflow = 4.08 cfs @ 12.45 hrs, Volume= 0.837 af

Primary = 4.08 cfs @ 12.45 hrs, Volume= 0.837 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



POST DEV

VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Prepared by Balzer & Associates, Inc.
HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Printed 5/25/2022

Page 33

Time span=0.00-48.00 hrs, dt=0.01 hrs, 4801 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: DA TO UG Runoff Area=4.657 ac 41.34% Impervious Runoff Depth=3.80"

Tc=6.0 min CN=76 Runoff=19.78 cfs 1.474 af

Subcatchment 2S: DA TO RATCLIFF Runoff Area=0.725 ac 45.34% Impervious Runoff Depth=4.01"

Tc=6.0 min CN=78 Runoff=3.25 cfs 0.242 af

Subcatchment 3S: DIRECT RUNOFF Runoff Area=0.203 ac 32.51% Impervious Runoff Depth=3.49"

Tc=0.0 min CN=73 Runoff=0.93 cfs 0.059 af

Subcatchment 4S: BYPASS AREA Runoff Area = 0.075 ac 72.00% Impervious Runoff Depth = 5.09"

Tc=6.0 min CN=88 Runoff=0.41 cfs 0.032 af

Pond 1P: UNDERGROUND DETENTION | Peak Elev=2,216.57' | Storage=0.263 af | Inflow=19.78 cfs 1.474 af

Outflow=21.55 cfs 1.474 af

Pond 2P: RC UG DETENTION Peak Elev=2,207.47' Storage=0.056 af Inflow=3.25 cfs 0.242 af

Outflow=3.16 cfs 0.242 af

Link 1L: POA Inflow=24.96 cfs 1.748 af

Primary=24.96 cfs 1.748 af

Total Runoff Area = 5.660 ac Runoff Volume = 1.807 af Average Runoff Depth = 3.83" 58.06% Pervious = 3.286 ac 41.94% Impervious = 2.374 ac

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 34

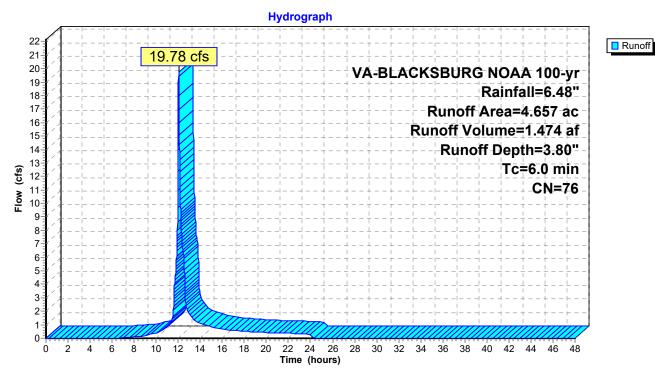
Summary for Subcatchment 1S: DA TO UG

Runoff = 19.78 cfs @ 12.04 hrs, Volume= 1.474 af, Depth= 3.80"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Area	(ac)	CN	Desc	Description			
2	.732	61	>75%	√ Grass co	over, Good,	, HSG B	
1	.925	98	Pave	ed parking,	HSG B		
4	.657	76	Weig	hted Aver	age		
2	.732		58.6	6% Pervio	us Area		
1	.925		41.3	4% Imperv	rious Area		
Tc	Lengt	th :	Slope	Velocity	Capacity	Description	
(min)	(fee		(ft/ft)	(ft/sec)	(cfs)	'	
6.0	•				·	Direct Entry, DIRECT	

Subcatchment 1S: DA TO UG



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 35

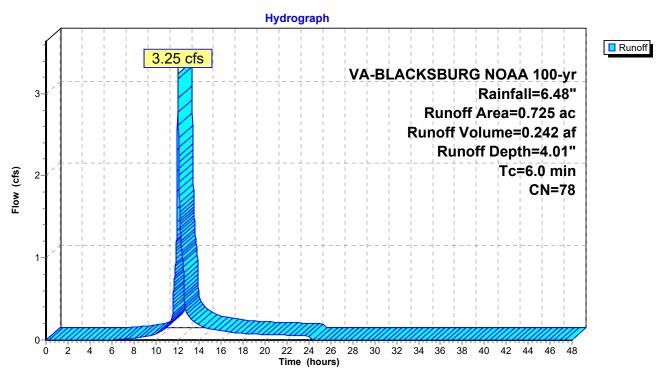
Summary for Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM

