

30-R AT CRC RESIDENTIAL DEVELOPMENT

Tax No: 317-3 13

PDG # 17-0117

Stormwater Management Concept Calculations For Rezoning Application

February 1, 2018

PREPARED FOR:

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PROJECT NARRATIVE

Project Description

The purpose of this project is the development of 7.03 acres of land into a Planned Residential Development for SAS Builder, Inc. The Site is located in the Town of Blacksburg along Research Center Drive across the road from multiple Knollwood Residential developments.

Existing Site Conditions

The 7.033 undeveloped site is entirely wooded. The site is bound by the research and development properties of Virginia Tech Foundation Inc. and VPI Buildings and Land to the North, Virginia Tech Corporate Research Center Inc (CRC). to the west, and Virginia Tech Corporate Research Center Inc. to the south.

The existing soil information and maps are shown in Appendix A. The existing soils include the following soils:

- 1C: Berks-Clymer complex, 7 to 15 percent slopes
- 19B: Guernsey silt loam, 2 to 7 percent slopes
- 25: McGary and Purdy Soils

Proposed Development Plans

The proposed development will include three multi-family dwellings and amenity areas. The site also proposes an access connection to CRC property and connection to the existing CRC trail network.

During construction the wooded areas will become disturbed and runoff from the site will need to be controlled by temporary erosion and sediment control measures. These measures will adhere to the set forth by the Virginia Erosion and Sediment Control Handbook.

STORMWATER SUMMARY

Pre-Development Conditions

The pre-developed site is divided into three drainage areas. Appendix B contains the pre-developed drainage map and runoff calculations. Existing conditions were evaluated using AutoCAD's Hydraflow Hydrograph's extension and utilizing the SCS method.

Area "A" drains to the front of the property towards Research Center Drive.

Area "B" drains to the southwest side of the property into a natural channel to Kraft Drive then through a manmade channel to a pond on Pratt Drive. The pond (Facility #4) on Pratt Drive is an existing water quality BMP for the CRC sized for a 30 hr drawdown of 2xWQV. The evaluation point for Area "B" is a point just prior to the culvert under Kraft drive.

Area "C" sheet flows towards/across Virginia Tech property and ultimately reaches a roadside ditch.

The following tables are a summary for the Pre-Developed Condition:

Pre-Development Land Coverage				
	Area "A"	Area "B"	Area "C"	
Coverage Type	Wooded	Wooded/Grassed	Wooded	
Soil Type	В	B/D	B/C	
Total Area	1.31	4.53	2.77	
Composite CN	55	58	60	

Pre-Development Peak Runoff Rates				
	Area "A"	Area "B"	Area "C"	
1-Year Storm	0.007 cfs	0.061 cfs	0.095 cfs	
2-Year Storm	0.042 cfs	0.40 cfs	0.58 cfs	
10-Year Storm	0.77 cfs	3.23 cfs	3.23 cfs	
25-Year Storm	1.59 cfs	5.97 cfs	5.5 cfs	
100-Year Storm	3.30 cfs	11.50 cfs	9.97 cfs	

Existing Facility #4 Verification (CRC Pond)

Ultimately, Area "B" reaches the Facility #4. The CRC provided the Regional Stormwater Management Summary for Virginia Tech CRC Phase 1 dated June 24, 2014, which included information for Facility #4.

The following data from the CRC calculations were utilized to recreate/verify downstream conditions.

Table 1. SCS 24 Hour Rainfall Totals for NOAA Station Blacksburg 3 SE (44-0766)

	1 Year	2 Year	10 Year	25 Year	100 Year
24 Hour Rainfall (in)	2.28	2.76	4.11	5.00	6.54

Table 2. Curve Number Weighting Summary for Predevelopment Area 4

Land Cover	HSG Soil Group	CN	Area	Component CN (CN x A/Total Area)	ent
Pasture	В	61	21.85	41.3	E 4
Forest	В	55	2.23	3.8	ea
Pasture	С	74	3.00	6.9	S &
Pasture	D	80	5.18	12.9	P. P.
			Total	64.9~65	•

Predevelopment Storm Runoff - Area 4

TOTAL DRAINAGE AREA 32.26 Acres

TIME OF CONCENTRATION

	c' Factor	Length	Slope	Tc (min)
Overland Flow	0.3	100	3.0%	10.1
	1	Length	Relief	Tc (min)
Channel Flow		538	34	6.0
	1	Length	Relief	Tc (min)
Channel Flow		175	6	1.6
	1	Length	Relief	
		Lengin	Relief	Ic (min)
Channel Flow		523	16	3.9
Channel Flow				Tc (min) 3.9
Channel Flow		523	16	3.9
_		523 Length	16 Relief	3.9 Tc (min)

Total Time of Concentration 25.0 minutes (see drainage map)

Table 3. Peak flow for Predevelopment Area 4

Return Period	Peak Flow (cfs)	¥
A. HELF EVEN		je je
1-Year	3.09	0 pd
2-Year	7.56	Fe Se
10-Year	26.08	- g ^
25-Year	41.12	<u>7</u>
100-Year	70.41	

Table 4. Curve Number Weighting Summary for Postdevelopment Area 4

Land Cover	HSG Soil Group	CN	Area	Component CN (CN x A/Total Area)	+
Pasture	В	61	11.11	21.0	nent
Forest	В	55	2.09	3.6	ng 4
Pasture	С	74	1.50	3.4	evelo
Pasture	D	80	3.72	9.2	Ae
Impervious	N/A	98	13.84	42.0	Postdevelop Area 4
		8	Total	79.2~79	3

Postdevelopment Storm Runoff - Area 4

TOTAL DRAINAGE AREA

32.26 Acres

TIME OF CONCENTRATION

	c' Factor	Length	Slope	Tc (min)	
Overland Flow	0.3	100	3.0%	10.1]
	1	Length	Relief	Tc (min)	1
Channel Flow		544	32	6.3	1
	ĺ	Length	Relief	Tc (min)	1
Channel Flow		430	16	3.1	
	1	Length	Relief	Tc (min)	1
Channel Flow		76	1	0.6	*Pipe Flow
	1	Length	Relief	Tc (min)	1
Channel Flow		398	18	2.7]
	1	Length	Relief	Tc (min)	1
Channel Flow		173	6	1.6	1

Total Time of Concentration_ (see drainage map) 24.5 minutes

Outlet Structure Configuration

Stage 1: Circular Orifice

Invert Elevation = 2101.9 feet Diameter = 0.25 feet Discharge Coefficient = 0.6

Stage 2: Grate Inlet

Crest Elevation = 2109.7 feet Effective Perimeter = 9 feet Effective Flow Area = 4.5 square feet

Stage 3: Outfall Culvert

Invert Elevation = 2101.77 feet
Pipe Diameter = 3 feet
Pipe Length = 150.69 feet
Pipe Slope = 0.03915 ft/ft
Manning n = 0.013
Entrance Condition = SEH
Number of Barrels = 1

Basin Storage/Elevation Input

Elevation (ft)	Storage (acre-ft)
2101.90	0.0000
2102.00	0.0000
2104.00	0.0430
2106.00	0.2330
2108.00	0.6540
2110.00	1.3900
2112.00	2.5370
2114.00	4.0350

Table 5. Peak flow and Routed Flow Summary for Postdevelopment Area 4

Return Period	Peak Inflow (cfs)	Routed Flow (cfs)	Predevelopment Peak Flow (cfs)	Target Met?	rea 4
1-Year	17.15	0.65	3.09	Yes	4
2-Year	25.84	4.37	7.56	Yes	de
10-Year	53.77	25.54	26.08	Yes	Postdev
25-Year	73.51	34.37	41.12	Yes	-
100-Year	108.76	45.55	70.41	Yes	

Recreated hydrographs and information can be found in Appendix C. Results from the recreated hydrographs were compared to the 2015 calculation package. See summary below compared to Table 5 above.

Verification of Peak Flow and Routed Flow for Post Development Area 4					
	2015 Peak Inflow	2015 Routed Flow			
1-Year Storm	20.21 cfs	0.65 cfs			
2-Year Storm	30.71 cfs	3.87 cfs			
10-Year Storm	64.15 cfs	34.79 cfs			
25-Year Storm	87.99 cfs	42.70 cfs			
100-Year Storm	130.63 cfs	56.50 cfs			

Calculations show differences in the peak inflows and routed flows. However, WSEL elevations for the storm events were comparable. When trying to maintain the hydraulics of Facility #4 in the 2018 Post Developed condition, the routed flows from our software will be utilized.

Post-Development Conditions

The post-developed site is divided into three drainage areas. Appendix D contains the post-developed drainage map and runoff calculations for the three drainage areas. Proposed conditions were evaluated using AutoCAD's Hydraflow Hydrograph's extension and utilizing the SCS method. Min Tc of 6 minutes was used except for Area "C". Tc for Area "C" is also included in the Appendix.

The proposed development will place a maximum of 5.5 acres of impervious area as part of the development. Drainage areas also take into consideration a planned CRC building expansion.

Area "A" from the existing condition is being incorporated into Area "B (to UG Det)".

Area "B (DA to Det Pond @ Kraft) drains to the southwest side of the property into the swale.

Area "C" sheet flows towards/across Virginia Tech property and ultimately reaches a roadside ditch.

The following tables are a summary for the Post-Developed Condition:

Post-Development Land Coverage						
	Area "B (to UG	Area "B (to Pond @	Area "C"			
	Det)"	Kraft"				
Coverage Type	Grassed/Impervious	Grassed/Impervious	Wooded/Grass/Impervious			
Soil Type	В	B/D	B/C			
Total Area	5.35	2.39	0.83			
Composite CN	96	85	68			

Post-Development Peak Runoff Rates				
	Area "B (to UG Det)"	Area "B (to Pond @ Kraft"	Area "C"	
1-Year Storm	15.43 cfs	4.07 cfs	0.28 cfs	
2-Year Storm	19.07 cfs	5.59 cfs	0.58 cfs	
10-Year Storm	29.20 cfs	10.15 cfs	1.65 cfs	
25-Year Storm	35.82 cfs	13.21 cfs	2.47 cfs	
100-Year Storm	47.22 cfs	18.53 cfs	4.0 cfs	

Quantity Considerations

Due to the increase in peak runoff rates, measures are needed to control stormwater. The plan is install an ADS Storm Tech Chamber system with 60,000 cf of storage on site and an above ground detention pond between the southwest corner of the site and Kraft Drive in the existing drainage swale. There is potential for wetlands in the existing drainage swale and is in the process of being evaluated. Should wetlands be present and the disturbance limit for wetlands is hit other options will be evaluated and/or wetlands will be mitigated.

The Storm Tech Chamber system will collect the majority of the site, detain and discharge to the Pond at Kraft. The Pond at Kraft will collect some additional area from the site and area between the CRC and the site. A potential building addition for the CRC was also taken into consideration.

The evaluation point is the outfall of the Pond at Kraft. Routing calculations between the two ponds show that the 10-year discharge from our site is being met at the outfall point of the Kraft Pond.

The following table is a summary for the Post-Developed Routed Condition:

	J			
Summary of Post-Development Routed Peak Flow at Evaluation Point				
	Existing Area "B"	Inflow to Pond	Routed Flow	
		@ Kraft		
1-Year Storm	0.061 cfs	4.36 cfs	0.199 cfs	
2-Year Storm	0.40 cfs	5.89 cfs	0.35 cfs	
10-Year Storm	3.23 cfs	12.26 cfs	3.23 cfs	
25-Year Storm	5.97 cfs	17.04 cfs	5.68 cfs	
100-Year Storm	11.50 cfs	24.32 cfs	16.41 cfs	

Downstream Considerations

The minimum standards of 9VAC25-70-66 have been used to address channel protection and flood protection for this site. For channel protection, discharge is to a manmade channel that drains to the CRC's Facility #4 and the existing 2-yr rate is being met. For flood protection, the existing 10-yr rate is being met.

The outfall from CRC's Facility #4 is to a natural channel. 9VAC25-70-66 requires any development to meet an energy balance equation for the 1-yr storm event. Due to the existing and proposed conditions of the site, the site is allowed a discharge of 0 cfs for the 1-yr storm event. Measures were taken onsite and at the new Kraft Drive pond to reduce the 1-yr storm event to the extend possible.

Considering that the release from the site ultimately reaches the CRC's Facility #4, routing calculations were provided for the 2018 Post Developed Condition. 2018 Post — Development Facility #4 Verification calculations can be found in Appendix F. The calculations show that the existing hydraulics of the pond are being maintained. Since the 1-yr release rate for Facility #4 is not being increase, the development meets the energy balance equation at the point of discharge to a natural channel.

The following table is a summary for the 2018 Post-Developed Routed Condition for Facility #4:

Summary of 2018 Post Development for Facility #4			
2015 Routed Flow 2018 Routed Flow			
1-Year Storm	0.65 cfs	0.63 cfs	
2-Year Storm	3.87 cfs	2.24 cfs	
10-Year Storm	34.79 cfs	32.92 cfs	
25-Year Storm	42.70 cfs	40.10 cfs	
100-Year Storm	56.50 cfs	55.60 cfs	

Quality Considerations

The VRRM spreadsheet was used to calculate the phosphorous removal requirements for the site. A max of 5.5 acres of impervious area will be placed as part of the development and a maximum disturbance of 9.5 acres. The VRRM spreadsheet shows that 9.9 lbs of phosphorous per year are required to be moved. Since the development stays under 10 lbs of phosphorous, the purchase of nutrient credits are allowed. Appendix G shows the VRRM spreadsheet and a letter of availability for the purchase of 10 lbs of phosphorous credits.

STORM WATER MANAGEMENT MAINTENANCE & INSPECTION PLAN

The Town of Blacksburg requires a formal stormwater maintenance agreement to be provided, reviewed, and recorded at the Montgomery County Courthouse. This plan will be fully developed at time of site plan. All facility inspections and any required maintenance shall be

handled solely by the owner. The following are a minimum list of requirements not representative of the full responsibility of the land owner:

- 1) The "Operation and Maintenance Checklist" located in Appendix 3B (Checklists Detention, Retention, and Impoundment BMPs) of the Virginia Stormwater Management Handbook Volume I, should be completed every six months.
- 2) All outfall structures to shall be inspected after each runoff producing storm event. These structures shall be checked for debris and sediment accumulation that could inhibit the performance of the structure.
- 3) All underground detention basins require regular maintenance and inspection. These structures shall be checked for debris, sediment accumulation, structural integrity. After runoff producing events the outfall structure of the detention facilities will be inspected for debris and sediment accumulation that could inhibit the performance of the structure.

APPENDICES

Appendix A – Soils/Rainfall Information

- o Soil Map
- o 1C Soils
- o 19B Soils
- o 25 Soils
- o NOAA Precipitation Data
- Watershed Model Schematic

Appendix B – Pre-Developed Hydrologic Conditions

- o Pre-Developed Drainage Map
- o Time of Concentration Worksheet
- o Pre-A SCS Method (1, 2, 10, 25 & 100-year storms)
- o Pre-B SCS Method (1, 2, 10, 25 & 100-year storms)
- o Pre-C SCS Method (1, 2, 10, 25 & 100-year storms)

Appendix C – Existing Facility #4 Verification

- o Pre Development (1970) SCS Method (1, 2, 10, 25 & 100-year storms)
- o Post Development (2015) SCS Method (1, 2, 10, 25 & 100-year storms)
- o Facility #4 (2015) Pond Report
- o Rte Facility #4 (2015) SCS Method (1, 2, 10, 25 & 100-year storms)

Appendix D – Post Developed Hydrologic Conditions

- Post Developed Drainage Map
- o Time of Concentration Worksheet
- o Post-B (DA to UG Det) SCS Method (1, 2, 10, 25 & 100-year storms)
- o Post-B (DA to Det Pond @ Kraft) SCS Method (1, 2, 10, 25 & 100-year storms)
- o Post-C SCS Method (1, 2, 10, 25 & 100-year storms)

Appendix E – Routing Calculations

- o UG Det (Storm Tech Chambers) Pond Report
- o Rte UG Det SCS Method (1, 2, 10, 25 & 100-year storms)
- o Post B (Total to Det Pond @ Kraft) SCS Method (1, 2, 10, 25 & 100-year storms)
- o New Det. Pond Report @ Kraft
- o Rte Det. Pond @ Kraft (1, 2, 10, 25 & 100-year storms)

Appendix F – 2018 Post Development Facility #4 Verification

- o Hyd 13 SCS Method (1, 2, 10, 25 & 100-year storms)
- o Hyd 14 SCS Method (1, 2, 10, 25 & 100-year storms)
- o Facility #4 (2015) Pond Report
- o Rte Facility #4 (2018) SCS Method (1, 2, 10, 25 & 100-year storms)

Appendix G – Virginia Runoff Reduction Calculations

- o Site Data Sheet
- o Availability Letter

APPENDIX A – SOILS/RAINFALL INFORMATION

(See attached Drawings)



MAP LEGEND

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Water Features

Transportation

Background

Spoil Area

Stony Spot

Wet Spot

Other

Rails

US Routes

Major Roads

Local Roads

Very Stony Spot

Special Line Features

Streams and Canals

Interstate Highways

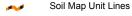
Aerial Photography

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons



Soil Map Unit Points

Special Point Features

Blowout

Borrow Pit

Clay Spot

Closed Depression

Gravel Pit

Gravelly Spot

Landfill

Lava Flow

Marsh or swamp

Mine or Quarry

Miscellaneous Water

Perennial Water

Rock Outcrop

Sandy Spot

Severely Eroded Spot

Saline Spot

Sinkhole

Slide or Slip

Sodic Spot

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:15.800.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Montgomery County, Virginia Survey Area Data: Version 10, Oct 3, 2017

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Oct 22, 2012—Feb 5, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
1C	Berks-Clymer complex, 7 to 15 percent slopes	6.2	86.2%
19B	Guernsey silt loam, 2 to 7 percent slopes	0.9	12.3%
25	McGary and Purdy soils	0.1	1.5%
Totals for Area of Interest		7.2	100.0%

Montgomery County, Virginia

1C—Berks-Clymer complex, 7 to 15 percent slopes

Map Unit Setting

National map unit symbol: kc2c Elevation: 1,700 to 3,000 feet

Mean annual precipitation: 30 to 45 inches Mean annual air temperature: 50 to 57 degrees F

Frost-free period: 117 to 185 days

Farmland classification: Not prime farmland

Map Unit Composition

Berks and similar soils: 50 percent Clymer and similar soils: 30 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Berks

Setting

Landform: Hills

Landform position (two-dimensional): Backslope, summit Landform position (three-dimensional): Side slope, interfluve

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Shale, siltstone, and sandstone residuum

Typical profile

H1 - 0 to 7 inches: channery silt loam
H2 - 7 to 23 inches: very channery silt loam
H3 - 23 to 33 inches: extremely channery silt loam

H4 - 33 to 79 inches: bedrock

Properties and qualities

Slope: 7 to 15 percent

Depth to restrictive feature: 20 to 40 inches to lithic bedrock

Natural drainage class: Well drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Very

low to high (0.00 to 5.95 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Very low (about 2.5 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Other vegetative classification: Unnamed (G128XB012VA)

Hydric soil rating: No

Description of Clymer

Setting

Landform: Hills

Landform position (two-dimensional): Backslope, summit Landform position (three-dimensional): Side slope, interfluve

Down-slope shape: Convex Across-slope shape: Convex

Parent material: Sandstone, siltstone, and shale residuum

Typical profile

H1 - 0 to 9 inches: loam H2 - 9 to 21 inches: clay loam

H3 - 21 to 32 inches: channery sandy clay loam H4 - 32 to 49 inches: very channery sandy loam

H5 - 49 to 79 inches: bedrock

Properties and qualities

Slope: 7 to 15 percent

Depth to restrictive feature: 40 to 60 inches to lithic bedrock

Natural drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Very

low (0.00 to 0.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Other vegetative classification: Unnamed (G128XB004VA)

Hydric soil rating: No

Data Source Information

Soil Survey Area: Montgomery County, Virginia Survey Area Data: Version 9, Dec 11, 2013

Montgomery County, Virginia

19B—Guernsey silt loam, 2 to 7 percent slopes

Map Unit Setting

National map unit symbol: kc2b Elevation: 1,300 to 2,200 feet

Mean annual precipitation: 30 to 45 inches Mean annual air temperature: 50 to 57 degrees F

Frost-free period: 117 to 185 days

Farmland classification: All areas are prime farmland

Map Unit Composition

Guernsey and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Guernsey

Setting

Landform: Stream terraces

Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Limestone, shale, sandstone and crystalline rock

alluvium

Typical profile

H1 - 0 to 10 inches: silt loam

H2 - 10 to 20 inches: silty clay loam

H3 - 20 to 53 inches: clay

H4 - 53 to 83 inches: silty clay loam

Properties and qualities

Slope: 2 to 7 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Moderately well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.57 in/hr)

Depth to water table: About 18 to 36 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Moderate (about 8.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 2e

Hydrologic Soil Group: C

Other vegetative classification: Unnamed (G128XY000VA)

Hydric soil rating: No

Data Source Information

Soil Survey Area: Montgomery County, Virginia Survey Area Data: Version 9, Dec 11, 2013

Montgomery County, Virginia

25—McGary and Purdy soils

Map Unit Setting

National map unit symbol: kc2k Elevation: 1,300 to 2,200 feet

Mean annual precipitation: 30 to 45 inches Mean annual air temperature: 50 to 57 degrees F

Frost-free period: 117 to 185 days

Farmland classification: Farmland of statewide importance

Map Unit Composition

Mcgary and similar soils: 40 percent Purdy and similar soils: 35 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Mcgary

Setting

Landform: Stream terraces

Landform position (three-dimensional): Tread

Down-slope shape: Concave Across-slope shape: Linear

Parent material: Limestone, shale, and sandstone alluvium

Typical profile

H1 - 0 to 9 inches: silt loam H2 - 9 to 37 inches: silty clay H3 - 37 to 79 inches: clay

Properties and qualities

Slope: 0 to 2 percent

Depth to restrictive feature: More than 80 inches Natural drainage class: Somewhat poorly drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)

Depth to water table: About 12 to 36 inches

Frequency of flooding: Occasional Frequency of ponding: None

Calcium carbonate, maximum in profile: 15 percent

Available water storage in profile: Moderate (about 8.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: D

Other vegetative classification: Unnamed (G128XY000VA)

Hydric soil rating: No

Description of Purdy

Setting

Landform: Stream terraces

Landform position (three-dimensional): Tread

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Limestone, shale, and sandstone alluvium

Typical profile

H1 - 0 to 11 inches: loam H2 - 11 to 34 inches: clay H3 - 34 to 79 inches: clay loam

Properties and qualities

Slope: 0 to 2 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Poorly drained

Runoff class: Negligible

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.20 in/hr)

Depth to water table: About 0 inches

Frequency of flooding: None Frequency of ponding: Frequent

Available water storage in profile: High (about 9.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4w

Hydrologic Soil Group: C/D

Other vegetative classification: Unnamed (G128XY000VA)

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: Montgomery County, Virginia Survey Area Data: Version 9, Dec 11, 2013



NOAA Atlas 14, Volume 2, Version 3 BLACKSBURG 3 SE Station ID: 44-0766

Location name: Blacksburg, Virginia, USA* Latitude: 37.2017°, Longitude: -80.4133° Elevation:

Elevation (station metadata): 2100 ft**

* source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

G.M. Bonnin, D. Martin, B. Lin, T. Parzybok, M.Yekta, and D. Riley

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS	OS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹									
Duration		Average recurrence interval (years)								
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.297 (0.270-0.329)	0.354 (0.321-0.393)	0.426 (0.385-0.472)	0.477 (0.429-0.527)	0.540 (0.483-0.596)	0.583 (0.518-0.646)	0.626 (0.551-0.696)	0.665 (0.580-0.743)	0.712 (0.612-0.803)	0.747 (0.633-0.848)
10-min	0.475 (0.431-0.525)	0.567 (0.514-0.628)	0.682 (0.616-0.755)	0.763 (0.687-0.843)	0.860 (0.769-0.950)	0.929 (0.825-1.03)	0.995 (0.876-1.11)	1.06 (0.919-1.18)	1.13 (0.968-1.27)	1.18 (0.997-1.34)
15-min	0.594 (0.538-0.656)	0.713 (0.646-0.790)	0.862 (0.779-0.955)	0.965 (0.869-1.07)	1.09 (0.975-1.21)	1.18 (1.04-1.30)	1.26 (1.11-1.40)	1.33 (1.16-1.49)	1.42 (1.22-1.60)	1.48 (1.25-1.68)
30-min	0.814 (0.738-0.900)	0.984 (0.892-1.09)	1.23 (1.11-1.36)	1.40 (1.26-1.55)	1.62 (1.44-1.78)	1.77 (1.57-1.96)	1.93 (1.70-2.14)	2.07 (1.81-2.31)	2.26 (1.94-2.54)	2.39 (2.03-2.72)
60-min	1.01 (0.920-1.12)	1.24 (1.12-1.37)	1.57 (1.42-1.74)	1.82 (1.64-2.01)	2.15 (1.92-2.38)	2.40 (2.13-2.66)	2.65 (2.34-2.95)	2.91 (2.53-3.25)	3.24 (2.78-3.65)	3.49 (2.96-3.96)
2-hr	1.18 (1.07-1.30)	1.43 (1.30-1.59)	1.83 (1.66-2.02)	2.13 (1.93-2.36)	2.54 (2.27-2.81)	2.86 (2.54-3.16)	3.18 (2.80-3.53)	3.50 (3.05-3.91)	3.93 (3.36-4.43)	4.26 (3.60-4.85)
3-hr	1.27 (1.15-1.40)	1.54 (1.40-1.70)	1.95 (1.77-2.15)	2.27 (2.06-2.50)	2.70 (2.43-2.97)	3.04 (2.71-3.35)	3.38 (2.99-3.75)	3.74 (3.26-4.16)	4.21 (3.61-4.74)	4.58 (3.86-5.21)
6-hr	1.55 (1.43-1.69)	1.87 (1.72-2.04)	2.34 (2.15-2.56)	2.73 (2.49-2.98)	3.26 (2.95-3.57)	3.69 (3.31-4.05)	4.14 (3.67-4.56)	4.62 (4.04-5.11)	5.29 (4.51-5.91)	5.82 (4.87-6.56)
12-hr	1.87 (1.73-2.05)	2.25 (2.08-2.46)	2.82 (2.59-3.08)	3.29 (3.01-3.59)	3.96 (3.59-4.32)	4.53 (4.06-4.95)	5.14 (4.54-5.63)	5.80 (5.03-6.40)	6.76 (5.71-7.55)	7.55 (6.24-8.52)
24-hr	2.28 (2.12-2.46)	2.76 (2.56-2.98)	3.51 (3.25-3.78)	4.11 (3.80-4.43)	5.00 (4.60-5.38)	5.74 (5.24-6.17)	6.54 (5.93-7.02)	7.40 (6.66-7.95)	8.66 (7.68-9.33)	9.70 (8.52-10.5)
2-day	2.71 (2.52-2.92)	3.28 (3.05-3.54)	4.14 (3.84-4.46)	4.84 (4.47-5.20)	5.83 (5.36-6.26)	6.65 (6.09-7.14)	7.52 (6.85-8.09)	8.46 (7.64-9.10)	9.79 (8.75-10.6)	10.9 (9.62-11.8)
3-day	2.88 (2.69-3.10)	3.48 (3.24-3.75)	4.39 (4.08-4.72)	5.11 (4.75-5.49)	6.15 (5.68-6.59)	7.00 (6.43-7.51)	7.90 (7.22-8.48)	8.86 (8.03-9.52)	10.2 (9.16-11.0)	11.4 (10.1-12.3)
4-day	3.05 (2.85-3.28)	3.69 (3.44-3.96)	4.63 (4.32-4.97)	5.39 (5.02-5.78)	6.47 (5.99-6.93)	7.35 (6.77-7.87)	8.29 (7.59-8.88)	9.27 (8.42-9.94)	10.7 (9.58-11.5)	11.8 (10.5-12.8)
7-day	3.55 (3.31-3.81)	4.27 (3.99-4.59)	5.31 (4.95-5.70)	6.13 (5.70-6.58)	7.27 (6.73-7.79)	8.19 (7.55-8.77)	9.14 (8.39-9.81)	10.1 (9.24-10.9)	11.5 (10.4-12.4)	12.6 (11.3-13.6)
10-day	4.09 (3.83-4.37)	4.91 (4.60-5.24)	6.00 (5.61-6.41)	6.85 (6.39-7.30)	7.99 (7.43-8.51)	8.87 (8.22-9.46)	9.77 (9.02-10.4)	10.7 (9.81-11.4)	11.9 (10.9-12.8)	12.9 (11.7-13.8)
20-day	5.56 (5.24-5.89)	6.61 (6.24-7.01)	7.92 (7.47-8.40)	8.94 (8.42-9.47)	10.3 (9.68-10.9)	11.4 (10.6-12.0)	12.5 (11.6-13.2)	13.5 (12.5-14.4)	15.0 (13.8-15.9)	16.1 (14.7-17.2)
30-day	6.90 (6.52-7.30)	8.17 (7.71-8.63)	9.60 (9.06-10.1)	10.7 (10.1-11.3)	12.1 (11.4-12.8)	13.2 (12.3-13.9)	14.2 (13.3-15.0)	15.2 (14.1-16.1)	16.5 (15.3-17.5)	17.5 (16.1-18.6)
45-day	8.75 (8.30-9.22)	10.3 (9.76-10.8)	11.9 (11.3-12.5)	13.1 (12.4-13.8)	14.7 (13.8-15.4)	15.8 (14.9-16.6)	16.9 (15.8-17.8)	17.9 (16.8-18.9)	19.2 (17.9-20.3)	20.1 (18.7-21.3)
60-day	10.5 (10.0-11.1)	12.3 (11.7-13.0)	14.1 (13.4-14.8)	15.3 (14.6-16.1)	16.9 (16.1-17.8)	18.1 (17.1-19.0)	19.1 (18.1-20.1)	20.1 (19.0-21.2)	21.3 (20.0-22.5)	22.1 (20.8-23.4)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

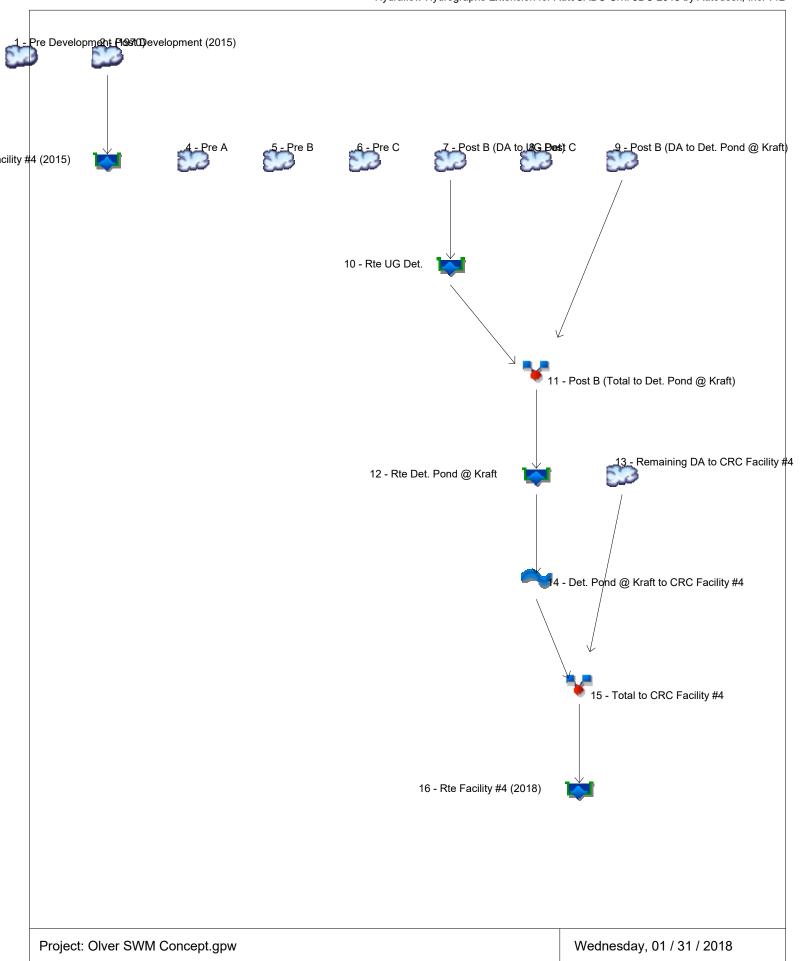
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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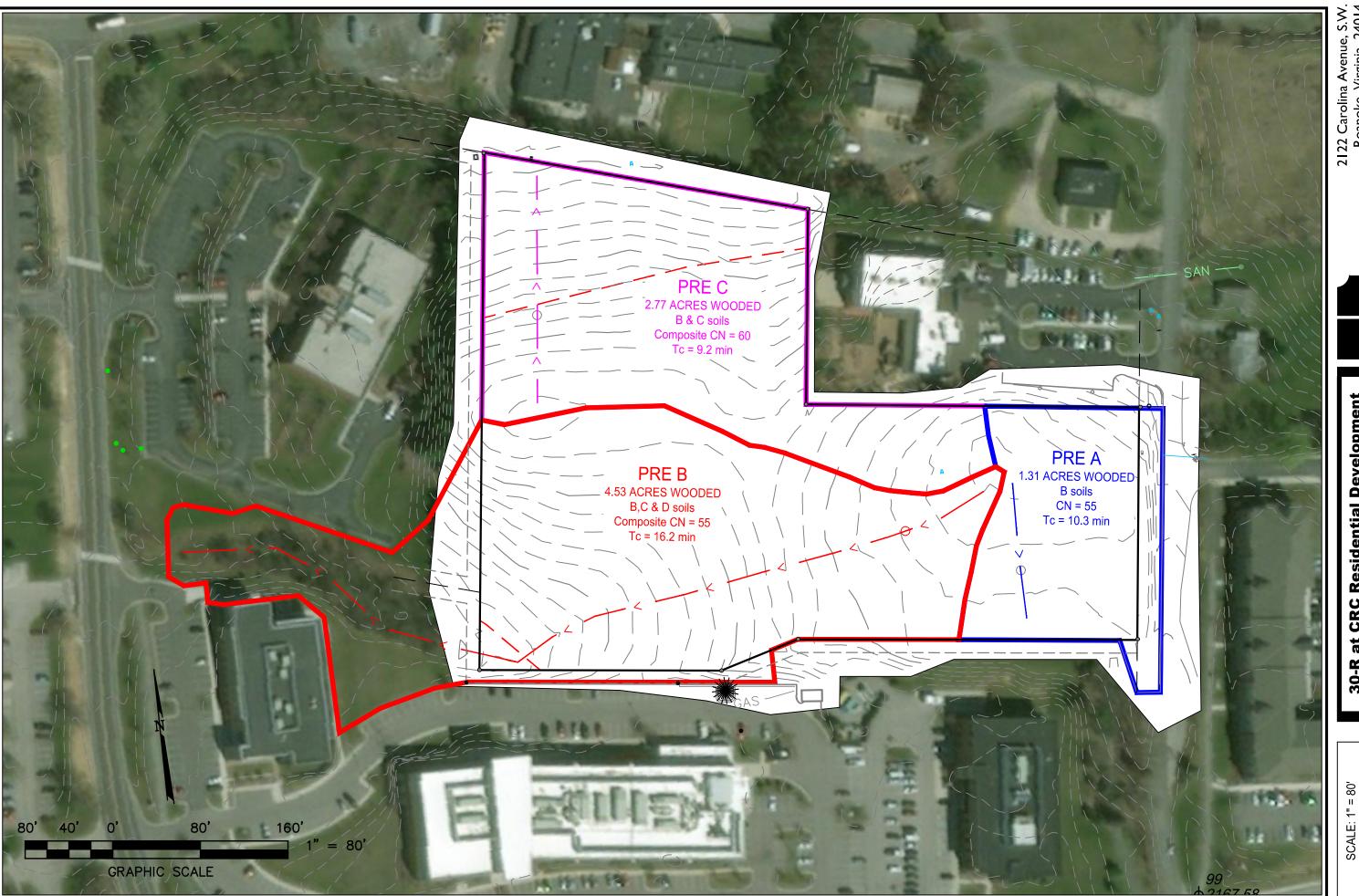
PF graphical

1 of 4 1/2/2018, 5:09 PM



APPENDIX B – PRE DEVELOPED HYDROLOGIC CONDITIONS

(See attached Drawings)



2122 Carolina Avenue, S.W.
Roanoke, Virginia 24014

Rear Phone: 540-387-1153
Fax: 540-389-5767
GROUP, INC. www.parkerdg.com

30-R at CRC Residential Development
Stormwater Concept Plan
On-Site Pre-Development Drainage Map
Town of Blacksburg, VA

SCALE: 1" = 80' PROJECT NO. 17-0117

DATE: 02/01/2018
APPENDIX C.2

Pre Development Area A

Time of Co	ncentration				
100	ft OLF@	4	% =	9.56	min
60	ft CF @	4	% =	0.69	min
0	ft CF @	0	% =	0.00	min
			Tc =	10.26	min
User define	ed				
C factor for OLF=			0.	.30	

Pre Development Area B

Time of Concentration	:			
100 ft OLF@	3 % =	10.10 min		
475 ft CF @	6.3 % =	2.76 min		
416 ft CF @	2.9 % =	3.35 min		
	Tc =	16.21 min		
User defined				
C factor for OLF=	0.30			

Pre Development Area C

Time of Concentration					
100 ft OLF@	9 % =	8.20 min			
160 ft CF @	11 % =	0.99 min			
0 ft CF @	0 % =	0.00 min			
	Tc =	9.19 min			
User defined					
C factor for OLF=	0.30				

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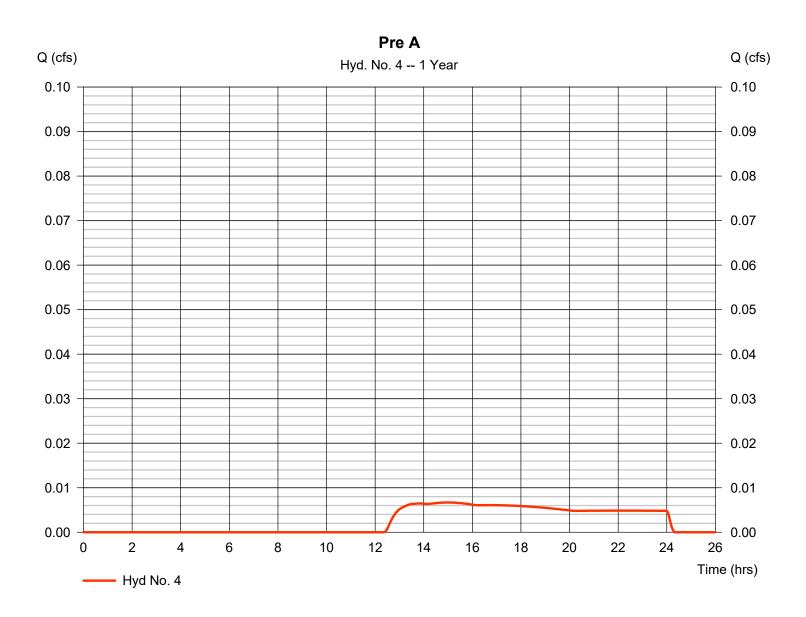
Wednesday, 01 / 31 / 2018

Hyd. No. 4

Pre A

Hydrograph type = 0.007 cfs= SCS Runoff Peak discharge Storm frequency Time to peak = 15.00 hrs= 1 yrsTime interval = 2 min Hyd. volume = 230 cuft Drainage area = 1.310 acCurve number = 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 10.30 min = User Total precip. = 2.28 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.310 x 55)] / 1.310



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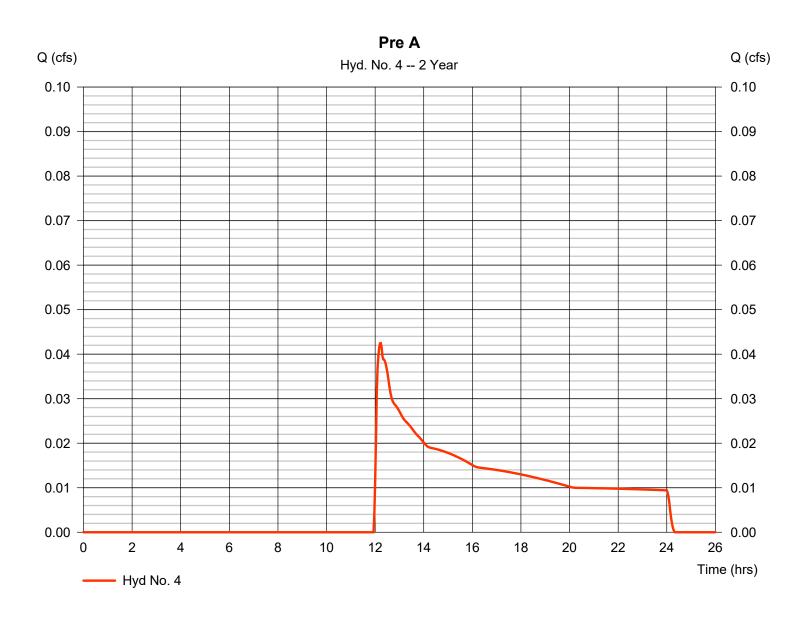
Wednesday, 01 / 31 / 2018

Hyd. No. 4

Pre A

Hydrograph type = SCS Runoff Peak discharge = 0.042 cfsStorm frequency = 2 yrsTime to peak $= 12.23 \, hrs$ Time interval = 2 min Hyd. volume = 663 cuft Drainage area = 1.310 acCurve number = 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 10.30 \, \text{min}$ Total precip. = 2.76 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.310 x 55)] / 1.310



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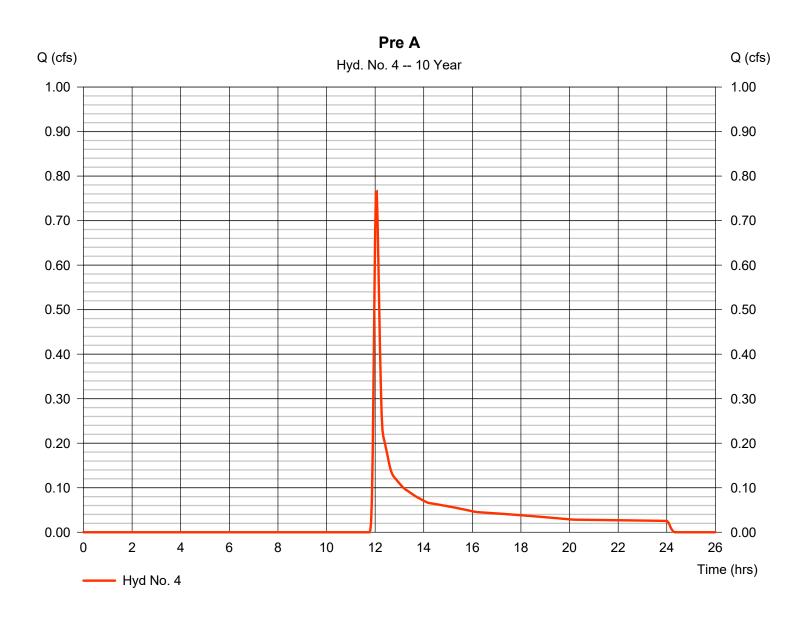
Wednesday, 01 / 31 / 2018

Hyd. No. 4

Pre A

Hydrograph type = 0.765 cfs= SCS Runoff Peak discharge Storm frequency = 10 yrsTime to peak = 12.07 hrsTime interval = 2 min Hyd. volume = 2.816 cuft Drainage area = 1.310 acCurve number = 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 10.30 min = User Total precip. = 4.11 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.310 x 55)] / 1.310



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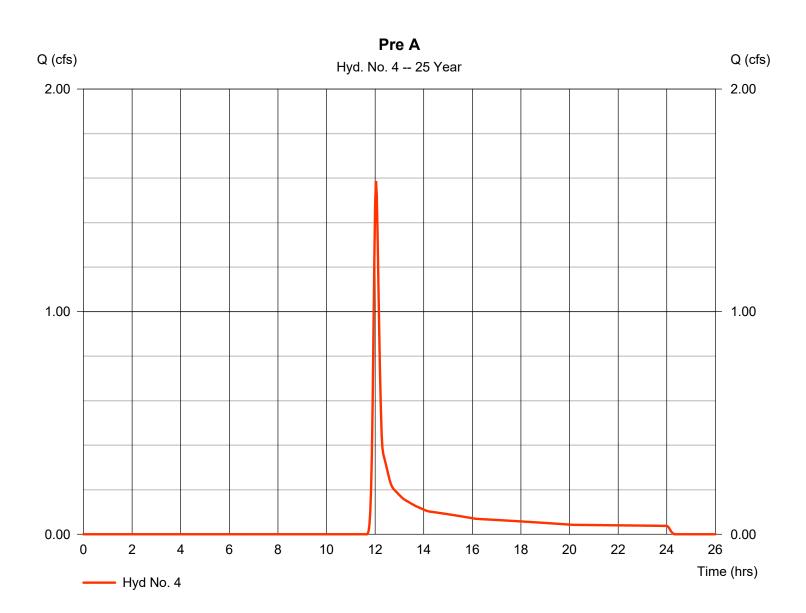
Wednesday, 01 / 31 / 2018

Hyd. No. 4

Pre A

Hydrograph type = SCS Runoff Peak discharge = 1.587 cfsStorm frequency = 25 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 4,805 cuft= 1.310 acCurve number Drainage area = 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 10.30 min = User Total precip. = 5.00 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.310 x 55)] / 1.310



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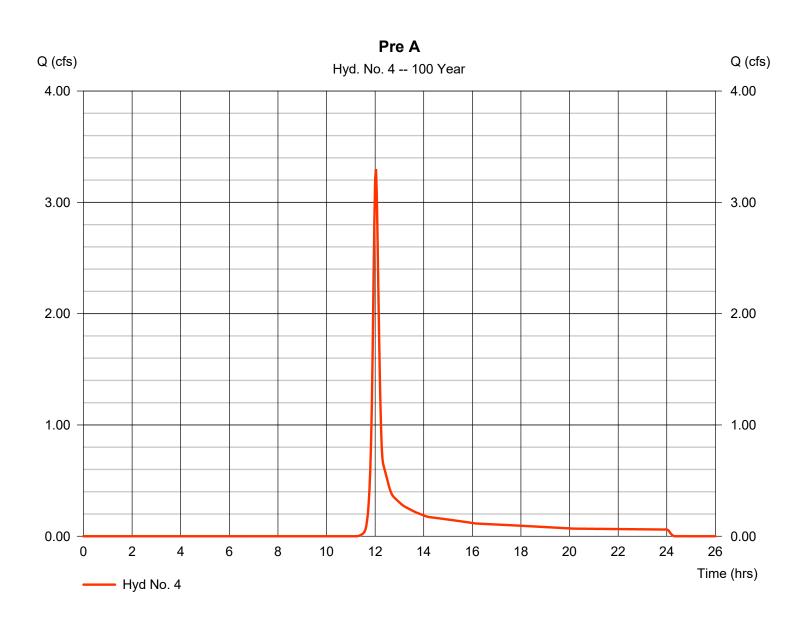
Wednesday, 01 / 31 / 2018