Runoff = 3.25 cfs @ 12.04 hrs, Volume= 0.242 af, Depth= 4.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Area	(ac)	CN	Desc	Description			
0.	310	61	>75%	√ Grass co	ver, Good	, HSG B	
0.	.300	98	Pave	ed parking,	HSG B		
0.	.115	70	1/2 a	cre lots, 2	5% imp, H	SG B	
0.	725	78	Weig	hted Aver	age		
0.	.396		54.60	6% Pervio	us Area		
0.	.329		45.34	4% Imperv	ious Area		
т.	1	LI_	01	\	O:h.	Description	
Tc	Leng		Slope	Velocity	Capacity	Description	
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
6.0						Direct Entry, TR-55	

Subcatchment 2S: DA TO RATCLIFF CORNER SWM SYSTEM



Printed 5/25/2022 Page 36

Summary for Subcatchment 3S: DIRECT RUNOFF

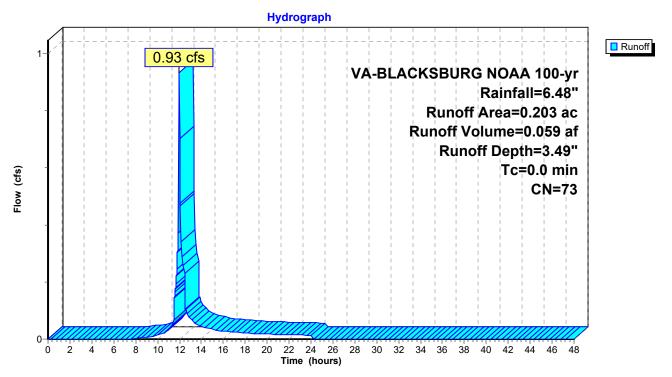
[46] Hint: Tc=0 (Instant runoff peak depends on dt)

Runoff = 0.93 cfs @ 11.99 hrs, Volume= 0.059 af, Depth= 3.49"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

	Area (ac)	CN	Description
	0.137	61	>75% Grass cover, Good, HSG B
*	0.066	98	Paved parking, HSG B
	0.203	73	Weighted Average
	0.137		67.49% Pervious Area
	0.066		32.51% Impervious Area

Subcatchment 3S: DIRECT RUNOFF



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 37

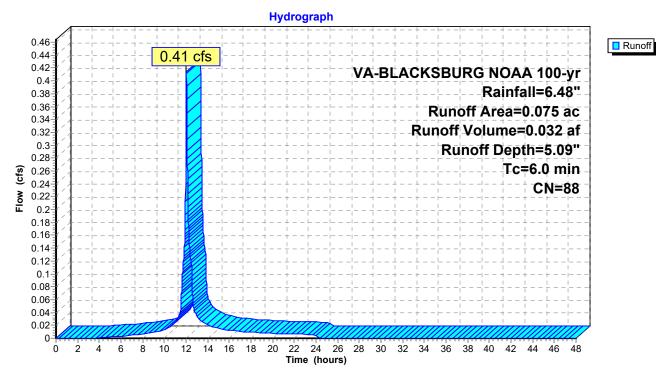
Summary for Subcatchment 4S: BYPASS AREA

Runoff = 0.41 cfs @ 12.04 hrs, Volume= 0.032 af, Depth= 5.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs VA-BLACKSBURG NOAA 100-yr Rainfall=6.48"

Area	(ac)	CN	Desc	Description			
0	.021	61	>75%	√ Grass co	over, Good,	I, HSG B	
0	.054	98	Pave	ed parking,	HSG B		
0	.075	88	Weig	hted Aver	age		
0	.021		28.0	0% Pervio	us Area		
0	.054		72.0	0% Imperv	ious Area		
Тс	Lengt	:h S	Slope	Velocity	Capacity	Description	
(min)	(fee	t)	(ft/ft)	(ft/sec)	(cfs)		
6.0						Direct Entry, DIRECT	

Subcatchment 4S: BYPASS AREA



POST DEV

Prepared by Balzer & Associates, Inc.