Hyd. No. 4

Pre A

Hydrograph type = SCS Runoff Peak discharge = 3.298 cfsStorm frequency = 100 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 9.011 cuft = 1.310 acCurve number Drainage area = 55* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 10.30 min = User Total precip. = 6.54 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.310 x 55)] / 1.310



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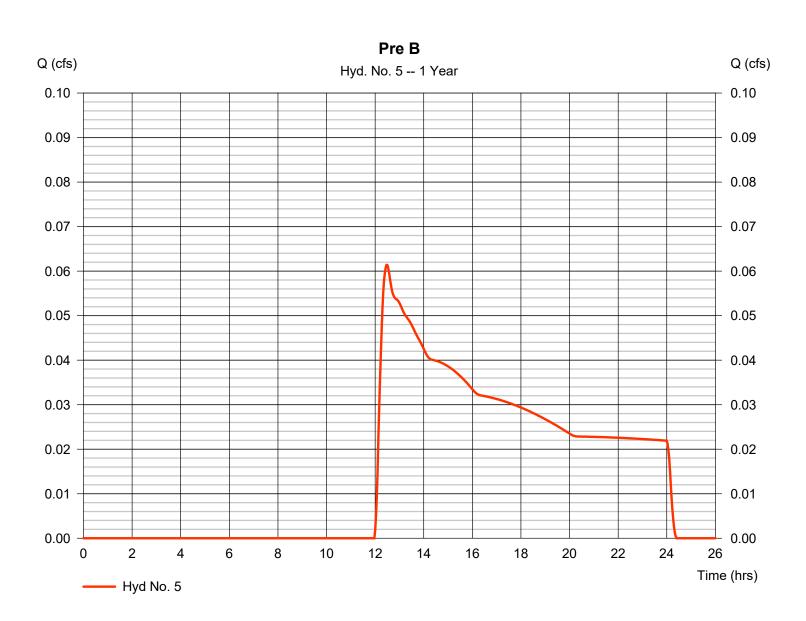
Wednesday, 01 / 31 / 2018

Hyd. No. 5

Pre B

Hydrograph type = SCS Runoff Peak discharge = 0.061 cfsStorm frequency Time to peak $= 12.47 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 1,372 cuftCurve number Drainage area = 4.530 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 16.20 \, \text{min}$ Total precip. = 2.28 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484

^{*} Composite (Area/CN) = $[(3.480 \times 55) + (0.550 \times 77) + (0.500 \times 61)] / 4.530$



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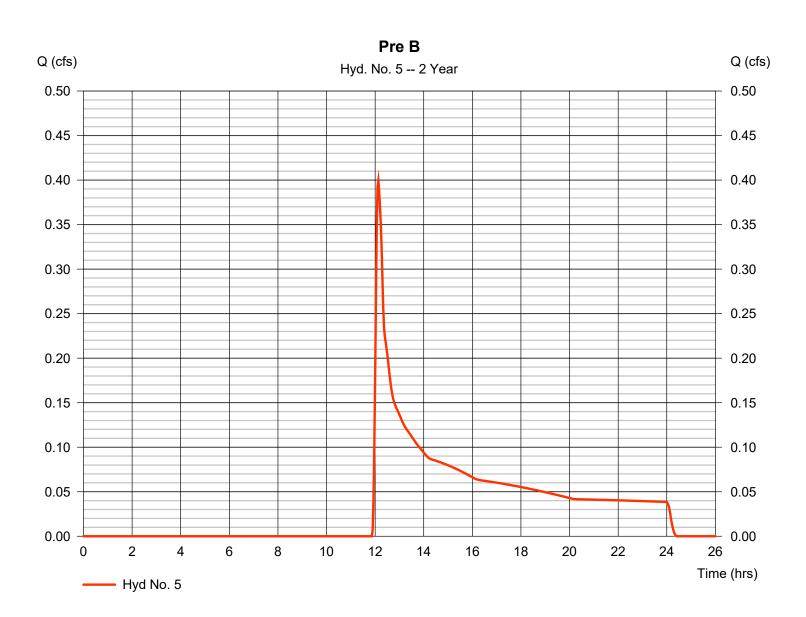
Wednesday, 01 / 31 / 2018

Hyd. No. 5

Pre B

Hydrograph type = SCS Runoff Peak discharge = 0.400 cfsStorm frequency = 2 yrsTime to peak $= 12.13 \, hrs$ Time interval = 2 min Hyd. volume = 3.224 cuft Curve number Drainage area = 4.530 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 16.20 \, \text{min}$ Total precip. = 2.76 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(3.480 \times 55) + (0.550 \times 77) + (0.500 \times 61)] / 4.530$



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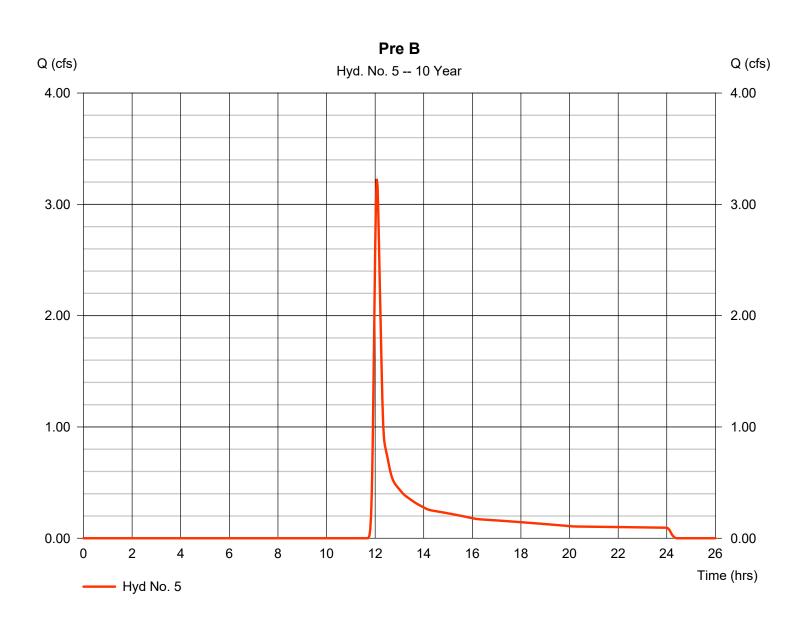
Hyd. No. 5

Pre B

Hydrograph type = SCS Runoff Peak discharge = 3.229 cfsStorm frequency = 10 yrsTime to peak = 12.07 hrsTime interval = 2 min Hyd. volume = 11,470 cuftCurve number Drainage area = 4.530 ac= 58* Basin Slope = 0.0 %Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 16.20 min
Total precip. = 4.11 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(3.480 \times 55) + (0.550 \times 77) + (0.500 \times 61)] / 4.530$



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Hyd. No. 5

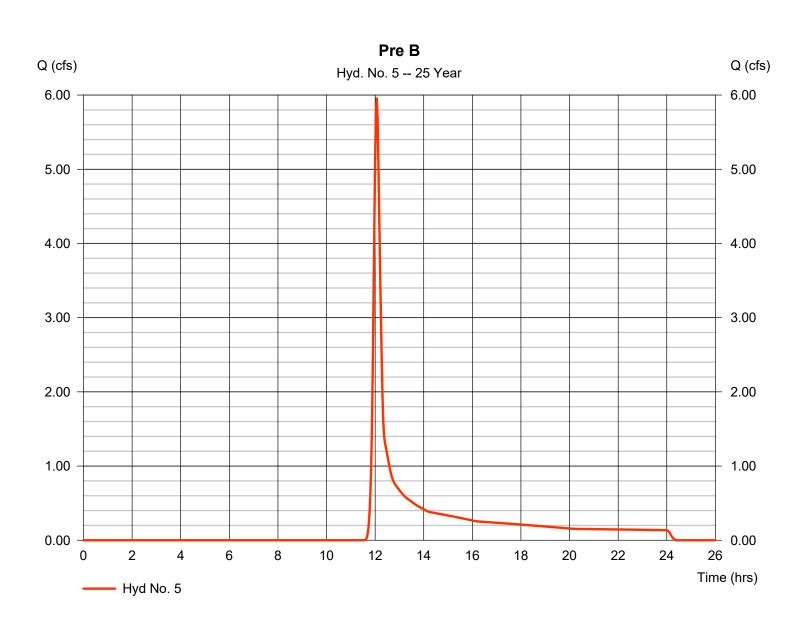
Pre B

Hydrograph type = SCS Runoff Peak discharge = 5.966 cfsStorm frequency = 25 yrsTime to peak = 12.07 hrsTime interval = 2 min Hyd. volume = 18,739 cuft= 4.530 acCurve number Drainage area = 58*

Basin Slope = 0.0 %Hydraulic length = 0 ft

Tc method Time of conc. (Tc) = User = 16.20 min Total precip. = 5.00 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(3.480 \times 55) + (0.550 \times 77) + (0.500 \times 61)] / 4.530$



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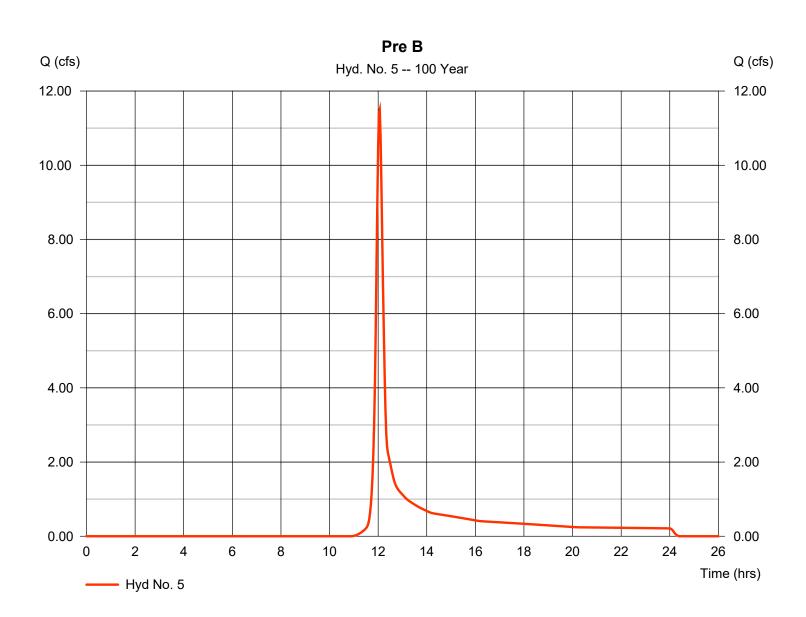
Wednesday, 01 / 31 / 2018

Hyd. No. 5

Pre B

Hydrograph type = SCS Runoff Peak discharge = 11.50 cfsStorm frequency = 100 yrsTime to peak = 12.07 hrsTime interval = 2 min Hyd. volume = 33,703 cuft= 4.530 acCurve number Drainage area = 58* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 16.20 min = User Total precip. = 6.54 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(3.480 \times 55) + (0.550 \times 77) + (0.500 \times 61)] / 4.530$



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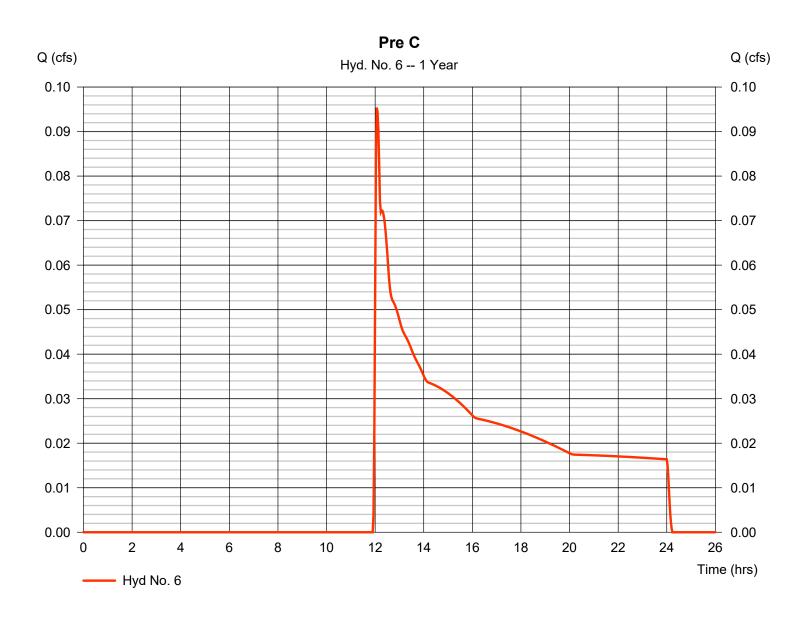
Wednesday, 01 / 31 / 2018

Hyd. No. 6

Pre C

Hydrograph type = SCS Runoff Peak discharge = 0.095 cfsStorm frequency Time to peak = 12.07 hrs= 1 yrsTime interval = 2 min Hyd. volume = 1,181 cuft Curve number Drainage area = 2.770 ac= 60* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.20 min = User Total precip. = 2.28 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.820 x 55) + (0.950 x 70)] / 2.770



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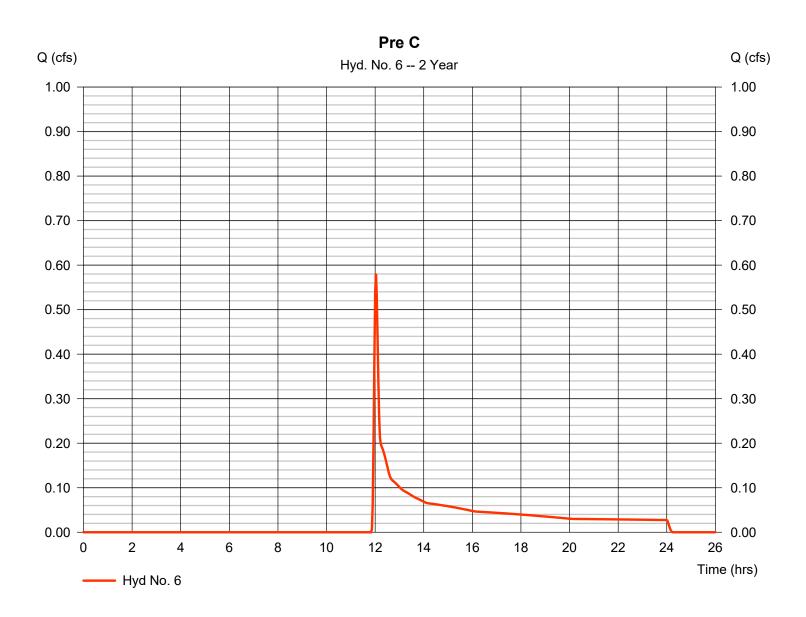
Wednesday, 01 / 31 / 2018

Hyd. No. 6

Pre C

Hydrograph type = SCS Runoff Peak discharge = 0.580 cfsStorm frequency = 2 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 2,526 cuftCurve number Drainage area = 2.770 ac= 60* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.20 min = User Total precip. = 2.76 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.820 x 55) + (0.950 x 70)] / 2.770



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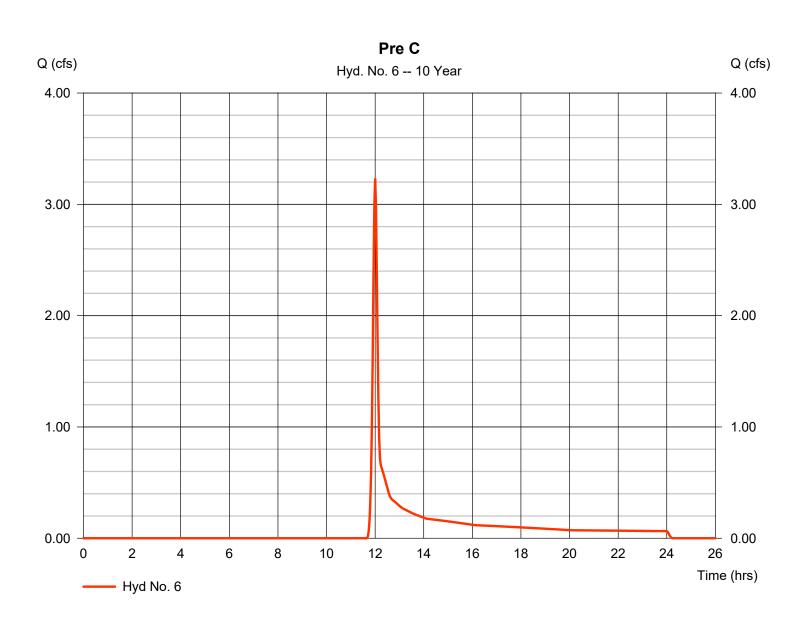
Wednesday, 01 / 31 / 2018

Hyd. No. 6

Pre C

Hydrograph type = SCS Runoff Peak discharge = 3.232 cfsStorm frequency = 10 yrsTime to peak = 12.00 hrsTime interval = 2 min Hyd. volume = 8,209 cuft= 2.770 acCurve number Drainage area = 60* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.20 min = User Total precip. = 4.11 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.820 x 55) + (0.950 x 70)] / 2.770



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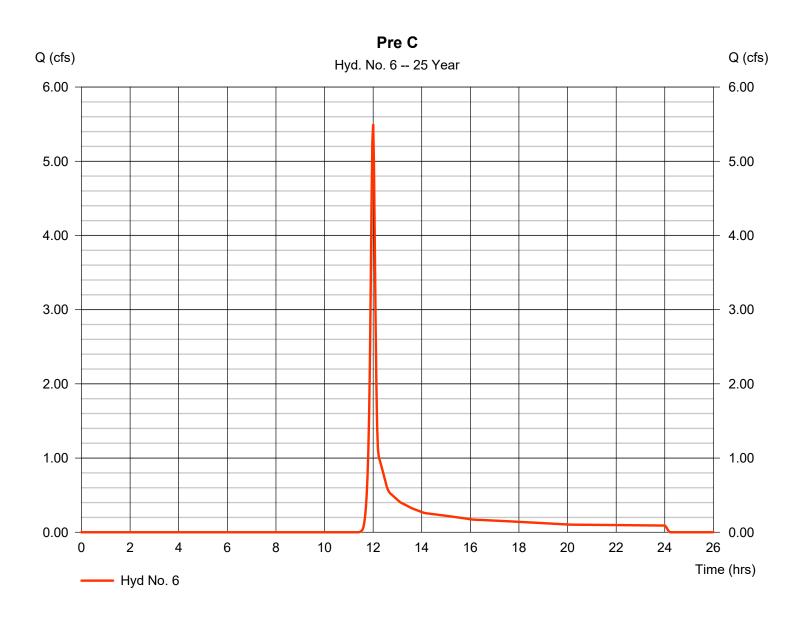
Wednesday, 01 / 31 / 2018

Hyd. No. 6

Pre C

Hydrograph type = SCS Runoff Peak discharge = 5.498 cfsStorm frequency = 25 yrsTime to peak = 12.00 hrsTime interval = 2 min Hyd. volume = 13.082 cuft = 2.770 acCurve number Drainage area = 60* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.20 min = User Total precip. = 5.00 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.820 x 55) + (0.950 x 70)] / 2.770



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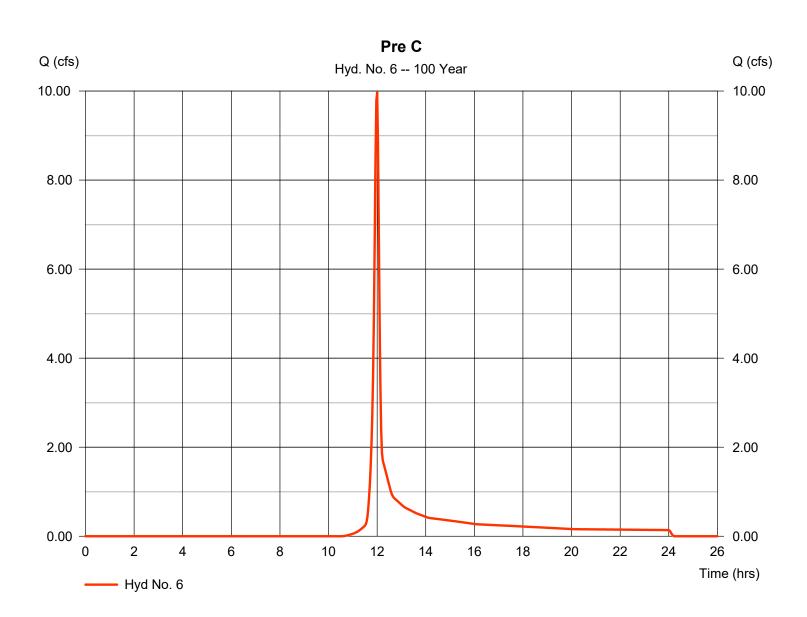
Wednesday, 01 / 31 / 2018

Hyd. No. 6

Pre C

Hydrograph type = SCS Runoff Peak discharge $= 9.973 \, \text{cfs}$ Storm frequency = 100 yrsTime to peak = 12.00 hrsTime interval = 2 min Hyd. volume = 22,958 cuft = 2.770 acCurve number Drainage area = 60* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 9.20 min = User Total precip. = 6.54 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(1.820 x 55) + (0.950 x 70)] / 2.770



APPENDIX C – EXISTING FACILITY #4 VERIFICATION

(See attached Drawings)

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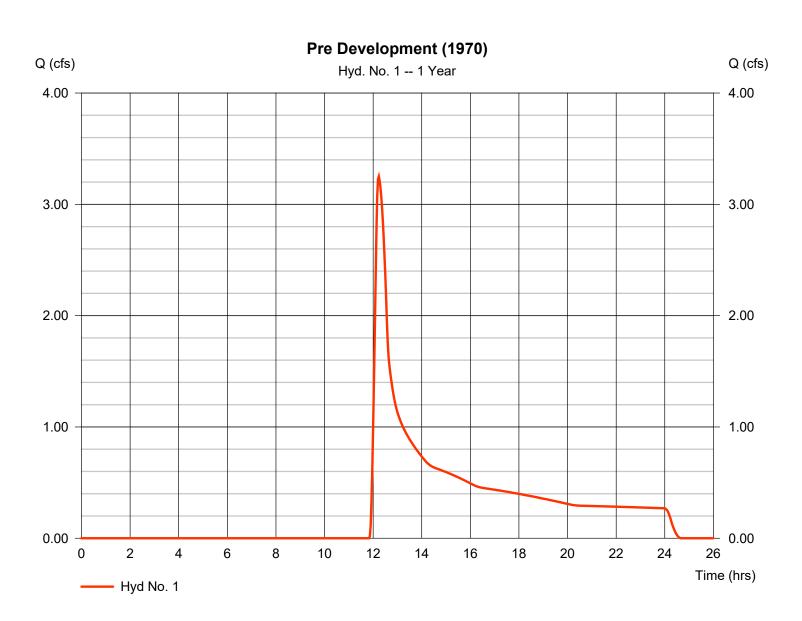
Hyd. No. 1

Pre Development (1970)

Hydrograph type = SCS Runoff Peak discharge = 3.251 cfsStorm frequency Time to peak $= 12.23 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 25.326 cuft Drainage area = 32.260 acCurve number = 65* Basin Slope = 0.0 %Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 25.00 min
Total precip. = 2.28 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(21.850 x 61) + (2.230 x 55) + (3.000 x 74) + (5.180 x 80)] / 32.260



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Wednesday, 01 / 31 / 2018

= 484

Hyd. No. 1

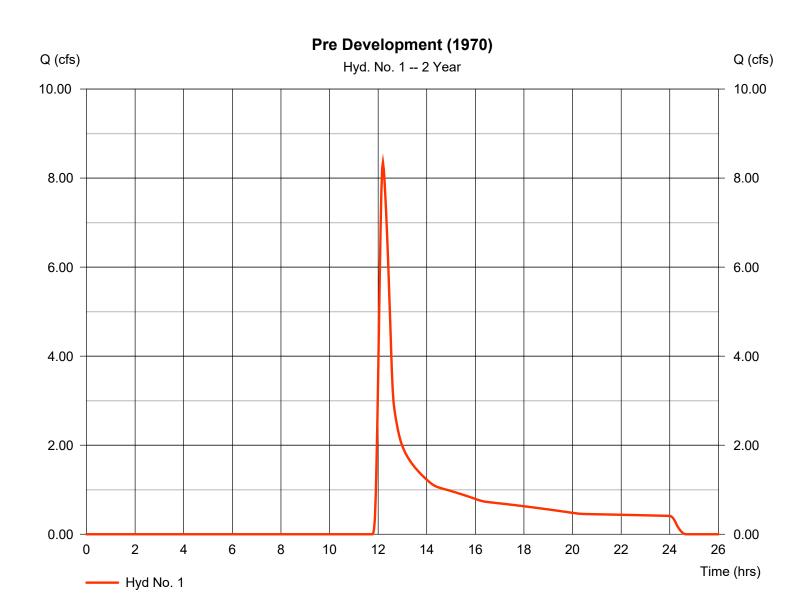
Storm duration

Pre Development (1970)

Hydrograph type = SCS Runoff Peak discharge = 8.347 cfsStorm frequency = 2 yrsTime to peak = 12.20 hrsTime interval = 2 min Hyd. volume = 46.188 cuft Curve number Drainage area = 32.260 ac= 65* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 25.00 min = User Total precip. = 2.76 inDistribution = Type II

Shape factor

= 24 hrs



^{*} Composite (Area/CN) = [(21.850 x 61) + (2.230 x 55) + (3.000 x 74) + (5.180 x 80)] / 32.260

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 1

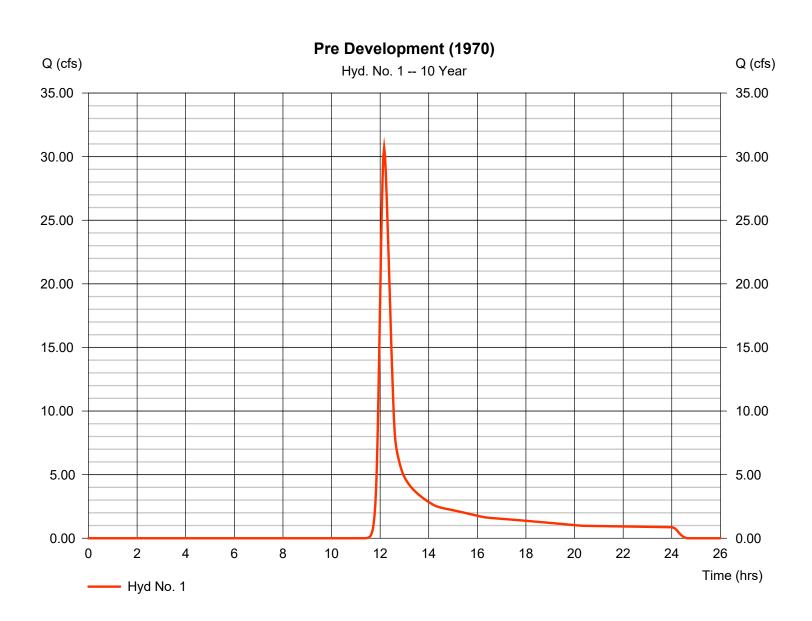
Pre Development (1970)

Hydrograph type= SCS RunoffPeak discharge= 30.69 cfsStorm frequency= 10 yrsTime to peak= 12.17 hrsTime interval= 2 minHyd. volume= 125,981 cuft

Drainage area = 32.260 ac Curve number = 65^* Basin Slope = 0.0 % Hydraulic length = 0.0 ft

Tc method = User Time of conc. (Tc) = 25.00 min
Total precip. = 4.11 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(21.850 x 61) + (2.230 x 55) + (3.000 x 74) + (5.180 x 80)] / 32.260



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 1

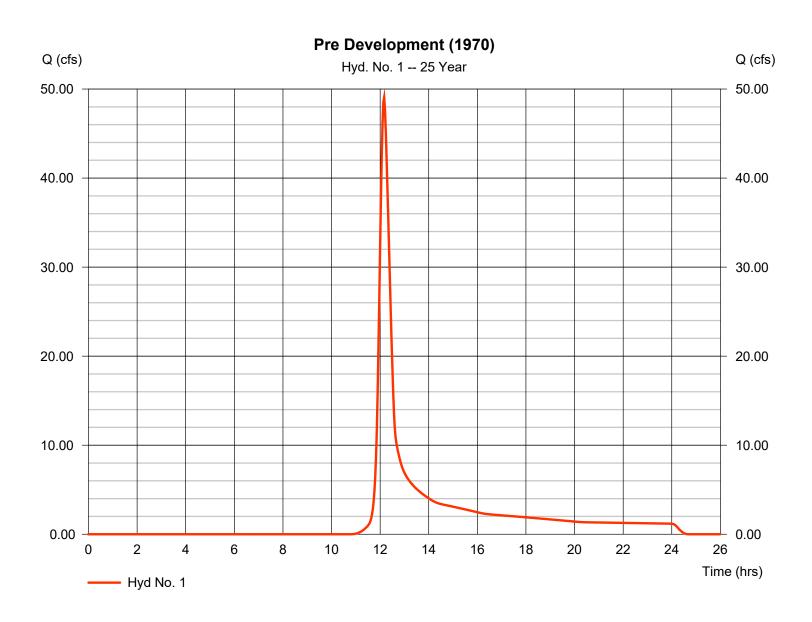
Pre Development (1970)

Hydrograph type= SCS RunoffPeak discharge= 49.05 cfsStorm frequency= 25 yrsTime to peak= 12.17 hrsTime interval= 2 minHyd. volume= 190,609 cuft

Drainage area = 32.260 ac Curve number = 65^* Basin Slope = 0.0 % Hydraulic length = 0.0 ft

Tc method = User Time of conc. (Tc) = 25.00 min
Total precip. = 5.00 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(21.850 \times 61) + (2.230 \times 55) + (3.000 \times 74) + (5.180 \times 80)] / 32.260$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 1

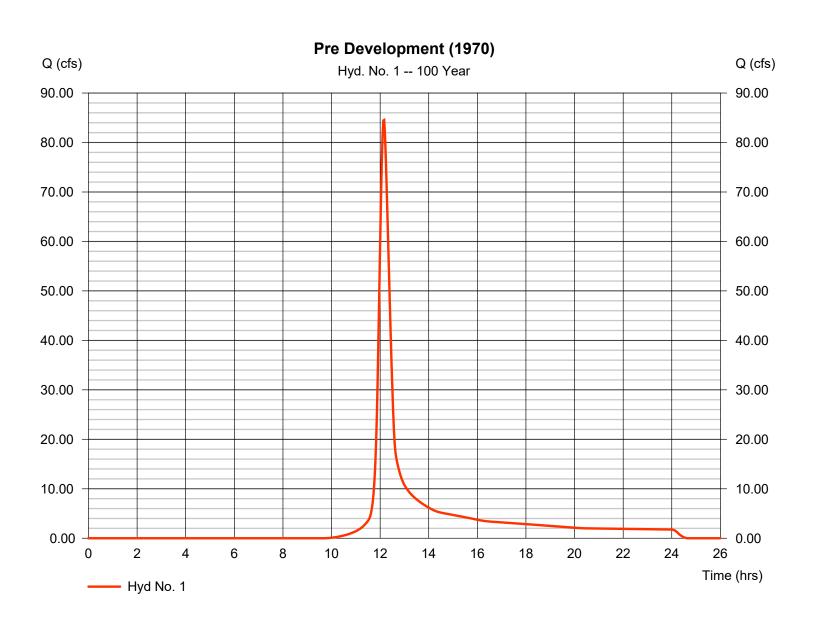
Pre Development (1970)

Hydrograph type= SCS RunoffPeak discharge= 84.55 cfsStorm frequency= 100 yrsTime to peak= 12.17 hrsTime interval= 2 minHyd. volume= 317,153 cuft

Drainage area = 32.260 ac Curve number = 65^* Basin Slope = 0.0 % Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 25.00 min
Total precip. = 6.54 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(21.850 x 61) + (2.230 x 55) + (3.000 x 74) + (5.180 x 80)] / 32.260



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 2

Post Development (2015)

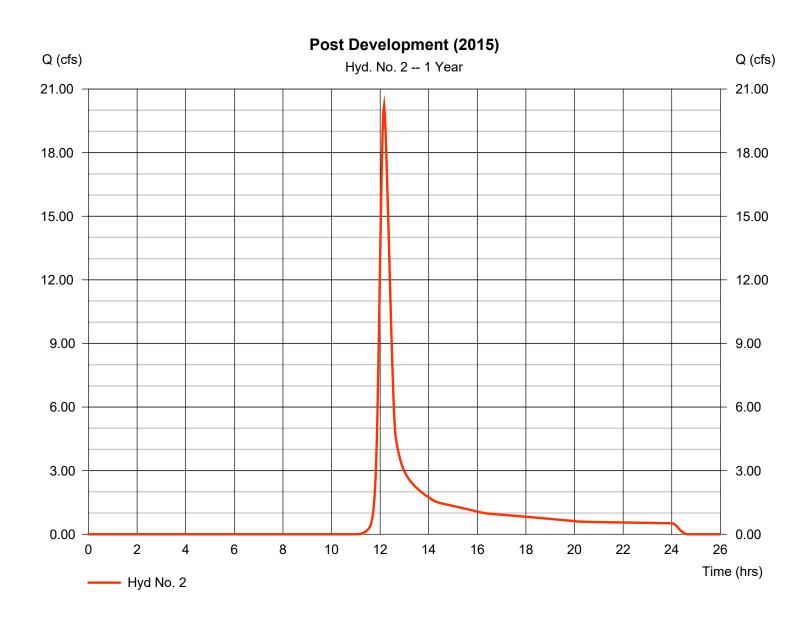
Hydrograph type = SCS Runoff Peak discharge = 20.21 cfsStorm frequency Time to peak $= 12.17 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 79.963 cuft Curve number Drainage area = 32.260 ac= 79* Basin Slope = 0.0 %Hydraulic length = 0 ft= 24.50 min Tc method Time of conc. (Tc) = User

Tc method = User Time of conc. (Tc) = 24.50 min

Total precip. = 2.28 in Distribution = Type II

Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(11.110 x 61) + (2.090 x 55) + (1.500 x 74) + (3.720 x 80) + (13.840 x 98)] / 32.260



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Wednesday, 01 / 31 / 2018

Hyd. No. 2

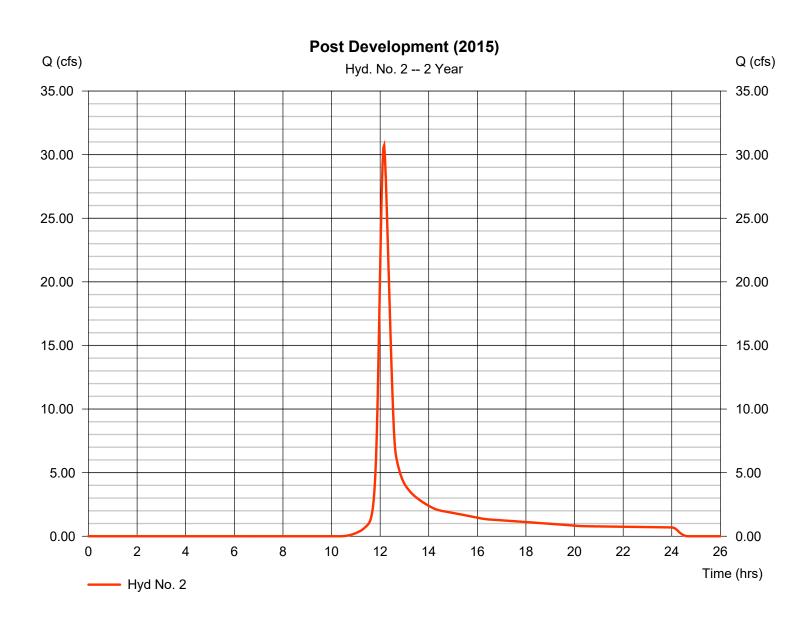
Post Development (2015)

Hydrograph type = SCS Runoff Peak discharge = 30.71 cfsStorm frequency = 2 yrsTime to peak $= 12.17 \, hrs$ Time interval = 2 min Hyd. volume = 117,137 cuft Curve number = 79* Drainage area = 32.260 ac

Basin Slope = 32.260 ac Curve number = 79° Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 24.50 min
Total precip. = 2.76 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(11.110 \times 61) + (2.090 \times 55) + (1.500 \times 74) + (3.720 \times 80) + (13.840 \times 98)] / 32.260$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 2

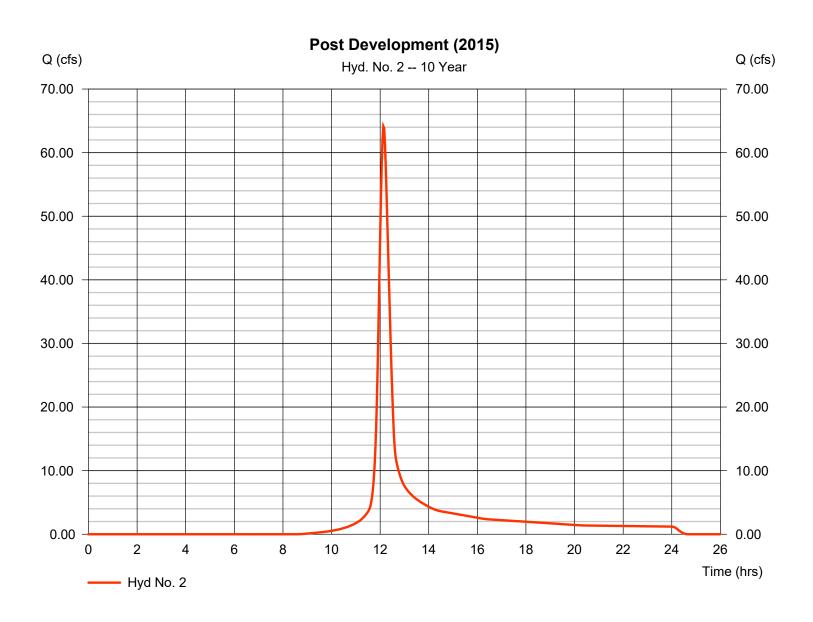
Post Development (2015)

Hydrograph type = SCS Runoff Peak discharge = 64.15 cfs
Storm frequency = 10 yrs Time to peak = 12.13 hrs
Time interval = 2 min Hyd. volume = 236,675 cuft

Drainage area = 32.260 ac Curve number = 79^* Basin Slope = 0.0 % Hydraulic length = 0.0 ft

Tc method = User Time of conc. (Tc) = 24.50 min
Total precip. = 4.11 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(11.110 \times 61) + (2.090 \times 55) + (1.500 \times 74) + (3.720 \times 80) + (13.840 \times 98)] / 32.260$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 2

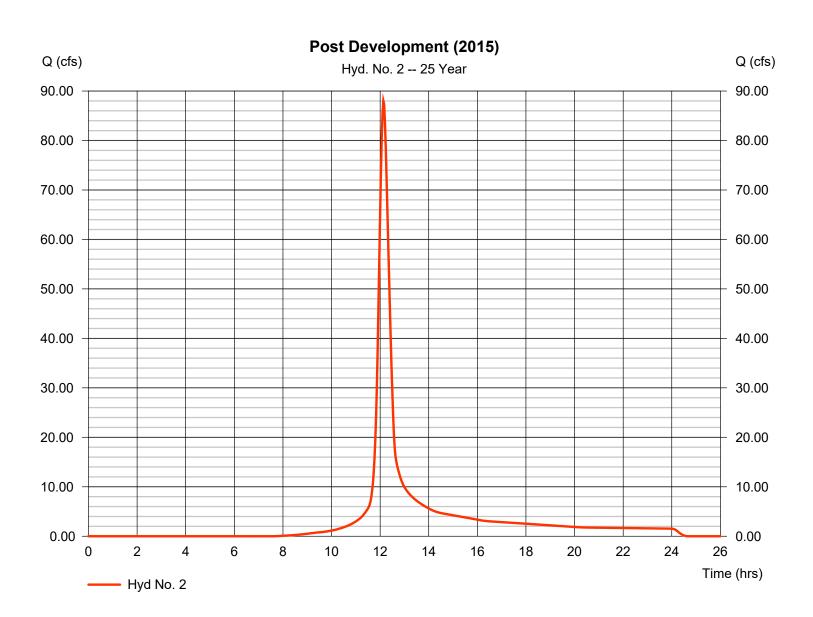
Post Development (2015)

Hydrograph type= SCS RunoffPeak discharge= 87.99 cfsStorm frequency= 25 yrsTime to peak= 12.13 hrsTime interval= 2 minHyd. volume= 322,958 cuft

Drainage area = 32.260 ac Curve number = 79^* Basin Slope = 0.0 % Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 24.50 min
Total precip. = 5.00 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(11.110 x 61) + (2.090 x 55) + (1.500 x 74) + (3.720 x 80) + (13.840 x 98)] / 32.260



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 2

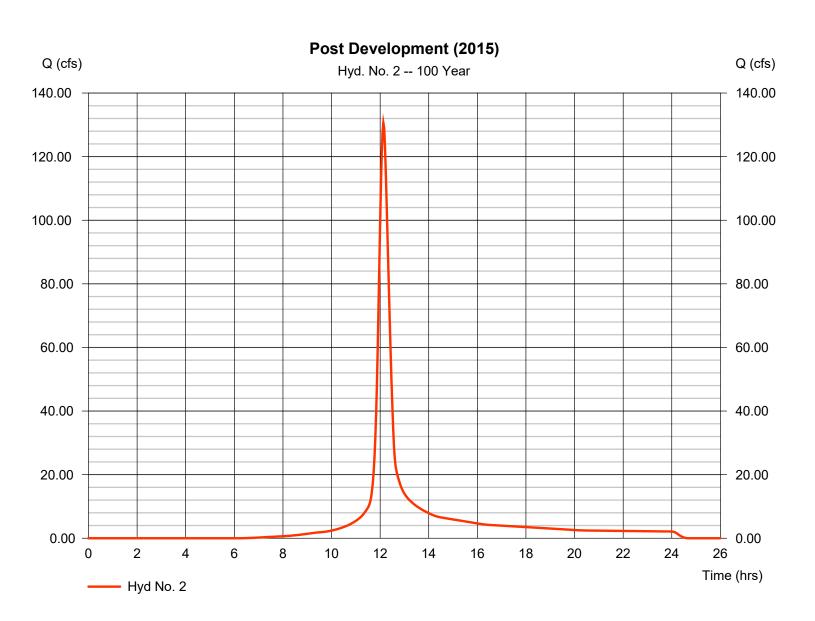
Post Development (2015)

Hydrograph type= SCS RunoffPeak discharge= 130.63 cfsStorm frequency= 100 yrsTime to peak= 12.13 hrsTime interval= 2 minHyd. volume= 480,170 cuftDrainage area= 32.260 acCurve number= 79*

Drainage area = 32.260 ac Curve number = 79^* Basin Slope = 0.0 % Hydraulic length = 0.0 ft

Tc method = User Time of conc. (Tc) = 24.50 min
Total precip. = 6.54 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(11.110 x 61) + (2.090 x 55) + (1.500 x 74) + (3.720 x 80) + (13.840 x 98)] / 32.260



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Pond No. 1 - Facility #4 (2015)

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	2101.90	n/a	0	0
2.10	2104.00	n/a	1,873	1,873
4.10	2106.00	n/a	8,276	10,149
6.10	2108.00	n/a	18,339	28,488
8.10	2110.00	n/a	32,060	60,548
10.10	2112.00	n/a	49,963	110,511
12.10	2114.00	n/a	65,253	175,764

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 36.00	3.00	0.00	0.00	Crest Len (ft)	= 9.00	0.00	0.00	0.00
Span (in)	= 36.00	3.00	0.00	0.00	Crest El. (ft)	= 2109.70	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 2101.77	2101.90	0.00	0.00	Weir Type	= 1			
Length (ft)	= 150.69	0.10	0.00	0.00	Multi-Stage	= Yes	No	No	No
Slope (%)	= 3.90	0.10	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by)	Wet area)		
Multi-Stage	= n/a	Yes	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	CIv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	2101.90	0.00	0.00			0.00						0.000
0.21	187	2102.11	0.14 ic	0.07 ic			0.00						0.069
0.42	375	2102.32	0.14 ic	0.13 ic			0.00						0.128
0.63	562	2102.53	0.18 ic	0.17 ic			0.00						0.168
0.84	749	2102.74	0.20 ic	0.20 ic			0.00						0.200
1.05	937	2102.95	0.23 ic	0.23 ic			0.00						0.227
1.26	1,124	2103.16	0.26 ic	0.25 ic			0.00						0.252
1.47	1,311	2103.37	0.29 ic	0.27 ic			0.00						0.274
1.68	1,498	2103.58	0.29 ic	0.29 ic			0.00						0.295
1.89	1,686	2103.79	0.33 ic	0.31 ic			0.00						0.314
2.10	1,873	2104.00	0.33 ic	0.33 ic			0.00						0.332
2.30	2,701	2104.20	0.37 ic	0.35 ic			0.00						0.349
2.50	3,528	2104.40	0.37 ic	0.36 ic			0.00						0.364
2.70	4,356	2104.60	0.41 ic	0.38 ic			0.00						0.379
2.90	5,183	2104.80	0.41 ic	0.39 ic			0.00						0.394
3.10	6,011	2105.00	0.41 ic	0.41 ic			0.00						0.408
3.30	6,839	2105.20	0.45 ic	0.42 ic			0.00						0.421
3.50	7,666	2105.40	0.45 ic	0.43 ic			0.00						0.434
3.70	8,494	2105.60	0.45 ic	0.45 ic			0.00						0.447
3.90	9,321	2105.80	0.46 ic	0.46 ic			0.00						0.459
4.10	10,149	2106.00	0.50 ic	0.47 ic			0.00						0.471
4.30	11,983	2106.20	0.50 ic	0.48 ic			0.00						0.483
4.50	13,817	2106.40	0.50 ic	0.49 ic			0.00						0.494
4.70	15,651	2106.60	0.51 ic	0.51 ic			0.00						0.505
4.90	17,485	2106.80	0.55 ic	0.52 ic			0.00						0.516
5.10	19,319	2107.00	0.55 ic	0.53 ic			0.00						0.527
5.30	21,152	2107.20	0.55 ic	0.54 ic			0.00						0.537
5.50	22,986	2107.40	0.55 ic	0.55 ic			0.00						0.548
5.70	24,820	2107.60	0.56 ic	0.56 ic			0.00						0.558
5.90	26,654	2107.80	0.57 ic	0.57 ic			0.00						0.567
6.10	28,488	2108.00	0.61 ic	0.58 ic			0.00						0.577
6.30	31,694	2108.20	0.61 ic	0.59 ic			0.00						0.586
6.50	34,900	2108.40	0.61 ic	0.60 ic			0.00						0.596
6.70	38,106	2108.60	0.61 ic	0.61 ic			0.00						0.605
6.90	41,312	2108.80	0.61 ic	0.61 ic			0.00						0.614
7.10	44,518	2109.00	0.62 ic	0.62 ic			0.00						0.623
7.30	47,724	2109.20	0.67 ic	0.63 ic			0.00						0.632
											Cantinu		4 2222

Continues on next page...