Printed 5/25/2022

HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 38

Summary for Pond 1P: UNDERGROUND DETENTION SYSTEM

[93] Warning: Storage range exceeded by 8.22'

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

Inflow Area = 4.657 ac, 41.34% Impervious, Inflow Depth = 3.80" for 100-yr event

Inflow = 19.78 cfs @ 12.04 hrs, Volume= 1.474 af

Outflow = 21.55 cfs @ 12.04 hrs, Volume= 1.474 af, Atten= 0%, Lag= 0.0 min

Primary = 21.55 cfs @ 12.04 hrs, Volume= 1.474 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,216.57' @ 12.04 hrs Surf.Area= 0.099 ac Storage= 0.263 af

Plug-Flow detention time= 93.1 min calculated for 1.474 af (100% of inflow)

Center-of-Mass det. time= 93.3 min (923.9 - 830.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,202.35'	0.000 af	44.50'W x 97.00'L x 6.00'H Field A
			0.595 af Overall - 0.263 af Embedded = 0.332 af x 0.0% Voids
#2A	2,202.85'	0.263 af	CMP Round 60 x 30 Inside #1
			Effective Size= 60.0"W x 60.0"H => 19.63 sf x 20.00'L = 392.7 cf
			Overall Size= 60.0"W x 60.0"H x 20.00'L
			Row Length Adjustment= -10.00' x 19.63 sf x 6 rows
			42.50' Header x 19.63 sf x 1 = 834.5 cf Inside
		0.000 5	T 4 1 A 31 1 1 C4

0.263 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,202.65'	15.0" Round 15" HDPE Culvert
			L= 10.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,202.65' / 2,202.45' S= 0.0200 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,202.75'	2.2" Vert. 2.25" Dia. Orifice C= 0.600
			Limited to weir flow at low heads
#3	Device 1	2,204.95'	12.0" W x 3.0" H Vert. 10" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#4	Device 1	2,205.45'	16.0" W x 3.0" H Vert. 18" x 3" Rect. Orifice C= 0.600
			Limited to weir flow at low heads
#5	Device 1	2,206.85'	6.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=21.54 cfs @ 12.04 hrs HW=2,216.56' (Free Discharge)

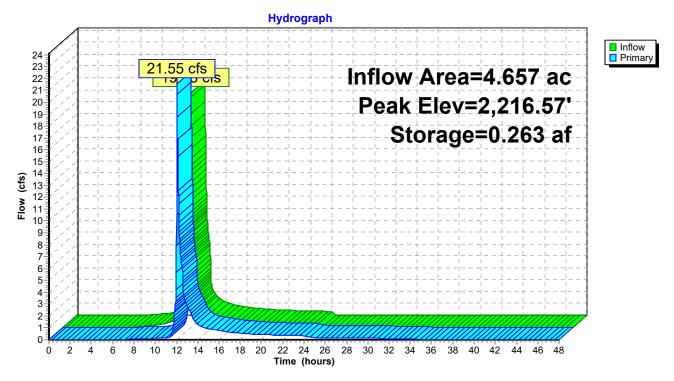
-1=15" HDPE Culvert (Inlet Controls 21.54 cfs @ 17.55 fps)
-2=2.25" Dia. Orifice (Passes < 0.47 cfs potential flow)

-3=10" x 3" Rect. Orifice (Passes < 4.08 cfs potential flow)

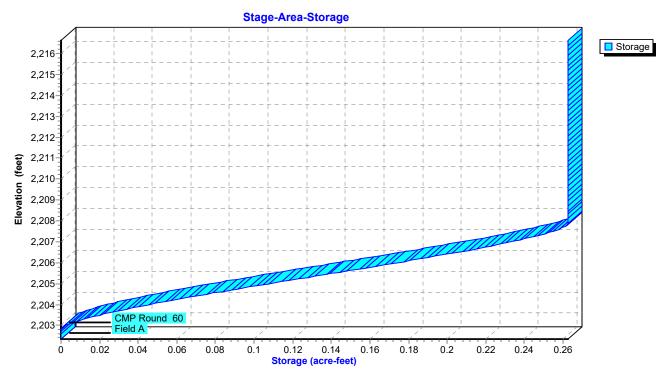
-4=18" x 3" Rect. Orifice (Passes < 5.32 cfs potential flow)