Facility #4 (2015) Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	CIv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
7.50	50,930	2109.40	0.67 ic	0.64 ic			0.00						0.640
7.70	54,136	2109.60	0.67 ic	0.65 ic			0.00						0.649
7.90	57,342	2109.80	1.64 ic	0.65 ic			0.95						1.600
8.10	60,548	2110.00	5.74 ic	0.64 ic			4.93						5.566
8.30	65,544	2110.20	11.49 ic	0.63 ic			10.60						11.23
8.50	70,541	2110.40	18.43 ic	0.62 ic			17.55						18.17
8.70	75,537	2110.40	26.28 ic	0.61 ic			25.60						26.21
8.90	80,533	2110.80	35.20 ic	0.60 ic			34.58						35.18
9.10	85,529	2111.00	34.93 ic	0.61 ic			31.04 ic						31.65
9.30	90,526	2111.20	34.66 ic	0.62 ic			33.34 ic						33.96
9.50	95,522	2111.40	36.12 ic	0.63 ic			35.49 ic						36.12
9.70	100,518	2111.60	38.15 ic	0.63 ic			37.52 ic						38.15
9.90	105,515	2111.80	40.08 ic	0.63 ic			39.45 ic						40.08
10.10	110,511	2112.00	41.92 ic	0.63 ic			41.28 ic						41.92
10.10	117,036	2112.20	43.68 ic	0.64 ic			43.04 ic						43.68
10.50	123,562	2112.40	45.37 ic	0.64 ic			44.73 ic						45.37
10.70	130,087	2112.40	47.00 ic	0.64 ic			46.35 ic						47.00
10.70	136,612	2112.80	47.00 ic 48.57 ic	0.65 ic			47.93 ic						48.57
11.10	143,138	2112.00	50.10 ic	0.65 ic			49.45 ic						50.10
11.10	143,136	2113.00	50.10 lc	0.65 ic			50.92 ic						51.58
11.50	156,188	2113.40	53.01 ic	0.66 ic			52.36 ic						53.01
	,												
11.70	162,713	2113.60	54.42 ic	0.66 ic			53.76 ic						54.41
11.90	169,239	2113.80	55.78 ic	0.66 ic			55.12 ic						55.78 57.11
12.10	175,764	2114.00	57.11 ic	0.66 ic			56.44 ic						57.11

...End

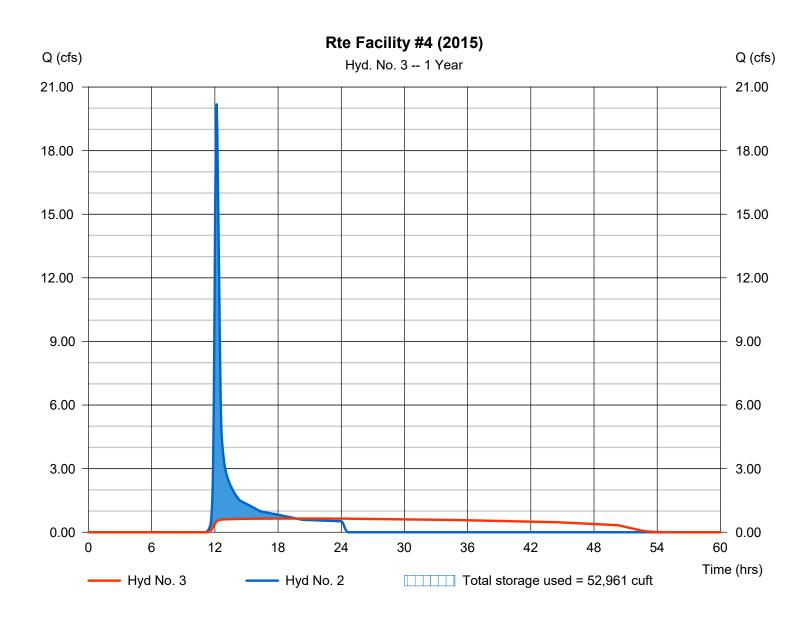
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Wednesday, 01 / 31 / 2018

Hyd. No. 3

Rte Facility #4 (2015)

Hydrograph type = Reservoir Peak discharge = 0.646 cfsStorm frequency Time to peak $= 19.73 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 79,960 cuft= 2 - Post Development (2015) Max. Elevation Inflow hyd. No. = 2109.53 ft= Facility #4 (2015) Reservoir name Max. Storage = 52,961 cuft



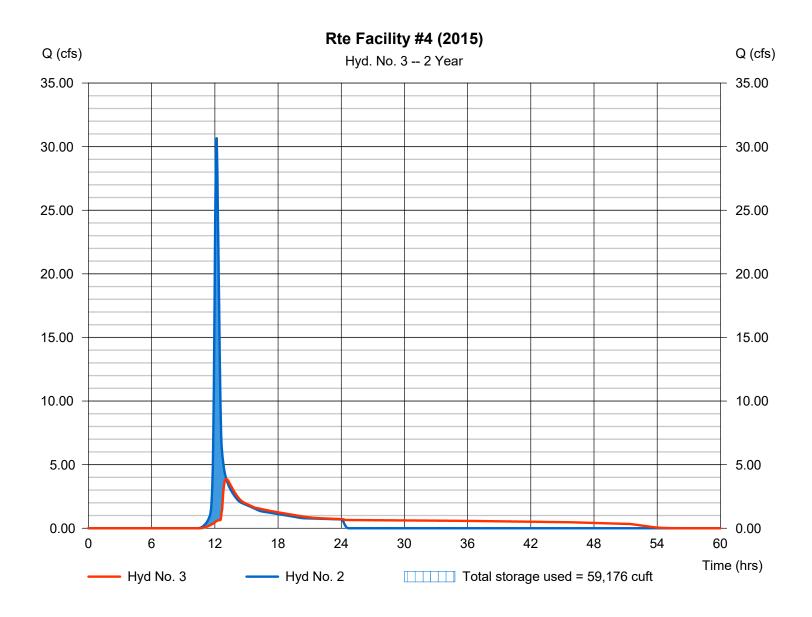
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Wednesday, 01 / 31 / 2018

Hyd. No. 3

Rte Facility #4 (2015)

Hydrograph type = Reservoir Peak discharge = 3.869 cfsStorm frequency = 2 yrsTime to peak = 13.10 hrsTime interval = 2 min Hyd. volume = 117,134 cuft Max. Elevation Inflow hyd. No. = 2 - Post Development (2015) $= 2109.91 \, \text{ft}$ = Facility #4 (2015) Reservoir name Max. Storage = 59,176 cuft



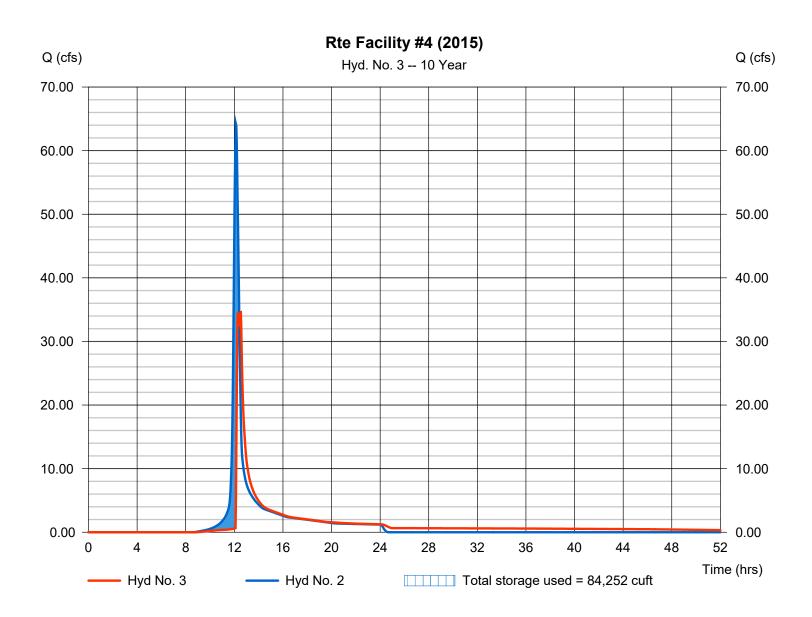
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Wednesday, 01 / 31 / 2018

Hyd. No. 3

Rte Facility #4 (2015)

Hydrograph type = Reservoir Peak discharge = 34.79 cfsStorm frequency = 10 yrsTime to peak $= 12.53 \, hrs$ Time interval = 2 min Hyd. volume = 236,672 cuft Max. Elevation Inflow hyd. No. = 2 - Post Development (2015) = 2110.96 ft= Facility #4 (2015) = 84,252 cuft Reservoir name Max. Storage



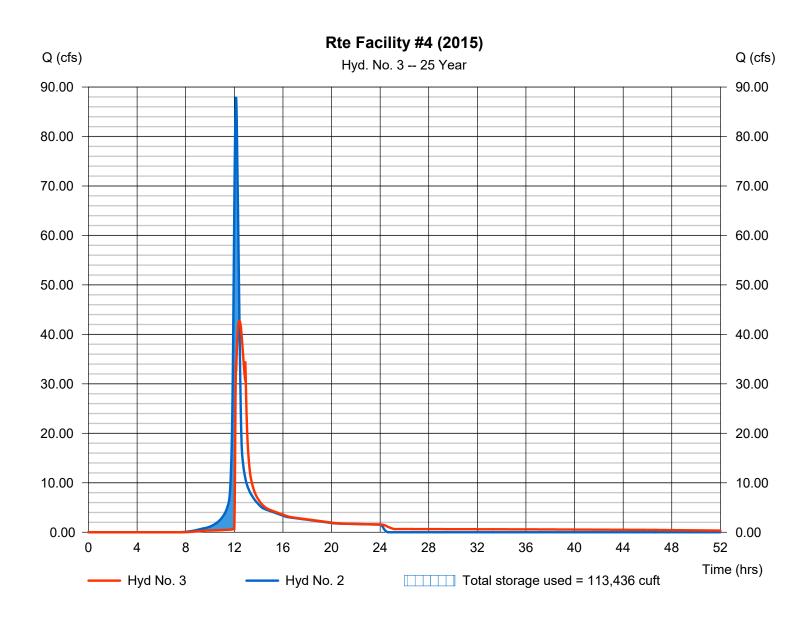
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Wednesday, 01 / 31 / 2018

Hyd. No. 3

Rte Facility #4 (2015)

Hydrograph type = Reservoir Peak discharge = 42.70 cfsStorm frequency = 25 yrsTime to peak $= 12.43 \, hrs$ Time interval = 2 min Hyd. volume = 322,955 cuft Max. Elevation Inflow hyd. No. = 2 - Post Development (2015) = 2112.09 ft= Facility #4 (2015) Reservoir name Max. Storage = 113,436 cuft



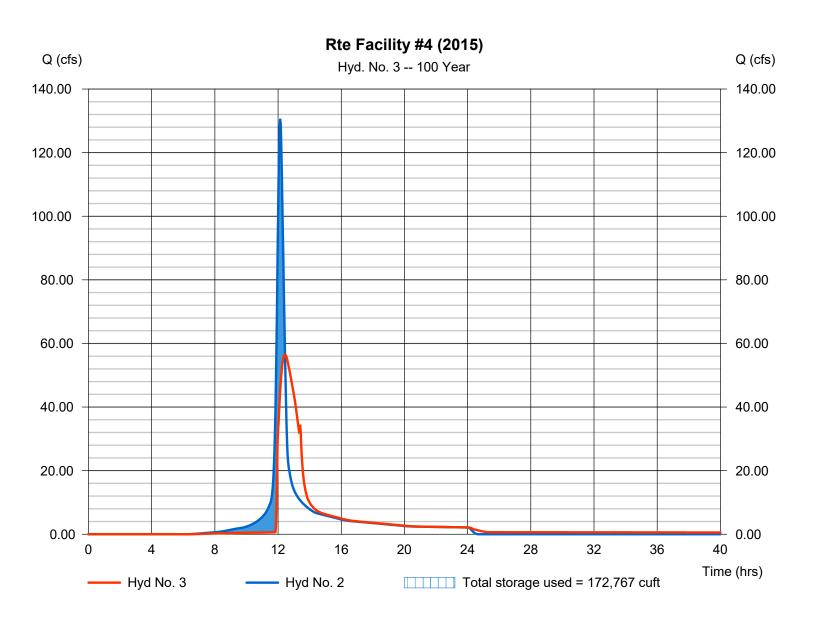
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Wednesday, 01 / 31 / 2018

Hyd. No. 3

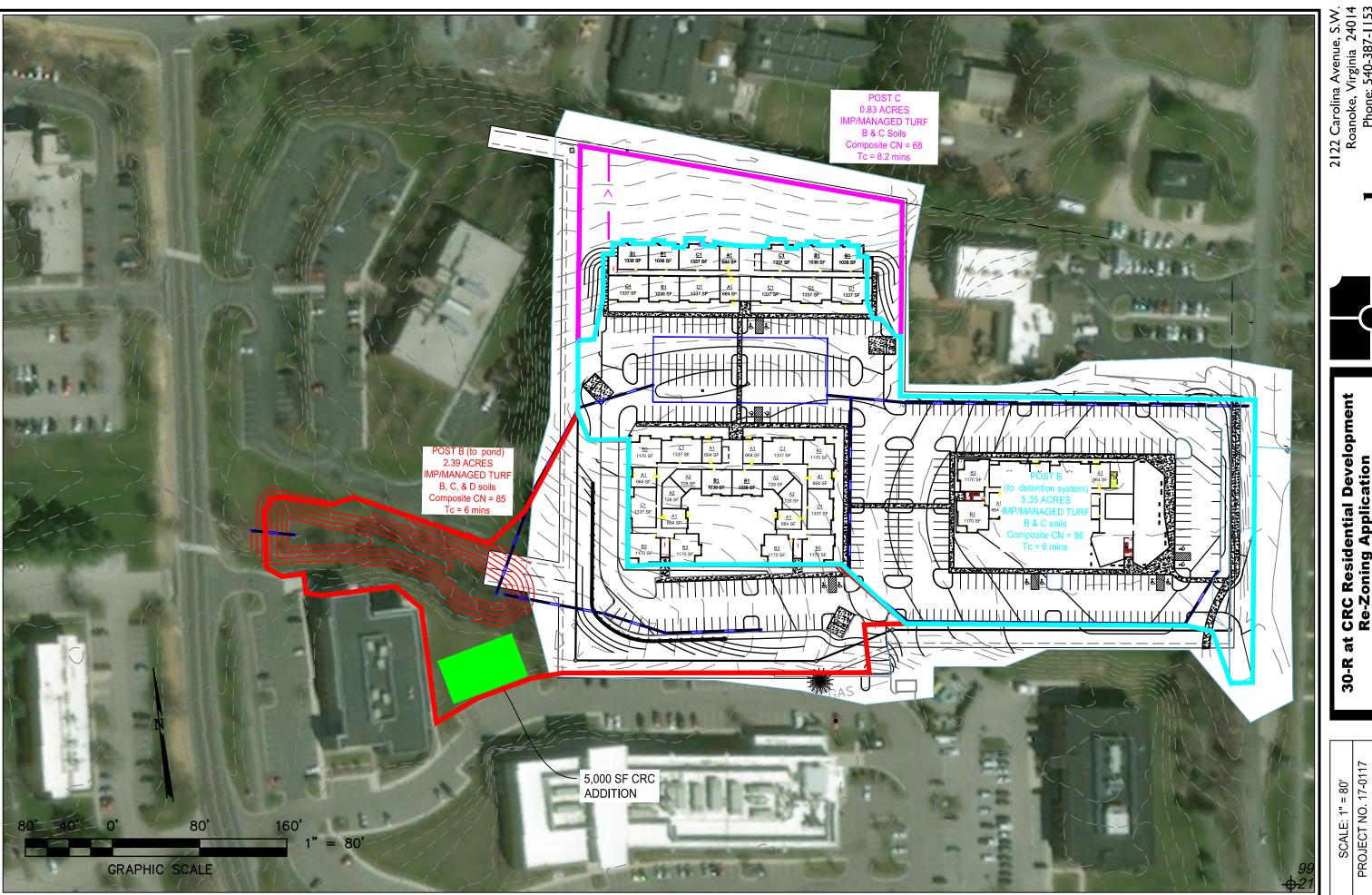
Rte Facility #4 (2015)

Hydrograph type = Reservoir Peak discharge = 56.50 cfsStorm frequency Time to peak $= 12.43 \, hrs$ = 100 yrsTime interval = 2 min Hyd. volume = 480,167 cuft Max. Elevation Inflow hyd. No. = 2 - Post Development (2015) = 2113.91 ft = Facility #4 (2015) Reservoir name Max. Storage = 172,767 cuft



APPENDIX D – POST-DEVELOPMENT HYDROLOGIC CONDITIONS

(See attached Drawings)



Development

30-R at CRC Residential **Post Developm**

רשיפוסpment Drainage Map Town of Blacksburg, VA

DATE: 02/01/2018 APPENDIX C.2

Post Development Area C

<u>Time of Concentration</u>									
100 ft OLF@	9 % =	8.20 min							
0 ft CF @	0 % =	0.00 min							
0 ft CF @	0 % =	0.00 min							
	Tc=	8.20 min							
User defined									
C factor for OLF=	0.30								

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

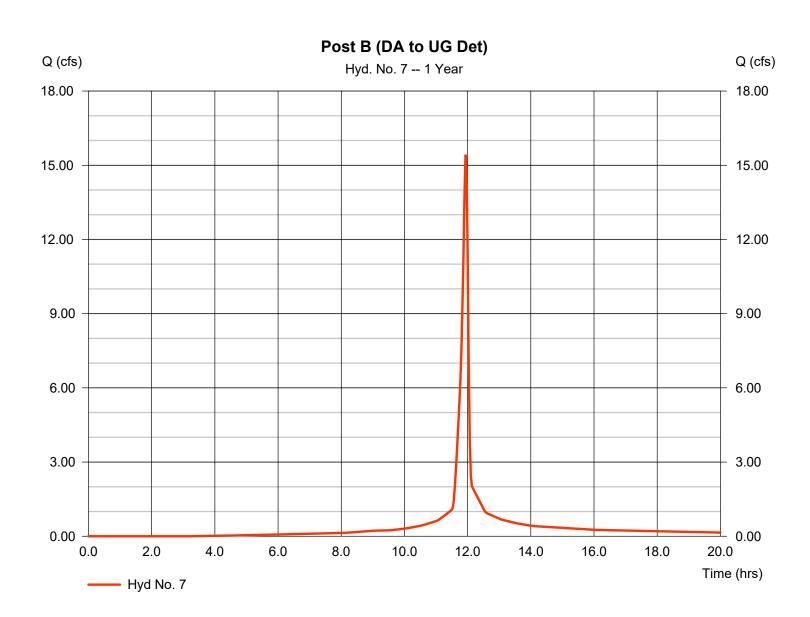
Wednesday, 01 / 31 / 2018

Hyd. No. 7

Post B (DA to UG Det)

Hydrograph type = SCS Runoff Peak discharge = 15.43 cfsStorm frequency Time to peak = 11.93 hrs= 1 yrsTime interval = 2 min Hyd. volume = 33.617 cuft Curve number Drainage area = 5.350 ac= 96* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 2.28 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(5.100 \times 98) + (0.250 \times 61)] / 5.350$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

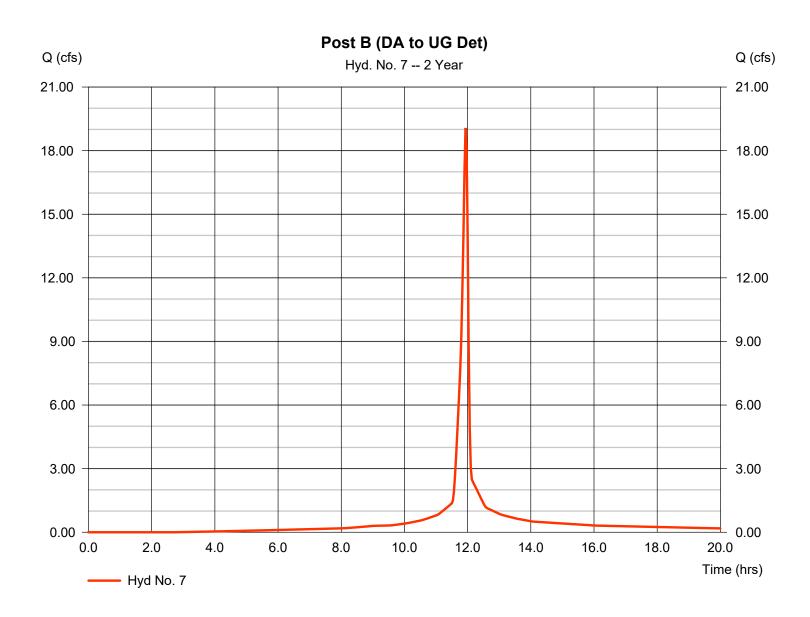
Wednesday, 01 / 31 / 2018

Hyd. No. 7

Post B (DA to UG Det)

Hydrograph type = SCS Runoff Peak discharge = 19.07 cfsStorm frequency = 2 yrsTime to peak $= 11.93 \, hrs$ Time interval = 2 min Hyd. volume = 42.169 cuftCurve number Drainage area = 5.350 ac= 96* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 2.76 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(5.100 \times 98) + (0.250 \times 61)] / 5.350$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

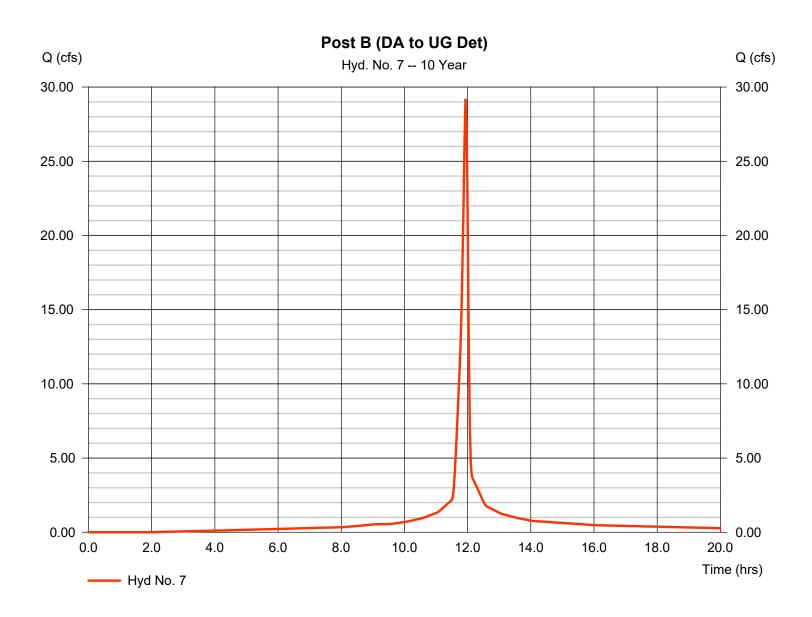
Wednesday, 01 / 31 / 2018

Hyd. No. 7

Post B (DA to UG Det)

Hydrograph type = SCS Runoff Peak discharge = 29.20 cfsStorm frequency = 10 yrsTime to peak = 11.93 hrsTime interval = 2 min Hyd. volume = 66.438 cuft Curve number Drainage area = 5.350 ac= 96* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 4.11 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(5.100 \times 98) + (0.250 \times 61)] / 5.350$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

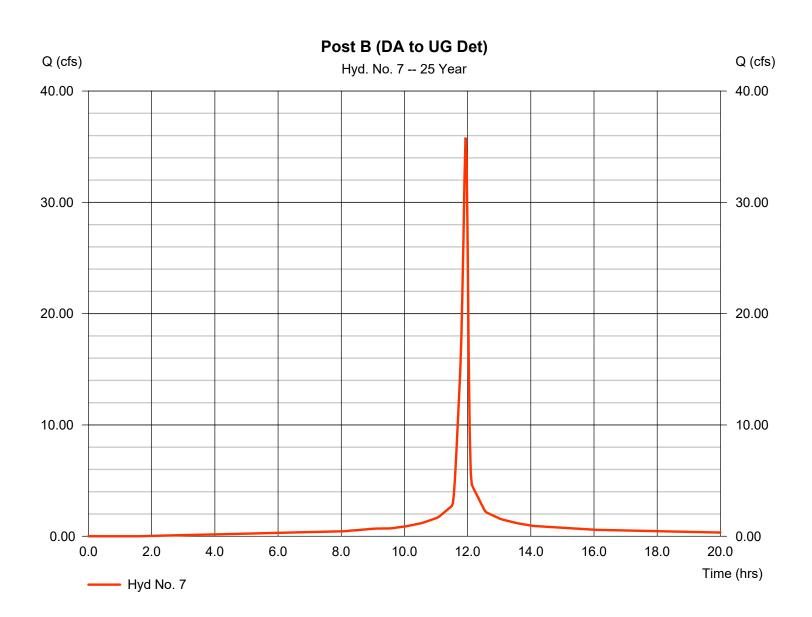
Wednesday, 01 / 31 / 2018

Hyd. No. 7

Post B (DA to UG Det)

Hydrograph type = SCS Runoff Peak discharge = 35.82 cfsStorm frequency = 25 yrsTime to peak = 11.93 hrsTime interval = 2 min Hyd. volume = 82.523 cuft = 5.350 acCurve number Drainage area = 96* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User Total precip. = 5.00 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(5.100 \times 98) + (0.250 \times 61)] / 5.350$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

= 24 hrs

Wednesday, 01 / 31 / 2018

= 484

Hyd. No. 7

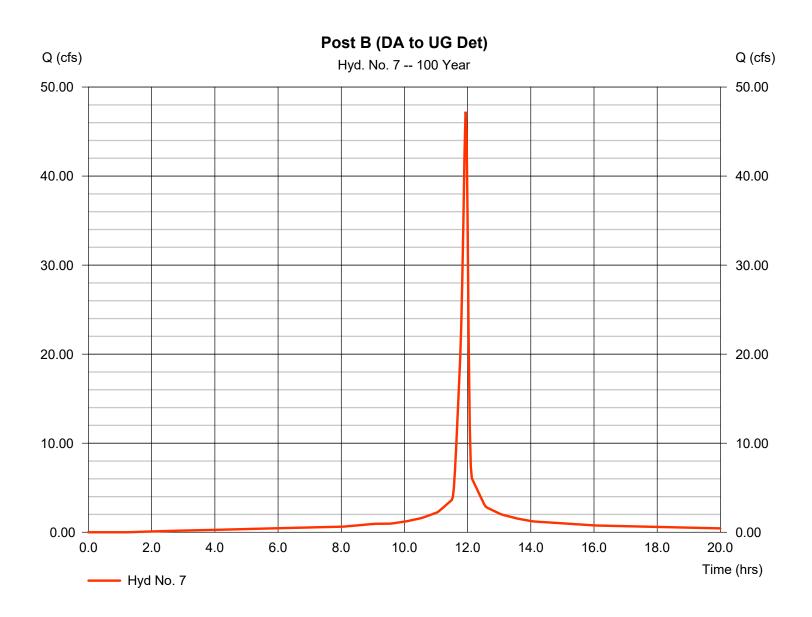
Storm duration

Post B (DA to UG Det)

Hydrograph type = SCS Runoff Peak discharge = 47.22 cfsStorm frequency = 100 yrsTime to peak $= 11.93 \, hrs$ Time interval = 2 min Hyd. volume = 110.428 cuft Drainage area = 5.350 acCurve number = 96* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 6.00 min Tc method = User Total precip. = 6.54 inDistribution = Type II

Shape factor

^{*} Composite (Area/CN) = $[(5.100 \times 98) + (0.250 \times 61)] / 5.350$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

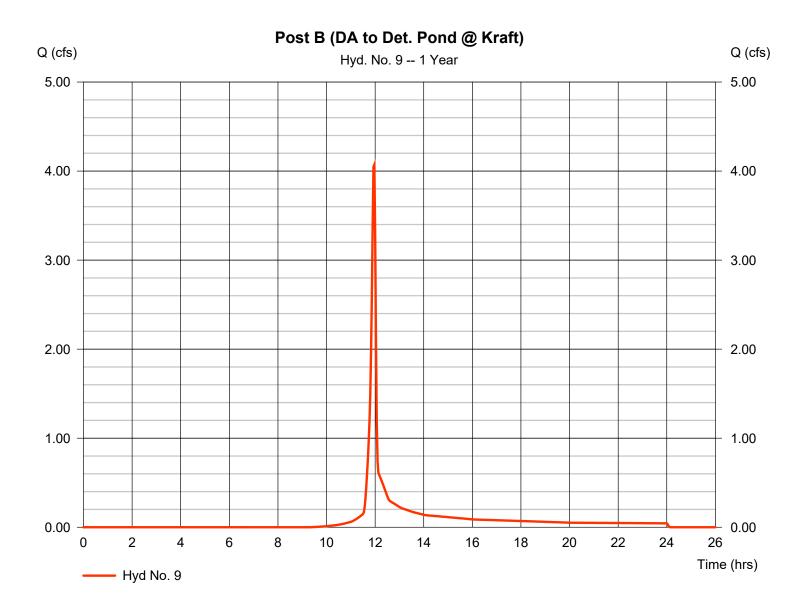
Wednesday, 01 / 31 / 2018

Hyd. No. 9

Post B (DA to Det. Pond @ Kraft)

Hydrograph type = SCS Runoff Peak discharge = 4.072 cfsStorm frequency Time to peak $= 11.97 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 8,181 cuft Drainage area = 2.390 acCurve number = 85* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 6.00 \, \text{min}$ Total precip. = 2.28 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.620 x 98) + (1.770 x 80)] / 2.390



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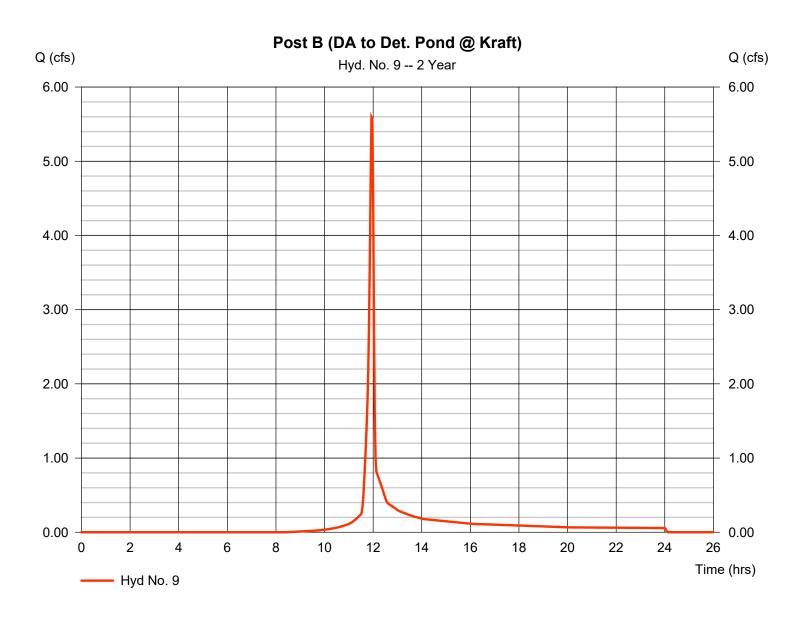
Wednesday, 01 / 31 / 2018

Hyd. No. 9

Post B (DA to Det. Pond @ Kraft)

Hydrograph type = SCS Runoff Peak discharge = 5.592 cfsStorm frequency Time to peak $= 11.93 \, hrs$ = 2 yrsTime interval = 2 min Hyd. volume = 11.296 cuft Curve number Drainage area = 2.390 ac= 85* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 6.00 \, \text{min}$ Total precip. = 2.76 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = [(0.620 x 98) + (1.770 x 80)] / 2.390



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

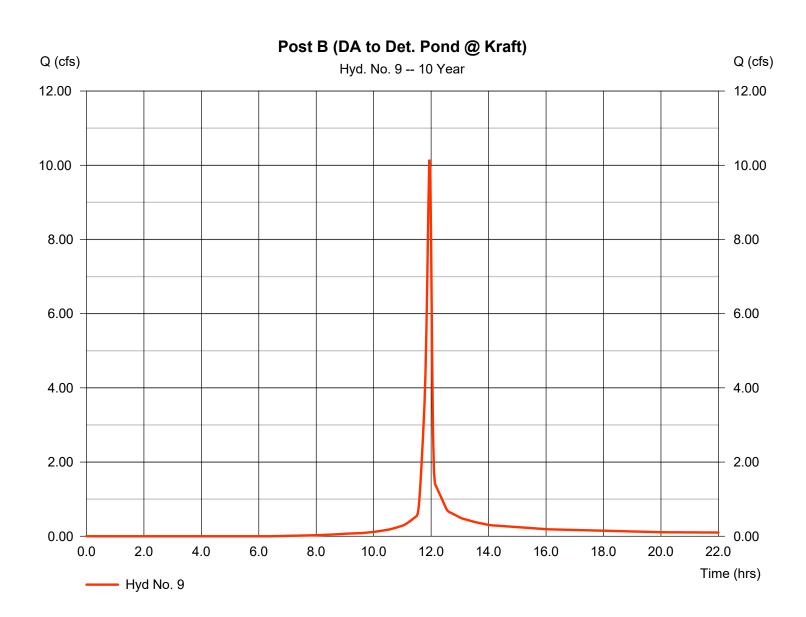
Wednesday, 01 / 31 / 2018

Hyd. No. 9

Post B (DA to Det. Pond @ Kraft)

Hydrograph type = SCS Runoff Peak discharge = 10.15 cfsStorm frequency = 10 yrsTime to peak = 11.93 hrsTime interval = 2 min Hyd. volume = 20.792 cuft Curve number Drainage area = 2.390 ac= 85* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 4.11 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.620 x 98) + (1.770 x 80)] / 2.390



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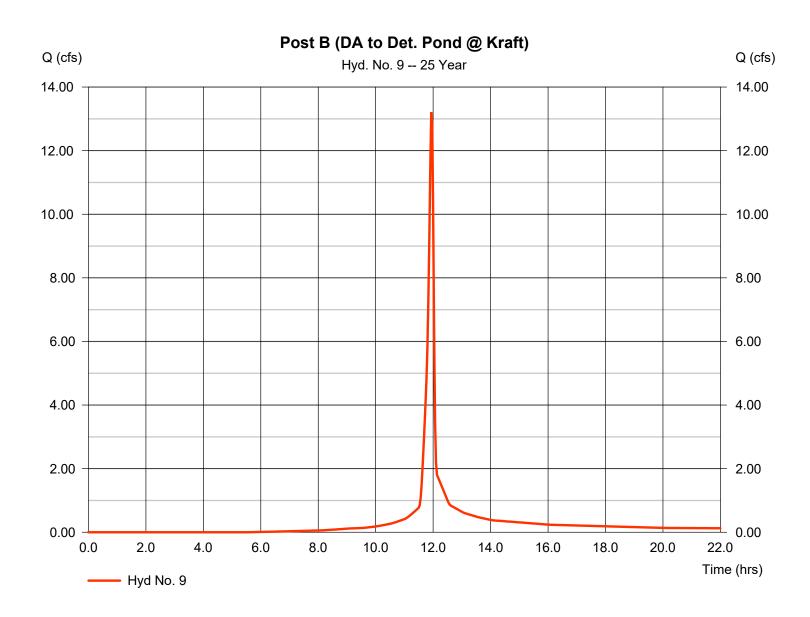
Wednesday, 01 / 31 / 2018

Hyd. No. 9

Post B (DA to Det. Pond @ Kraft)

Hydrograph type = SCS Runoff Peak discharge = 13.21 cfsStorm frequency = 25 yrs Time to peak $= 11.93 \, hrs$ Time interval = 2 min Hyd. volume = 27.394 cuft = 2.390 acCurve number Drainage area = 85* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 6.00 min = User = 5.00 inTotal precip. Distribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.620 x 98) + (1.770 x 80)] / 2.390



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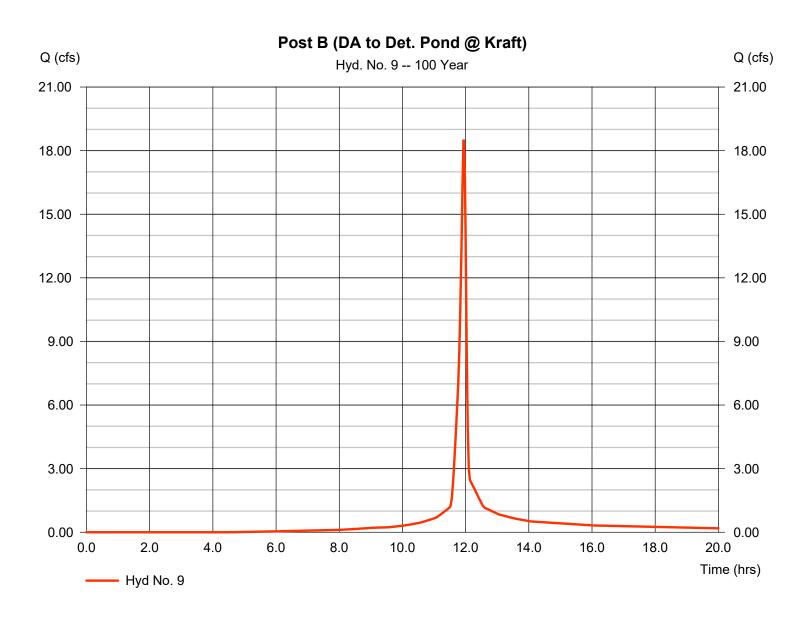
Wednesday, 01 / 31 / 2018

Hyd. No. 9

Post B (DA to Det. Pond @ Kraft)

Hydrograph type = SCS Runoff Peak discharge = 18.53 cfsStorm frequency = 100 yrsTime to peak $= 11.93 \, hrs$ Time interval = 2 min Hyd. volume = 39.154 cuft = 2.390 acCurve number Drainage area = 85* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 6.00 \, \text{min}$ = User Total precip. = 6.54 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = [(0.620 x 98) + (1.770 x 80)] / 2.390



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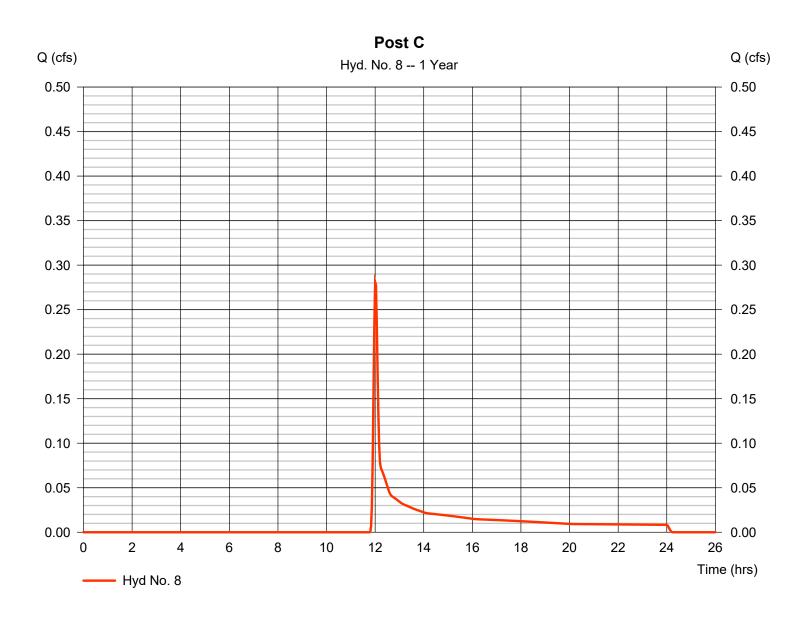
Wednesday, 01 / 31 / 2018

Hyd. No. 8

Post C

Hydrograph type = SCS Runoff Peak discharge = 0.281 cfsStorm frequency Time to peak = 12.00 hrs= 1 yrsTime interval = 2 min Hyd. volume = 893 cuft Curve number = 68* Drainage area = 0.830 acBasin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.20 min = User Total precip. = 2.28 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.390 \times 61) + (0.420 \times 74) + (0.020 \times 98)] / 0.830$



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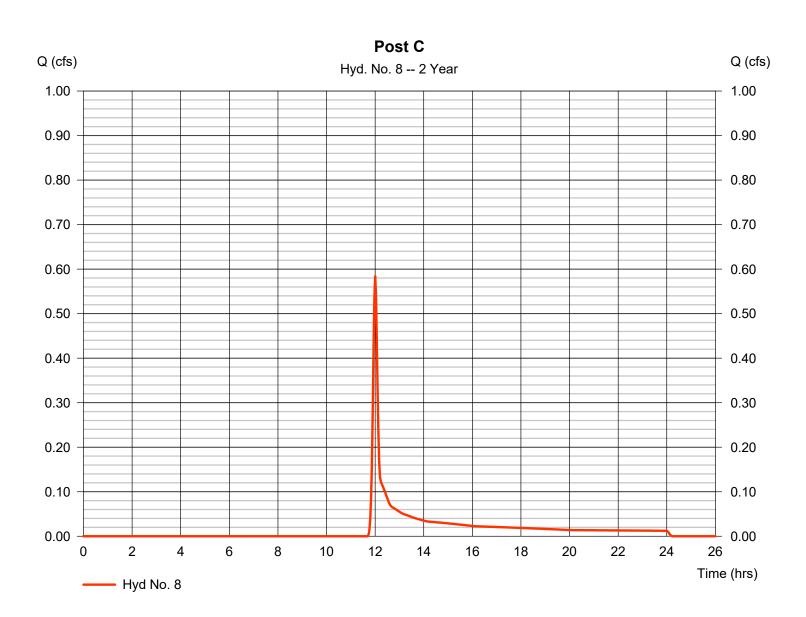
Wednesday, 01 / 31 / 2018

Hyd. No. 8

Post C

Hydrograph type = SCS Runoff Peak discharge = 0.585 cfsStorm frequency = 2 yrsTime to peak = 12.00 hrsTime interval = 2 min Hyd. volume = 1,527 cuftCurve number Drainage area = 0.830 ac= 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.20 min = User Total precip. = 2.76 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

^{*} Composite (Area/CN) = $[(0.390 \times 61) + (0.420 \times 74) + (0.020 \times 98)] / 0.830$



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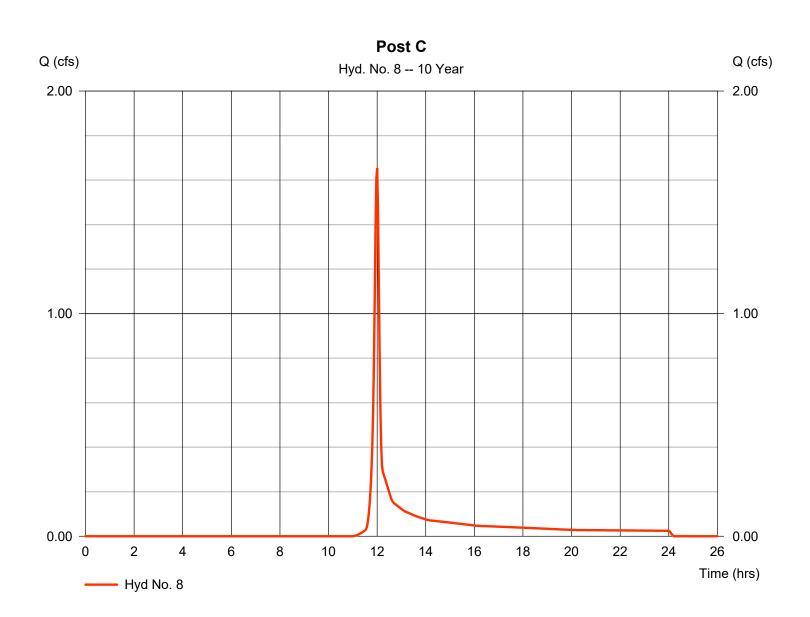
Wednesday, 01 / 31 / 2018

Hyd. No. 8

Post C

Hydrograph type = SCS Runoff Peak discharge = 1.653 cfsStorm frequency = 10 yrsTime to peak = 12.00 hrsTime interval = 2 min Hyd. volume = 3,842 cuftDrainage area = 0.830 acCurve number = 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 8.20 min = User Total precip. = 4.11 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.390 \times 61) + (0.420 \times 74) + (0.020 \times 98)] / 0.830$



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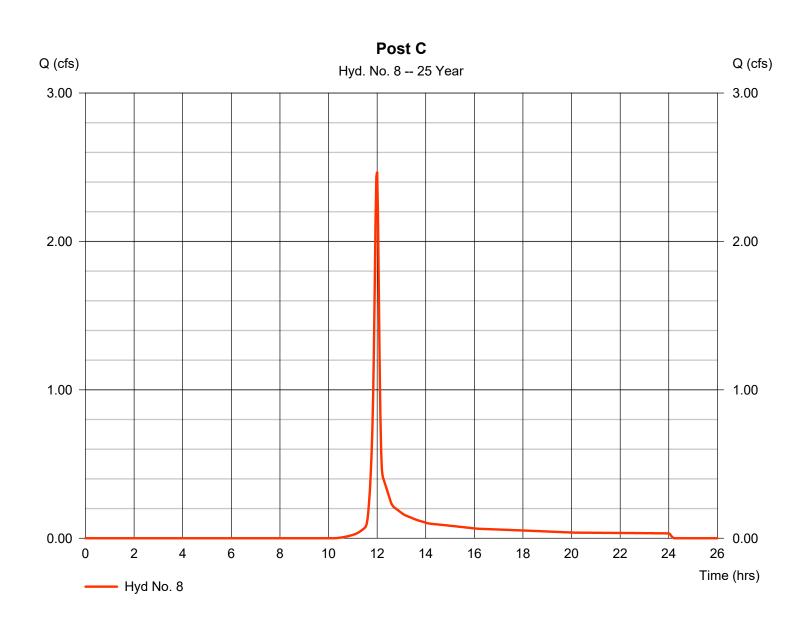
Wednesday, 01 / 31 / 2018

Hyd. No. 8

Post C

Hydrograph type = SCS Runoff Peak discharge = 2.469 cfsStorm frequency = 25 yrs Time to peak = 12.00 hrsTime interval = 2 min Hyd. volume = 5,663 cuftDrainage area = 0.830 acCurve number = 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 8.20 min Tc method = User Total precip. = 5.00 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.390 \times 61) + (0.420 \times 74) + (0.020 \times 98)] / 0.830$