-5=Broad-Crested Rectangular Weir (Passes < 602.57 cfs potential flow)

Pond 1P: UNDERGROUND DETENTION SYSTEM



Pond 1P: UNDERGROUND DETENTION SYSTEM



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 40

Summary for Pond 2P: RC UG DETENTION

Inflow Area = 0.725 ac, 45.34% Impervious, Inflow Depth = 4.01" for 100-yr event

Inflow = 3.25 cfs @ 12.04 hrs, Volume= 0.242 af

Outflow = 3.16 cfs @ 12.06 hrs, Volume= 0.242 af, Atten= 3%, Lag= 1.0 min

Primary = 3.16 cfs @ 12.06 hrs, Volume= 0.242 af

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Peak Elev= 2,207.47' @ 12.06 hrs Surf.Area= 0.032 ac Storage= 0.056 af

Plug-Flow detention time= 195.8 min calculated for 0.242 af (100% of inflow)

Center-of-Mass det. time= 195.9 min (1,021.2 - 825.3)

Volume	Invert	Avail.Storage	Storage Description
#1A	2,203.66'	0.000 af	21.25'W x 65.50'L x 4.50'H Field A
			0.144 af Overall - 0.057 af Embedded = 0.087 af x 0.0% Voids
#2A	2,204.16'	0.057 af	CMP Round- 42 x 12 Inside #1
			Effective Size= 42.0"W x 42.0"H => 9.61 sf x 20.00'L = 192.2 cf
			Overall Size= 42.0"W x 42.0"H x 20.00'L
			12 Chambers in 4 Rows
			19.25' Header x 9.61 sf x 1 = 185.0 cf Inside
	·	0.055 (T () A ()) O (

0.057 af Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	2,203.78'	15.0" Round 15" Culvert Extension
	-		L= 133.5' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.78' / 2,202.67' S= 0.0083 '/' Cc= 0.900
			n= 0.013 Corrugated PE, smooth interior, Flow Area= 1.23 sf
#2	Device 1	2,203.96'	15.0" Round 15" Culvert
			L= 54.0' CPP, square edge headwall, Ke= 0.500
			Inlet / Outlet Invert= 2,203.96' / 2,203.78' S= 0.0033 '/' Cc= 0.900
			n= 0.011, Flow Area= 1.23 sf
#3	Device 2	2,204.00'	1.1" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#4	Device 2	,	5.0" Vert. Orifice/Grate C= 0.600 Limited to weir flow at low heads
#5	Device 2	2,207.11'	4.0' long x 0.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00
			Coef. (English) 2.80 2.92 3.08 3.30 3.32

Primary OutFlow Max=3.15 cfs @ 12.06 hrs HW=2,207.47' (Free Discharge)

-1=15" Culvert Extension (Passes 3.15 cfs of 8.64 cfs potential flow)

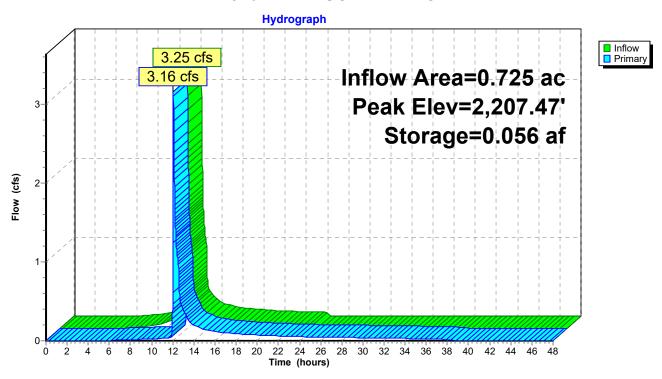
2=15" Culvert (Passes 3.15 cfs of 9.92 cfs potential flow)

-3=Orifice/Grate (Orifice Controls 0.06 cfs @ 8.91 fps)

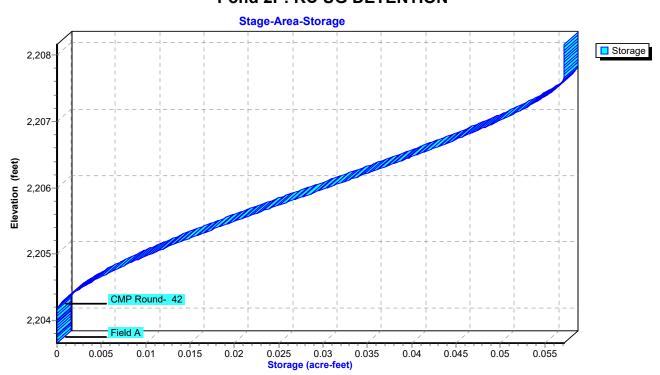
-4=Orifice/Grate (Orifice Controls 0.59 cfs @ 4.36 fps)

-5=Broad-Crested Rectangular Weir (Weir Controls 2.49 cfs @ 1.74 fps)

Pond 2P: RC UG DETENTION



Pond 2P: RC UG DETENTION



HydroCAD® 10.10-4a s/n 07713 © 2020 HydroCAD Software Solutions LLC

Page 42

Summary for Link 1L: POA

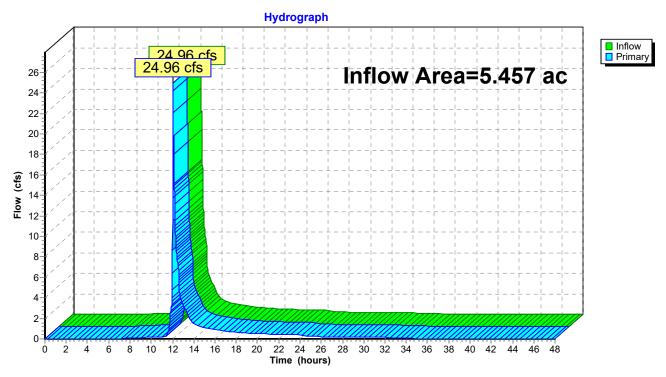
Inflow Area = 5.457 ac, 42.29% Impervious, Inflow Depth = 3.84" for 100-yr event

Inflow = 24.96 cfs @ 12.04 hrs, Volume= 1.748 af

Primary = 24.96 cfs @ 12.04 hrs, Volume= 1.748 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.01 hrs