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

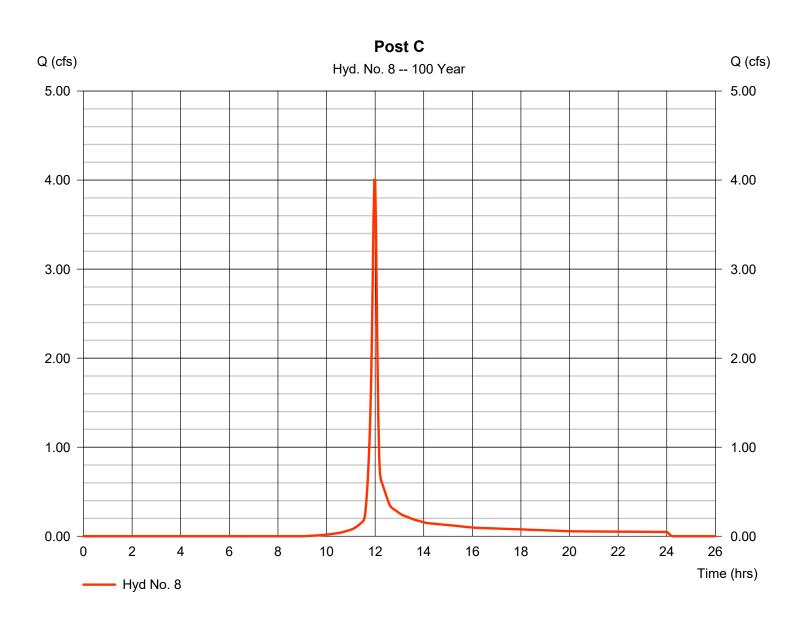
Wednesday, 01 / 31 / 2018

Hyd. No. 8

Post C

Hydrograph type = SCS Runoff Peak discharge = 4.001 cfsStorm frequency = 100 yrsTime to peak $= 11.97 \, hrs$ Time interval = 2 min Hyd. volume = 9,165 cuftDrainage area = 0.830 acCurve number = 68* Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) = 8.20 min Tc method = User Total precip. = 6.54 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

^{*} Composite (Area/CN) = $[(0.390 \times 61) + (0.420 \times 74) + (0.020 \times 98)] / 0.830$



APPENDIX E – ROUTING CALCULATIONS

(See attached Drawings)

Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Pond No. 5 - UG Det. (Storm Tech Chambers)

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	2152.00	n/a	0	0
0.50	2152.50	n/a	2,785	2,785
1.00	2153.00	n/a	4,381	7,166
1.50	2153.50	n/a	5,921	13,087
2.00	2154.00	n/a	5,824	18,911
2.50	2154.50	n/a	5,695	24,605
3.00	2155.00	n/a	5,530	30,135
3.50	2155.50	n/a	5,324	35,459
4.00	2156.00	n/a	5,065	40,525
4.50	2156.50	n/a	4,735	45,259
5.00	2157.00	n/a	4,194	49,453
5.50	2157.50	n/a	3,566	53,019
6.00	2158.00	n/a	2,852	55,871
6.50	2158.50	n/a	2,785	58,656
7.00	2159.00	n/a	1,392	60,048

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 24.00	3.00	12.00	0.00	Crest Len (ft)	= 4.00	0.00	0.00	0.00
Span (in)	= 24.00	3.00	12.00	0.00	Crest El. (ft)	= 2157.50	0.00	0.00	0.00
No. Barrels	= 1	1	1	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 2152.00	2152.00	2154.30	0.00	Weir Type	= Rect			
Length (ft)	= 50.00	0.10	0.10	0.00	Multi-Stage	= Yes	No	No	No
Slope (%)	= 2.00	0.10	0.10	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by)	Wet area)		
Multi-Stage	= n/a	Yes	Yes	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	2152.00	0.00	0.00	0.00		0.00						0.000
0.05	278	2152.05	0.01 ic	0.01 ic	0.00		0.00						0.005
0.10	557	2152.10	0.02 ic	0.02 ic	0.00		0.00						0.019
0.15	835	2152.15	0.04 ic	0.04 ic	0.00		0.00						0.040
0.20	1,114	2152.20	0.06 ic	0.06 ic	0.00		0.00						0.065
0.25	1,392	2152.25	0.08 ic	0.08 ic	0.00		0.00						0.084
0.30	1,671	2152.30	0.11 ic	0.10 ic	0.00		0.00						0.099
0.35	1,949	2152.35	0.12 ic	0.11 ic	0.00		0.00						0.111
0.40	2,228	2152.40	0.12 ic	0.12 ic	0.00		0.00						0.121
0.45	2,506	2152.45	0.13 ic	0.13 ic	0.00		0.00						0.132
0.50	2,785	2152.50	0.15 ic	0.14 ic	0.00		0.00						0.140
0.55	3,223	2152.55	0.15 ic	0.15 ic	0.00		0.00						0.149
0.60	3,661	2152.60	0.16 ic	0.16 ic	0.00		0.00						0.158
0.65	4,099	2152.65	0.17 ic	0.17 ic	0.00		0.00						0.166
0.70	4,537	2152.70	0.18 ic	0.17 ic	0.00		0.00						0.173
0.75	4,975	2152.75	0.18 ic	0.18 ic	0.00		0.00						0.181
0.80	5,414	2152.80	0.20 ic	0.19 ic	0.00		0.00						0.187
0.85	5,852	2152.85	0.20 ic	0.19 ic	0.00		0.00						0.194
0.90	6,290	2152.90	0.20 ic	0.20 ic	0.00		0.00						0.202
0.95	6,728	2152.95	0.22 ic	0.21 ic	0.00		0.00						0.207
1.00	7,166	2153.00	0.22 ic	0.21 ic	0.00		0.00						0.214
1.05	7,758	2153.05	0.22 ic	0.22 ic	0.00		0.00						0.220
1.10	8,350	2153.10	0.23 ic	0.23 ic	0.00		0.00						0.226
1.15	8,942	2153.15	0.24 ic	0.23 ic	0.00		0.00						0.231
1.20	9,534	2153.20	0.24 ic	0.24 ic	0.00		0.00						0.237
1.25	10,127	2153.25	0.24 ic	0.24 ic	0.00		0.00						0.243
1.30	10,719	2153.30	0.25 ic	0.25 ic	0.00		0.00						0.248
1.35	11,311	2153.35	0.27 ic	0.25 ic	0.00		0.00						0.253
1.40	11,903	2153.40	0.27 ic	0.26 ic	0.00		0.00						0.259

Continues on next page...

UG Det. (Storm Tech Chambers) Stage / Storage / Discharge Table

Stage /	Stage / Storage / Discharge Table												
Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
1.45	12,495	2153.45	0.27 ic	0.26 ic	0.00		0.00						0.264
1.50	13,087	2153.50	0.27 ic	0.27 ic	0.00		0.00						0.269
1.55	13,669	2153.55	0.27 ic	0.27 ic	0.00		0.00						0.274
1.60 1.65	14,252 14,834	2153.60 2153.65	0.29 ic 0.29 ic	0.28 ic 0.28 ic	0.00 0.00		0.00 0.00						0.279 0.284
1.70	15,416	2153.05	0.29 ic	0.29 ic	0.00		0.00						0.288
1.75	15,999	2153.75	0.29 ic	0.29 ic	0.00		0.00						0.293
1.80	16,581	2153.80	0.30 ic	0.30 ic	0.00		0.00						0.297
1.85	17,163	2153.85	0.32 ic	0.30 ic	0.00		0.00						0.302
1.90	17,746	2153.90	0.32 ic	0.31 ic	0.00		0.00						0.306
1.95	18,328	2153.95	0.32 ic	0.31 ic	0.00		0.00						0.311
2.00 2.05	18,911 19,480	2154.00 2154.05	0.32 ic 0.32 ic	0.32 ic 0.32 ic	0.00 0.00		0.00 0.00						0.315 0.320
2.10	20,050	2154.10	0.32 ic	0.32 ic	0.00		0.00						0.324
2.15	20,619	2154.15	0.35 ic	0.33 ic	0.00		0.00						0.328
2.20	21,189	2154.20	0.35 ic	0.33 ic	0.00		0.00						0.332
2.25	21,758	2154.25	0.35 ic	0.34 ic	0.00		0.00						0.336
2.30	22,327	2154.30	0.35 ic	0.34 ic	0.00 ic		0.00						0.340
2.35 2.40	22,897 23,466	2154.35 2154.40	0.35 ic 0.41 ic	0.34 ic 0.35 ic	0.01 ic 0.05 ic		0.00						0.355 0.394
2.45	24,036	2154.45	0.41 ic 0.45 ic	0.35 ic	0.03 ic		0.00						0.394
2.50	24,605	2154.50	0.56 ic	0.35 ic	0.18 ic		0.00						0.529
2.55	25,158	2154.55	0.64 ic	0.35 ic	0.26 ic		0.00						0.618
2.60	25,711	2154.60	0.73 ic	0.36 ic	0.37 ic		0.00						0.727
2.65	26,264	2154.65	0.89 ic	0.36 ic	0.50 ic		0.00						0.856
2.70	26,817	2154.70	1.01 ic	0.36 ic	0.65 ic		0.00						1.004
2.75 2.80	27,370 27,923	2154.75 2154.80	1.20 ic 1.34 ic	0.36 ic 0.36 ic	0.79 ic 0.96 ic		0.00 0.00						1.149 1.326
2.85	28,476	2154.85	1.49 ic	0.36 ic	1.13 ic		0.00						1.493
2.90	29,029	2154.90	1.73 ic	0.36 ic	1.30 ic		0.00						1.665
2.95	29,582	2154.95	1.90 ic	0.37 ic	1.50 ic		0.00						1.861
3.00	30,135	2155.00	2.08 ic	0.37 ic	1.69 ic		0.00						2.055
3.05	30,668	2155.05	2.28 ic	0.37 ic	1.87 ic		0.00						2.243
3.10 3.15	31,200 31,733	2155.10 2155.15	2.48 ic 2.61 ic	0.37 ic 0.37 ic	2.05 ic 2.24 ic		0.00 0.00						2.422 2.613
3.13	31,733	2155.15	2.81 ic	0.37 ic	2.41 ic		0.00						2.788
3.25	32,797	2155.25	2.94 ic	0.38 ic	2.56 ic		0.00						2.938
3.30	33,330	2155.30	3.05 ic	0.38 ic	2.67 ic		0.00						3.053
3.35	33,862	2155.35	3.27 ic	0.38 ic	2.80 ic		0.00						3.186
3.40	34,395	2155.40	3.39 ic	0.38 ic	2.93 ic		0.00						3.313
3.45 3.50	34,927 35,459	2155.45 2155.50	3.52 ic 3.64 ic	0.39 ic 0.39 ic	3.05 ic 3.16 ic		0.00 0.00						3.435 3.552
3.55	35,966	2155.55	3.67 ic	0.39 ic	3.10 ic		0.00						3.666
3.60	36,472	2155.60	3.78 ic	0.39 ic	3.38 ic		0.00						3.777
3.65	36,979	2155.65	3.90 ic	0.40 ic	3.49 ic		0.00						3.883
3.70	37,485	2155.70	4.03 ic	0.40 ic	3.59 ic		0.00						3.987
3.75	37,992	2155.75	4.17 ic	0.40 ic	3.69 ic		0.00						4.088
3.80 3.85	38,499 39,005	2155.80 2155.85	4.19 ic 4.30 ic	0.40 ic	3.78 ic 3.88 ic		0.00 0.00						4.186 4.283
3.90	39,512	2155.85	4.30 ic	0.41 ic 0.41 ic	3.97 ic		0.00						4.203
3.95	40,018	2155.95	4.47 ic	0.41 ic	4.06 ic		0.00						4.468
4.00	40,525	2156.00	4.58 ic	0.42 ic	4.14 ic		0.00						4.558
4.05	40,998	2156.05	4.72 ic	0.42 ic	4.23 ic		0.00						4.646
4.10	41,472	2156.10	4.73 ic	0.42 ic	4.31 ic		0.00						4.732
4.15	41,945	2156.15	4.87 ic	0.42 ic	4.39 ic		0.00						4.817
4.20 4.25	42,418 42,892	2156.20 2156.25	4.90 ic 5.01 ic	0.43 ic 0.43 ic	4.47 ic 4.55 ic		0.00 0.00						4.900 4.983
4.30	43,365	2156.30	5.16 ic	0.43 ic	4.63 ic		0.00						5.063
4.35	43,839	2156.35	5.16 ic	0.43 ic	4.71 ic		0.00						5.143
4.40	44,312	2156.40	5.31 ic	0.44 ic	4.78 ic		0.00						5.220
4.45	44,786	2156.45	5.31 ic	0.44 ic	4.86 ic		0.00						5.298
4.50	45,259 45,670	2156.50	5.46 ic	0.44 ic	4.93 ic		0.00						5.372
4.55 4.60	45,679 46,008	2156.55	5.46 ic	0.45 ic	5.00 ic		0.00						5.447 5.521
4.60 4.65	46,098 46,517	2156.60 2156.65	5.61 ic 5.61 ic	0.45 ic 0.45 ic	5.07 ic 5.14 ic		0.00 0.00						5.521 5.594
4.70	46,937	2156.70	5.76 ic	0.45 ic	5.14 ic		0.00						5.665
4.75	47,356	2156.75	5.76 ic	0.46 ic	5.28 ic		0.00						5.736
4.80	47,775	2156.80	5.91 ic	0.46 ic	5.35 ic		0.00						5.806
4.85	48,195	2156.85	5.91 ic	0.46 ic	5.41 ic		0.00						5.875
4.90	48,614	2156.90	5.94 ic	0.46 ic	5.48 ic		0.00						5.943
4.95 5.00	49,033 49,453	2156.95 2157.00	6.06 ic 6.08 ic	0.47 ic 0.47 ic	5.54 ic 5.61 ic		0.00 0.00						6.011 6.077
5.00	+3,+33	2101.00	0.0010	U.47 IC	J.U 1 IC		0.00						0.011

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UG Det. (Storm Tech Chambers) Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
π	curt	π	CTS	CIS	CIS	CIS	CTS	CTS	CTS	CTS	CIS	CIS	CTS
5.05	49.809	2157.05	6.22 ic	0.47 ic	5.67 ic		0.00						6.143
5.10	50,166	2157.10	6.22 ic	0.47 ic	5.73 ic		0.00						6.208
5.15	50,523	2157.15	6.37 ic	0.48 ic	5.80 ic		0.00						6.273
5.20	50,879	2157.20	6.37 ic	0.48 ic	5.86 ic		0.00						6.337
5.25	51,236	2157.25	6.40 ic	0.48 ic	5.92 ic		0.00						6.400
5.30	51,593	2157.30	6.53 ic	0.48 ic	5.98 ic		0.00						6.463
5.35	51,949	2157.35	6.53 ic	0.49 ic	6.04 ic		0.00						6.525
5.40	52,306	2157.40	6.69 ic	0.49 ic	6.10 ic		0.00						6.586
5.45	52,663	2157.45	6.69 ic	0.49 ic	6.16 ic		0.00						6.647
5.50	53,019	2157.50	6.71 ic	0.49 ic	6.21 ic		0.00						6.707
5.55	53,304	2157.55	7.00 ic	0.50 ic	6.27 ic		0.15						6.915
5.60	53,590	2157.60	7.32 ic	0.50 ic	6.33 ic		0.42						7.245
5.65	53,875	2157.65	7.66 ic	0.50 ic	6.38 ic		0.77						7.656
5.70	54,160	2157.70	8.13 ic	0.50 ic	6.44 ic		1.19						8.130
5.75	54,445	2157.75	8.76 ic	0.50 ic	6.49 ic		1.67						8.659
5.80	54,730	2157.80	9.24 ic	0.50 ic	6.55 ic		2.19						9.239
5.85	55,016	2157.85	9.86 ic	0.50 ic	6.60 ic		2.76						9.863
5.90	55,301	2157.90	10.59 ic	0.50 ic	6.66 ic		3.37						10.53
5.95	55,586	2157.95	11.29 ic	0.50 ic	6.71 ic		4.03						11.23
6.00	55,871	2158.00	11.97 ic	0.49 ic	6.76 ic		4.71						11.97
6.05	56,150	2158.05	12.77 ic	0.49 ic	6.82 ic		5.43						12.74
6.10	56,428	2158.10	13.56 ic	0.49 ic	6.87 ic		6.19						13.55
6.15	56,707	2158.15	14.39 ic	0.49 ic	6.92 ic		6.98						14.39
6.20	56,985	2158.20	15.26 ic	0.48 ic	6.97 ic		7.80						15.26
6.25	57,264	2158.25	16.16 ic	0.48 ic	7.02 ic		8.66						16.16
6.30	57,542	2158.30	17.09 ic	0.47 ic	7.07 ic		9.54						17.08
6.35	57,820	2158.35	18.04 ic	0.47 ic	7.12 ic		10.44						18.04
6.40	58,099	2158.40	19.02 ic	0.46 ic	7.17 ic		11.38						19.02
6.45	58,377	2158.45	20.02 ic	0.45 ic	7.22 ic		12.34						20.02
6.50	58,656	2158.50	20.93 ic	0.45 ic	7.16 ic		13.32						20.93
6.55	58,795	2158.55	21.82 ic	0.44 ic	7.04 ic		14.33						21.82
6.60	58,934	2158.60	22.72 ic	0.43 ic	6.91 ic		15.37						22.72
6.65	59,074	2158.65	23.63 ic	0.42 ic	6.77 ic		16.43						23.63
6.70	59,213	2158.70	24.55 ic	0.41 ic	6.62 ic		17.51						24.55
6.75	59,352	2158.75	25.48 ic	0.40 ic	6.45 ic		18.62						25.48
6.80	59,491	2158.80	26.41 ic	0.39 ic	6.27 ic		19.75						26.41
6.85	59,631	2158.85	27.35 ic	0.38 ic	6.07 ic		20.90						27.35
6.90	59,770	2158.90	28.30 ic	0.37 ic	5.86 ic		22.07						28.30
6.95	59,909	2158.95	29.24 ic	0.35 ic	5.62 ic		23.27						29.24
7.00	60,048	2159.00	30.18 ic	0.34 ic	5.37 ic		24.47						30.18

...End

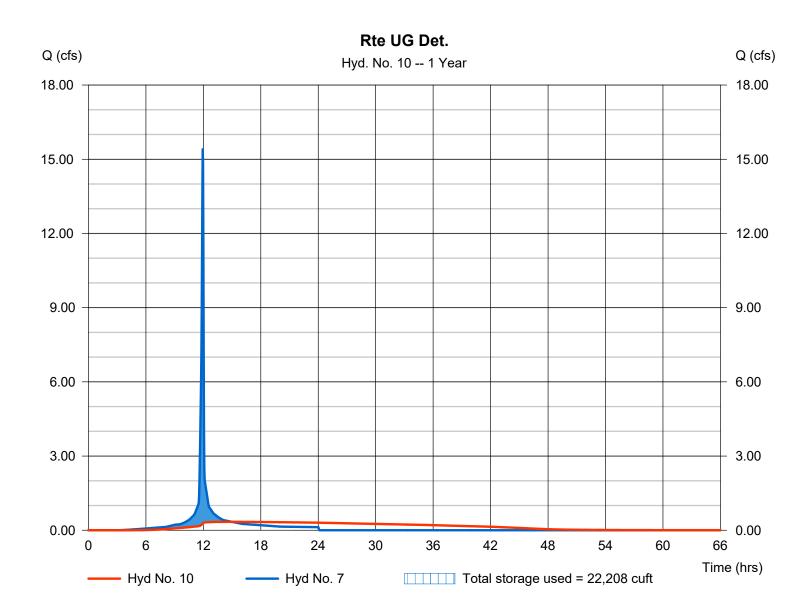
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 10

Rte UG Det.

Hydrograph type = Reservoir Peak discharge = 0.339 cfsStorm frequency Time to peak = 15.00 hrs= 1 yrsTime interval = 2 min Hyd. volume = 33,564 cuft = 7 - Post B (DA to UG Det) Max. Elevation Inflow hyd. No. = 2154.29 ft= UG Det. (Storm Tech Chambe Max. Storage Reservoir name = 22,208 cuft



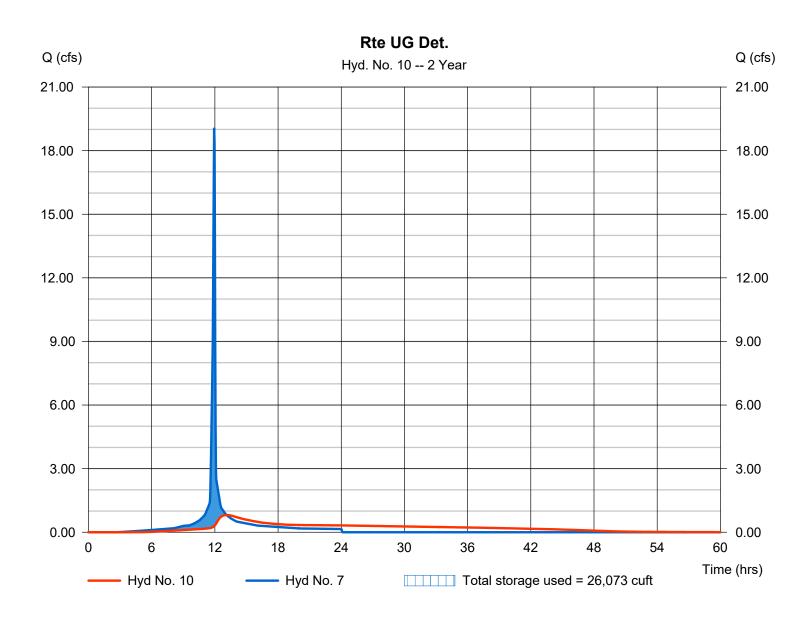
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 10

Rte UG Det.

Hydrograph type = Reservoir Peak discharge = 0.811 cfsStorm frequency = 2 yrsTime to peak = 13.10 hrsTime interval = 2 min Hyd. volume = 42,116 cuft = 7 - Post B (DA to UG Det) Max. Elevation = 2154.63 ftInflow hyd. No. = UG Det. (Storm Tech Chambe Max. Storage Reservoir name = 26,073 cuft



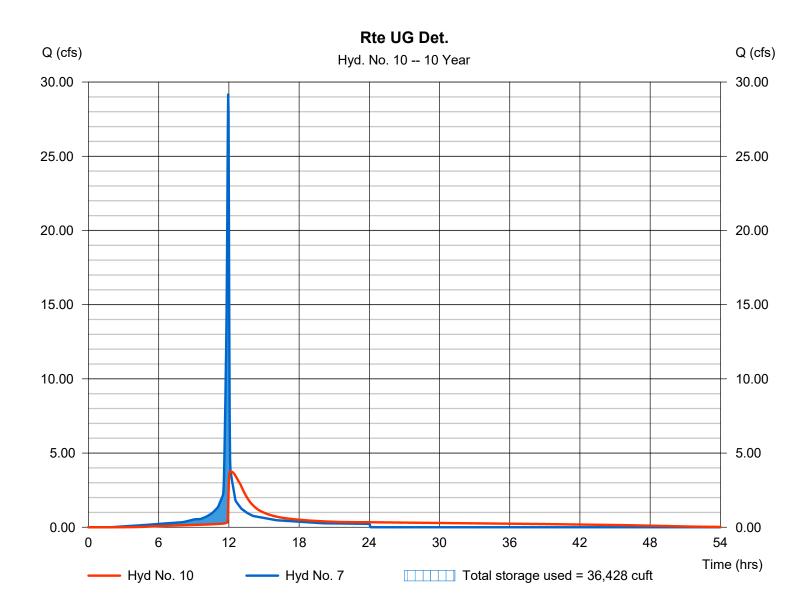
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 10

Rte UG Det.