Link 1L: POA



APPENDIX D: STORMWATER QUALITY CALCULATIONS

			Drive Townhomes 6/1/2022			CLEAR	ALL	data input cells constant values				ł
Site Information		Linear Dev	elopment Project?	No				calculation cells final results				
ite illorillation				<u> </u>	-			final results				+
ost-Development Project	(Treatme	nt Volume	and Loads)									
	-			d Area (acres) →	3.23	-		Check:				+
				reduction required:		-	BMP Design Spe	ecifications List:		aft Stds & Specs		
	_	The site's net i		ous cover (acres) is:		L	and cover areas ent	Linear project? tered correctly?	No ✓			
				tion for Site (lb/yr):	2.85	Ī		d area entered?	√			
Dra DaDavalanmant Land Caver (an												
Pre-ReDevelopment Land Cover (acr	A Soils	B Soils	C Soils	D Soils	Totals							t
Forest/Open Space (acres) undisturbed forest/open space					0.00							
Managed Turf (acres) disturbed, graded for yards or other turf to be mowed/managed		2.87			2.87							
Impervious Cover (acres)		0.36			0.36							
	1				3.23	1						Ŧ
Post-Development Land Cover (acres	s)											
Forest/Open Space (acres) undisturbed,	A Soils	B Soils	C Soils	D Soils	Totals							1
protected forest/open space or reforested land Managed Turf (acres) disturbed, graded for					0.00	<u></u>						
yards or other turf to be mowed/managed		1.41			1.41							4
Impervious Cover (acres)	6"	1.82	6"	6"	1.82	4						+
Area Check	OK.	OK.	OK.	OK.	3.23	4						1
Constants			Runoff Coefficien	te (Pu)								+
Constants Annual Rainfall (inches)	43			A Soils	B Soils	C Soils	D Soils					+
Target Rainfall Event (inches) Total Phosphorus (TP) EMC (mg/L)	1.00 0.26		Forest/Open Space Managed Turf	0.02 0.15	0.03	0.04	0.05 0.25					l
Total Nitrogen (TN) EMC (mg/L) Target TP Load (lb/acre/yr)	1.86 0.41		Impervious Cover	0.95	0.95	0.95	0.95					Ŧ
Pj (unitless correction factor)	0.90		***************************************									1
LAND COVER SUMMARY F	RE-REDEVE	LOPMENT				LAND COVER	R SUMMARY PO	OST DEVELO	PME	IT		1
Land Cover Sumn	nary-Pre			Land Cover Summa	ary-Post (Final)		Land Cover Sun	nmary-Post		Land Cover Summ	nary-Post	+
Pre-ReDevelopment	Listed	Adjusted ¹		Post ReDev. & Ne	w Impervious		Post-ReDeve	lopment		Post-Development Ne	w Impervious	
Forest/Open Space Cover (acres)	0.00	0.00		Forest/Open Space Cover (acres)	0.00		Forest/Open Space Cover (acres)	0.00				
Weighted Rv(forest) % Forest	0.00	0.00		Weighted Rv(forest) % Forest	0.00		Weighted Rv(forest) % Forest	0.00				ı
Managed Turf Cover (acres)	2.87	1.41		Managed Turf Cover	1.41		Managed Turf Cover	1.41				i
Weighted Rv(turf)	0.20	0.20		(acres) Weighted Rv (turf)	0.20	1	(acres) Weighted Rv (turf)	0.20				
% Managed Turf	89%	80%		% Managed Turf	44%		% Managed Turf	80%				
	0.36	0.36			1.82		ReDev. Impervious	0.36		New Impervious Cover	1.46	388
Impervious Cover (acres)				Impervious Cover (acres)			Cover (acres)			(acres)		1
Rv(impervious) % Impervious	0.95 11%	0.95 20%		Rv(impervious) % Impervious	0.95 56%	4	Rv(impervious) % Impervious	0.95 20%		Rv(impervious)	0.95	
Total Site Area (acres)	3.23	1.77		Final Site Area (acres)	3.23		Total ReDev. Site Area (acres)	1.77				
Site Rv	0.28	0.35		Final Post Dev Site Rv	0.62		ReDev Site Rv	0.35				
Treatment Volume an	d Nutrient Lo	nad				Treat	ment Volume and	Nutrient Loa	ıd			202
Treatment volume an	u wati citi Le	744]		Treatment Loa	iu			
Pre-ReDevelopment Treatment Volume (acre-ft)	0.0763	0.0520		Final Post-Development Treatment Volume	0.1676		Post-ReDevelopment Treatment Volume	0.0520		Post-Development Treatment Volume	0.1156	
(acte it)				(acre-ft)			(acre-ft)			(acre-ft)		
				Final Post-Development			(acre-ft) Post-ReDevelopment			Post-Development		-
Pre-ReDevelopment Treatment Volume (cubic feet)	3,325	2,265			7,300		(acre-ft)	2,265		(acre-ft) Post-Development Treatment Volume (cubic feet)	5,035	
Pre-ReDevelopment Treatment Volume	3,325	2,265		Final Post-Development Treatment Volume	7,300		(acre-ft) Post-ReDevelopment Treatment Volume			Post-Development Treatment Volume (cubic	5,035	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load				Final Post-Development Treatment Volume (cubic feet)			Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment	2,265		Post-Development Treatment Volume (cubic feet) Post-Development TP		
Pre-ReDevelopment Treatment Volume (cubic feet)	3,325	2,265		Final Post-Development Treatment Volume (cubic feet)	7,300 4.59	_	(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet)			Post-Development Treatment Volume (cubic feet)	5,035 3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per ace	2.09	1.42		Final Post-Development Treatment Volume (cubic feet) Final Post- Development TP Load (lb/yr) Final Post-Development TP	4.59		(acre-ft)	2,265		Post-Development Treatment Volume (cubic feet) Post-Development TP		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr)				Final Post-Development Treatment Volume (cubic feet) Final Post- Development TP Load (lb/yr)			Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment Load (TP) (lb/yr)*	2,265		Post-Development Treatment Volume (cubic feet) Post-Development TP		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr)	2.09	0.80		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59	_	Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment Load (TP) (lb/yr)* Post-ReDevelopment TP Load per acre (lb/acre/yr) Max. Reduction Required	2,265 1.42		Post-Development Treatment Volume (cubic feet) Post-Development TP		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr)	2.09 0.65 a excluding pervious	1.42		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59	-	Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment Load (TP) (lb/yr)* Post-ReDevelopment TP Load per acre (lb/acre/yr)	2,265		Post-Development Treatment Volume (cubic feet) Post-Development TP		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area	2.09 0.65 a excluding pervious	0.80		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59		(acre-ft) Post-ReDevelopment Trestment Volume (cubic feet) Post-ReDevelopment Load (TP) (lb/yr)* Post-ReDevelopment TP Load per acre (lb/acre/yr) Max. Reduction Required (felow Ye-	2,265 1.42		Post-Development Treatment Volume (cubic feet) Post-Development TP		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious co	2.09 0.65 a excluding pervious	0.80		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment Load (TP) (IB/yr)² Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction	2,265 1.42		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr)		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious co	2.09 0.65 a excluding pervious over)	0.80		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment Load (TP) ((b/yr)* Post-ReDevelopment TP Load feet) ((b/acre/yr) Max. Reduction Required ((blow Fre- ReDevelopment Load)	2,265 1.42		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New		
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious co	2.09 0.65 a excluding pervious over)	0.80 0.73 0.73		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment Load (TP) ((b/yr)* Post-ReDevelopment TP Load graze ((b/acre/yr) Max. Reduction Required (ReDevelopment Load) TP Load Reduction Required for	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (tb/yr) TP Load Reduction	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious co	2.09 0.65 a excluding pervious over)	0.80 0.73 0.73		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per are (lb/scre/yr) Baseline TP Load (lb/yr) (0.41 lbs/scre/yr applied to per-redevelopment are land proposed for new impervious collections of the land proposed for new impervious collections of the land proposed for new impervious collections of the land land collection of the land land land land land land land land	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed		Final Post-Development Treatment Volume (cubic feet) Final Post- Development TP Load (Ib/lyr) Final Post-Development TP Load (Ib/lyr) Final Post-Development TP Load per acre (Ib/lacer/yr)	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed		Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load per acre	4.59	Site Area	(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed	Post-Dev	Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lib/yr) Final Post-Development TP Load per acre (lb/scre/yr)	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed	Post-Dev	Final Post-Development Treatment Volume (cubic feet) Final Post- Development TP Load (Ib/lyr) Final Post-Development TP Load (Ib/lyr) Final Post-Development TP Load per acre (Ib/lacer/yr)	4.59	Site Area	(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed	Post-Dev	Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lib/yr) Final Post-Development TP Load per acre (lb/scre/yr)	4.59		(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed	Post-Dev TP Load	Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lb/yr) Final Post-Development TP Load ger alore (lb/scse/yr) Final Post-Development TP Load ger alore (lb/scse/yr)	4.59 1.42 iirement for (lb/yr)	2.85	(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	
Pre-ReDevelopment Treatment Volume (cubic feet) Pre-ReDevelopment TP Load (lb/yr) Pre-ReDevelopment TP Load per acre (lb/acre/yr) Baseline TP Load (lb/yr) (0.41 lbs/acre/yr applied to pre-redevelopment area land proposed for new impervious core. Adjusted Land Cover Summary: Pre ReDevelopment land cover minus pervious land proposed for new impervious cover. Adjusted total acreage is consistent with Post-ReL of new impervious coverl, adjusted total acreage is consistent with Post-ReL of new impervious coverl.	2.09 0.65 a excluding pervious over) and cover [forest/ope Development acrea	1.42 0.80 0.73 0.73 sn space or managed	Post-Dev TP Load	Final Post-Development Treatment Volume (cubic feet) Final Post-Development TP Load (lib/yr) Final Post-Development TP Load per acre (lb/scre/yr)	4.59 1.42 iirement for (lb/yr)	2.85	(acre-ft) Post-ReDevelopment Treatment Volume (cubic feet) Post-ReDevelopment To- Load (TP) (IB/yr)* Post-ReDevelopment TP Load per acre (Ib/acre/yr) Max. Reduction Required (Relow Yre- ReDevelopment Load) TP Load Reduction Required for Redeveloped Area Redeveloped Area	2,265 1.42 0.80		Post-Development Treatment Volume (cubic feet) Post-Development TP Load (lb/yr) TP Load Reduction Required for New	3.16	