Hydrograph type = Reservoir Peak discharge = 3.767 cfsStorm frequency = 10 yrsTime to peak $= 12.13 \, hrs$ Time interval = 2 min Hyd. volume = 66,385 cuft Max. Elevation Inflow hyd. No. = 7 - Post B (DA to UG Det) $= 2155.60 \, \text{ft}$ = UG Det. (Storm Tech Chambe Max. Storage Reservoir name = 36,428 cuft



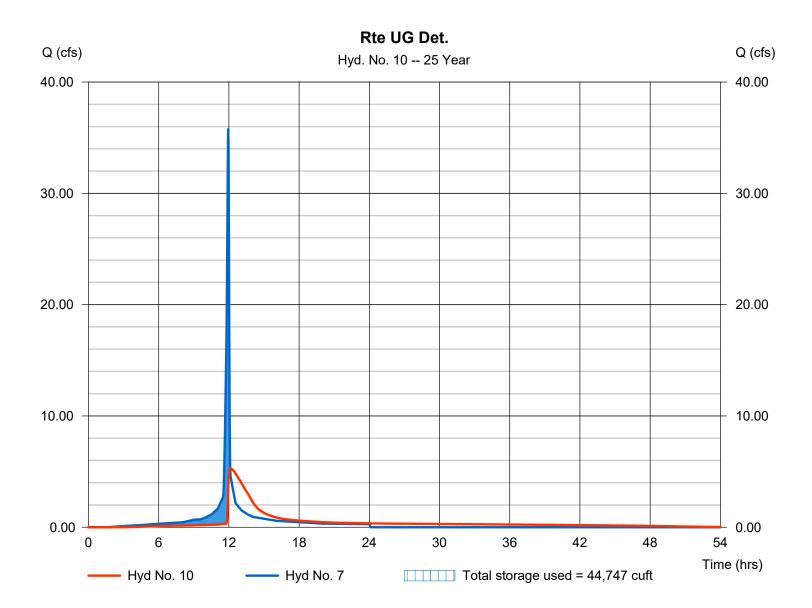
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 10

Rte UG Det.

Hydrograph type = Reservoir Peak discharge = 5.291 cfsStorm frequency = 25 yrsTime to peak = 12.10 hrsTime interval = 2 min Hyd. volume = 82,470 cuft= 7 - Post B (DA to UG Det) Max. Elevation Inflow hyd. No. = 2156.45 ft = UG Det. (Storm Tech Chambe Max. Storage Reservoir name = 44,747 cuft



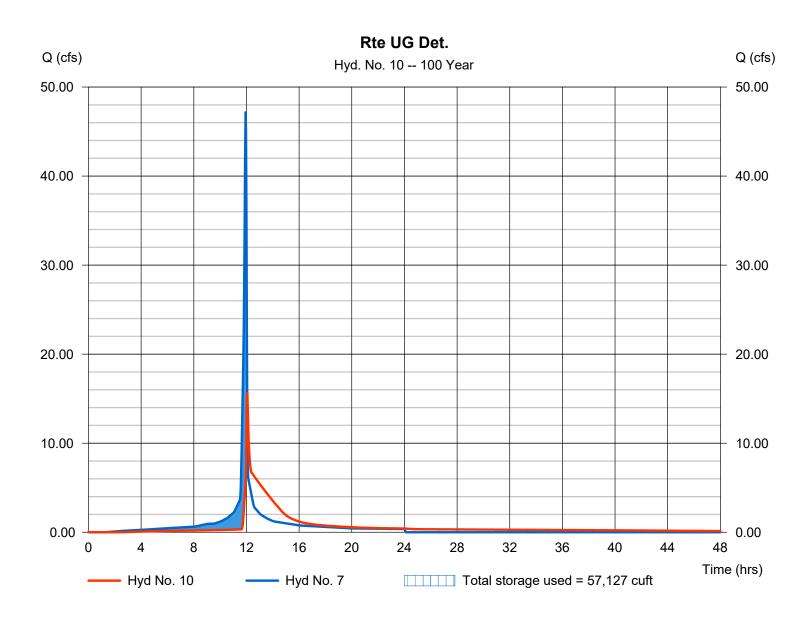
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 10

Rte UG Det.

Hydrograph type = Reservoir Peak discharge = 15.72 cfsStorm frequency Time to peak = 12.07 hrs= 100 yrsTime interval = 2 min Hyd. volume = 110,375 cuftMax. Elevation Inflow hyd. No. = 7 - Post B (DA to UG Det) = 2158.27 ft= UG Det. (Storm Tech Chambe Max. Storage Reservoir name = 57,127 cuft



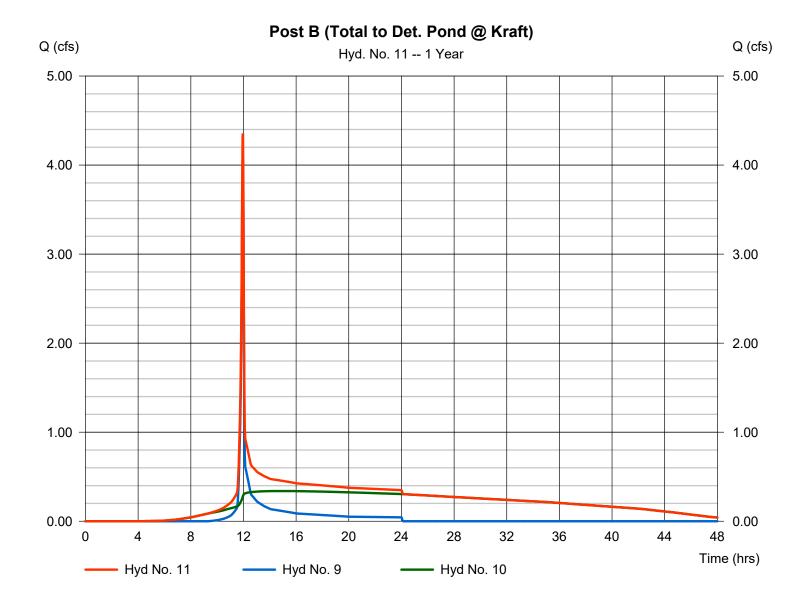
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 11

Post B (Total to Det. Pond @ Kraft)

= 4.355 cfsHydrograph type = Combine Peak discharge Storm frequency = 1 yrsTime to peak $= 11.97 \, hrs$ Time interval = 2 min Hyd. volume = 41,746 cuft = 9, 10 Contrib. drain. area = 2.390 acInflow hyds.



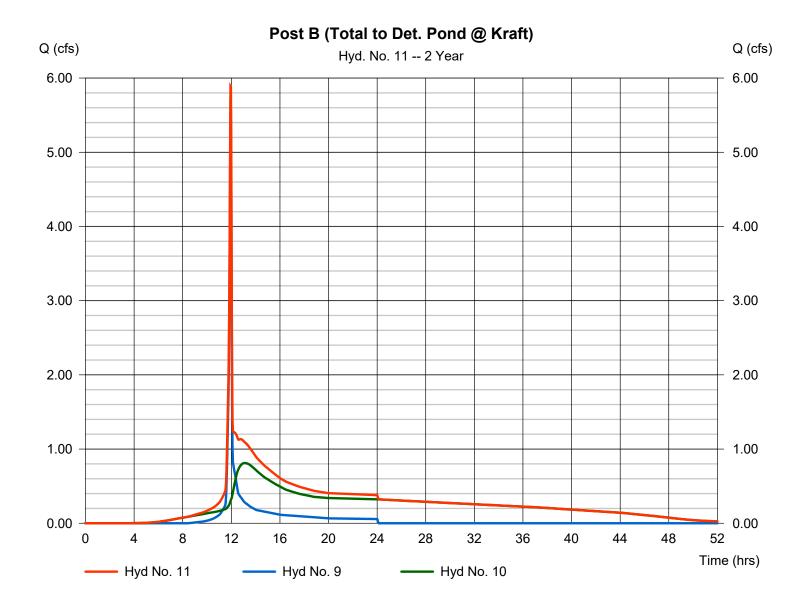
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Wednesday, 01 / 31 / 2018

Hyd. No. 11

Post B (Total to Det. Pond @ Kraft)

Hydrograph type = Combine Peak discharge = 5.891 cfsTime to peak Storm frequency = 2 yrs $= 11.93 \, hrs$ Time interval = 2 min Hyd. volume = 53,412 cuft= 9, 10 Contrib. drain. area Inflow hyds. = 2.390 ac



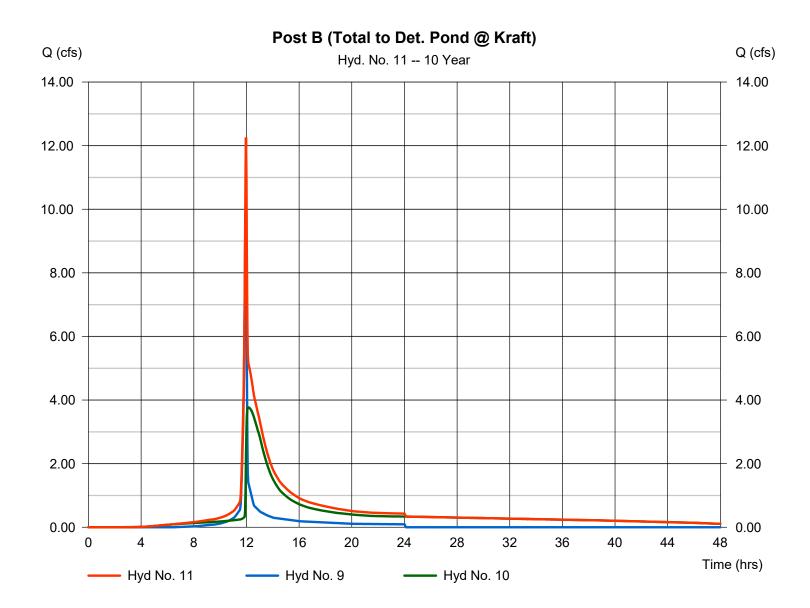
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Wednesday, 01 / 31 / 2018

Hyd. No. 11

Post B (Total to Det. Pond @ Kraft)

Hydrograph type = Combine Peak discharge = 12.26 cfsTime to peak Storm frequency = 10 yrs $= 11.97 \, hrs$ Time interval = 2 min Hyd. volume = 87,176 cuft Inflow hyds. = 9, 10 Contrib. drain. area = 2.390 ac



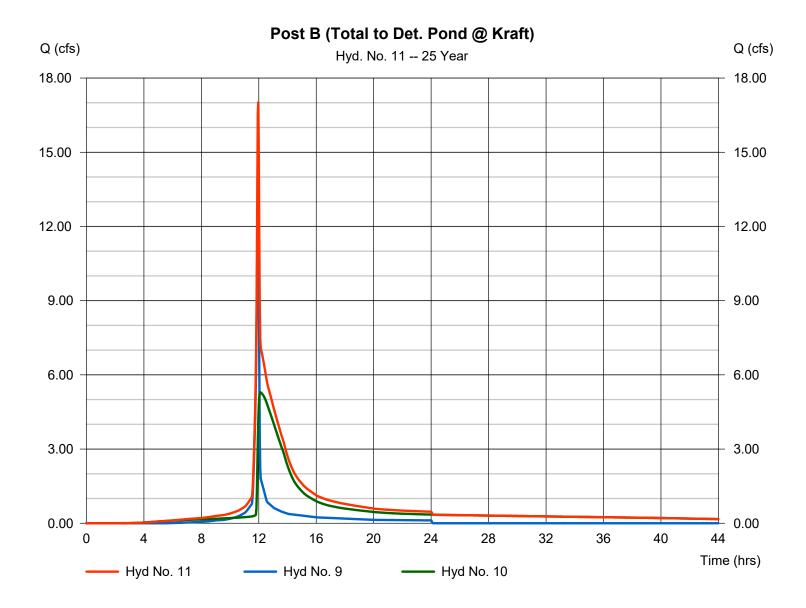
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 11

Post B (Total to Det. Pond @ Kraft)

= Combine Hydrograph type Peak discharge = 17.04 cfsTime to peak Storm frequency = 25 yrs $= 11.97 \, hrs$ Time interval = 2 min Hyd. volume = 109,864 cuft Inflow hyds. = 9, 10 Contrib. drain. area = 2.390 ac



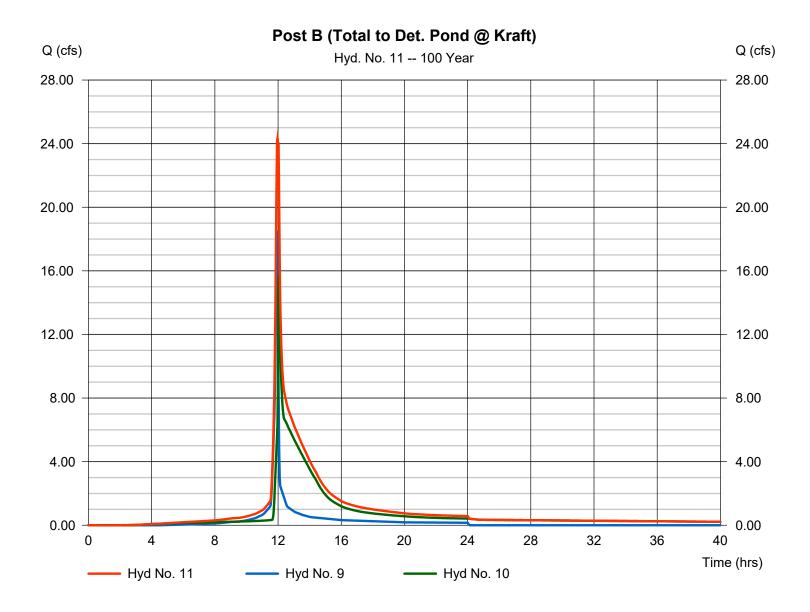
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 11

Post B (Total to Det. Pond @ Kraft)

= Combine Hydrograph type Peak discharge = 24.32 cfsTime to peak Storm frequency = 100 yrs $= 11.97 \, hrs$ Time interval = 2 min Hyd. volume = 149,530 cuftInflow hyds. = 9, 10 Contrib. drain. area = 2.390 ac



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Thursday, 02 / 1 / 2018

Pond No. 3 - New Det. Pond @ Kraft

Pond Data

N-Value

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 2426.00 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	2426.00	2,060	0	0
2.00	2428.00	4,747	6,622	6,622
4.00	2430.00	9,682	14,138	20,760
6.00	2432.00	13,625	23,193	43,952

Culvert / Orifice Structures Weir Structures [B] [C] [PrfRsr] [A] [B] [C] [D] [A] = 24.00 Rise (in) 2.00 4.00 0.00 Crest Len (ft) = 12.56 0.00 0.00 0.00 Span (in) = 24.002.00 4.00 0.00 Crest El. (ft) = 2430.30 0.00 0.00 0.00 No. Barrels = 1 0 Weir Coeff. = 3.33 3.33 3.33 3.33 1 1 Invert El. (ft) = 2426.00 2426.00 2429.80 0.00 Weir Type = 1 = 20.000.10 0.10 0.00 Multi-Stage Length (ft) = Yes No No No Slope (%) = 1.00 0.10 0.10 n/a

 Orifice Coeff.
 = 0.60
 0.60
 0.60
 Exfil.(in/hr)
 = 0.000 (by Wet area)

 Multi-Stage
 = n/a
 Yes
 Yes
 No
 TW Elev. (ft)
 = 0.00

n/a

.013

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stane	/ Storage	/ Discharge	Table
Staue	JULIAUE	/ DISCHALUE	Iable

= .013

.013

Stage	Storage	Elevation	Clv A	Clv B	Clv C	PrfRsr	Wr A	Wr B	Wr C	Wr D	Exfil	User	Total
ft	cuft	ft	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs	cfs
0.00	0	2426.00	0.00	0.00	0.00		0.00						0.000
0.20	662	2426.20	0.04 ic	0.04 ic	0.00		0.00						0.036
0.40	1,324	2426.40	0.06 ic	0.06 ic	0.00		0.00						0.058
0.60	1,987	2426.60	0.07 ic	0.07 ic	0.00		0.00						0.074
0.80	2,649	2426.80	0.09 ic	0.09 ic	0.00		0.00						0.087
1.00	3,311	2427.00	0.11 ic	0.10 ic	0.00		0.00						0.098
1.20	3,973	2427.20	0.12 ic	0.11 ic	0.00		0.00						0.109
1.40	4,635	2427.40	0.12 ic	0.12 ic	0.00		0.00						0.118
1.60	5,298	2427.60	0.13 ic	0.13 ic	0.00		0.00						0.127
1.80	5,960	2427.80	0.14 ic	0.14 ic	0.00		0.00						0.135
2.00	6,622	2428.00	0.15 ic	0.14 ic	0.00		0.00						0.143
2.20	8,036	2428.20	0.15 ic	0.15 ic	0.00		0.00						0.150
2.40	9,450	2428.40	0.16 ic	0.16 ic	0.00		0.00						0.157
2.60	10,863	2428.60	0.16 ic	0.16 ic	0.00		0.00						0.164
2.80	12,277	2428.80	0.18 ic	0.17 ic	0.00		0.00						0.171
3.00	13,691	2429.00	0.18 ic	0.18 ic	0.00		0.00						0.177
3.20	15,105	2429.20	0.18 ic	0.18 ic	0.00		0.00						0.183
3.40	16,518	2429.40	0.20 ic	0.19 ic	0.00		0.00						0.189
3.60	17,932	2429.60	0.20 ic	0.19 ic	0.00		0.00						0.194
3.80	19,346	2429.80	0.20 ic	0.20 ic	0.00		0.00						0.200
4.00	20,760	2430.00	0.29 ic	0.20 ic	0.08 ic		0.00						0.288
4.20	23,079	2430.20	0.41 ic	0.21 ic	0.20 ic		0.00						0.412
4.40	25,398	2430.40	1.81 ic	0.21 ic	0.28 ic		1.32						1.802
4.60	27,717	2430.60	7.44 oc	0.18 ic	0.33 ic		6.87						7.391
4.80	30,037	2430.80	15.33 oc	0.16 ic	0.38 ic		14.79						15.33
5.00	32,356	2431.00	25.04 ic	0.12 ic	0.43 ic		24.49						25.04
5.20	34,675	2431.20	29.47 ic	0.07 ic	0.27 ic		29.14 s						29.47
5.40	36,994	2431.40	30.86 ic	0.05 ic	0.20 ic		30.60 s						30.85
5.60	39,314	2431.60	31.88 ic	0.04 ic	0.17 ic		31.67 s						31.88
5.80	41,633	2431.80	32.76 ic	0.03 ic	0.14 ic		32.57 s						32.75
6.00	43,952	2432.00	33.55 ic	0.03 ic	0.12 ic		33.40 s						33.54

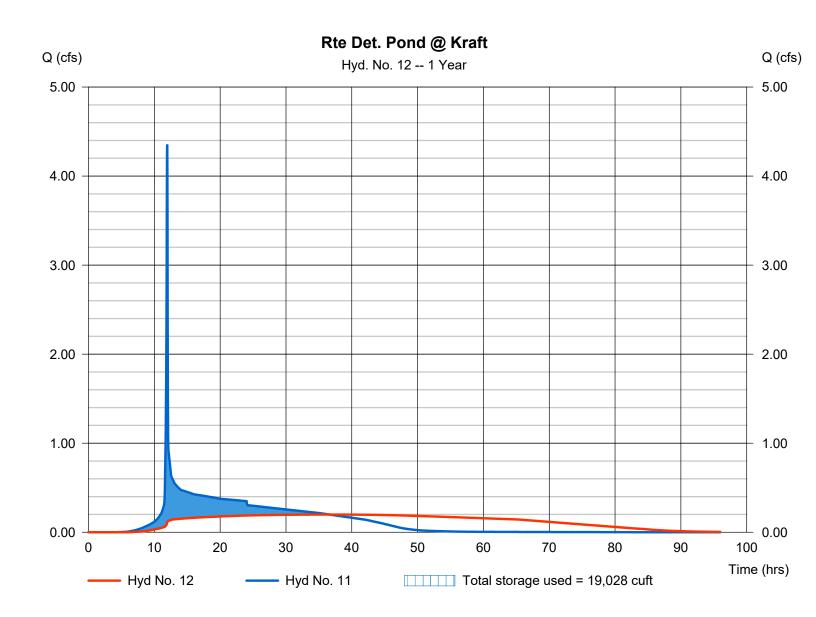
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Thursday, 02 / 1 / 2018

Hyd. No. 12

Rte Det. Pond @ Kraft

Hydrograph type = Reservoir Peak discharge = 0.199 cfsStorm frequency Time to peak $= 36.63 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 41,677 cuft Inflow hyd. No. = 11 - Post B (Total to Det. Pondly@akk.#alfe)vation = 2429.76 ft = New Det. Pond @ Kraft Reservoir name Max. Storage = 19,028 cuft



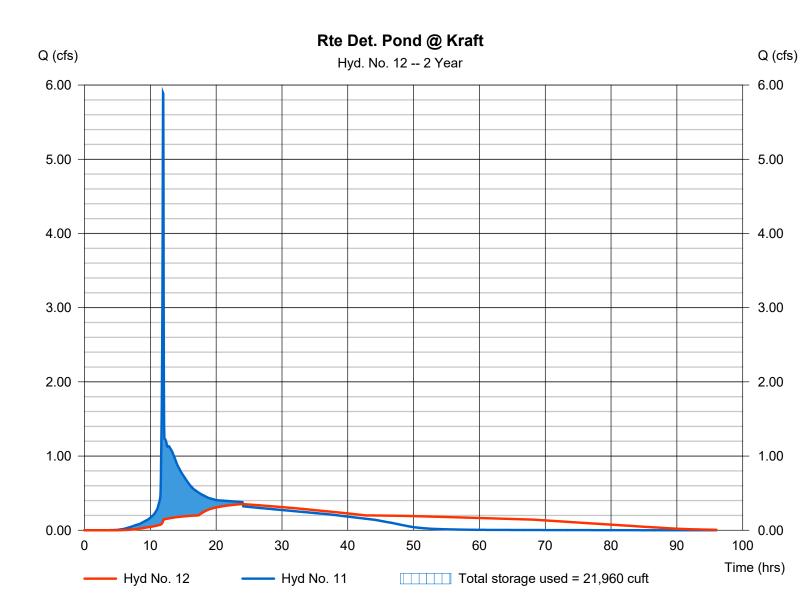
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Thursday, 02 / 1 / 2018

Hyd. No. 12

Rte Det. Pond @ Kraft

Hydrograph type = Reservoir Peak discharge = 0.352 cfsStorm frequency Time to peak = 24.07 hrs= 2 yrsTime interval = 2 min Hyd. volume = 53,293 cuft Inflow hyd. No. = 11 - Post B (Total to Det. Pondly@akk.#alfe)vation = 2430.10 ft= New Det. Pond @ Kraft Reservoir name Max. Storage = 21,960 cuft



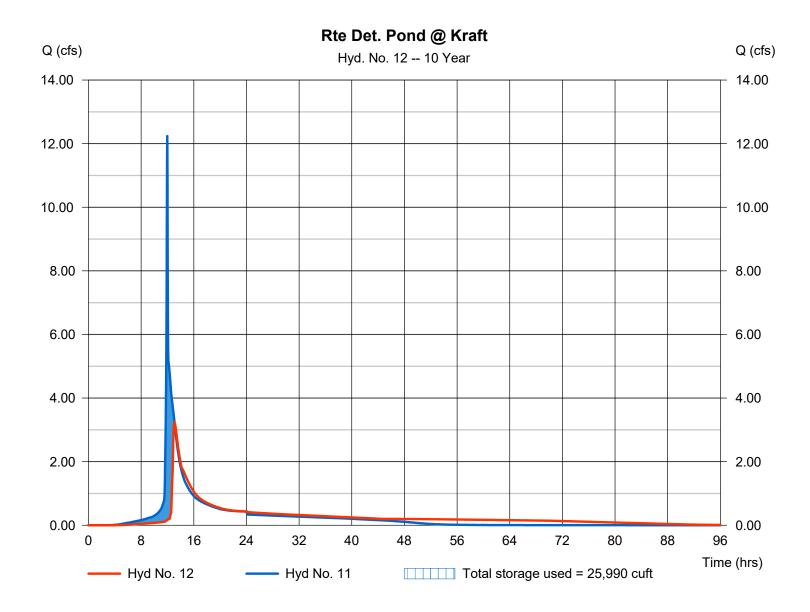
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Thursday, 02 / 1 / 2018

Hyd. No. 12

Rte Det. Pond @ Kraft

Hydrograph type = Reservoir Peak discharge = 3.229 cfsStorm frequency = 10 yrsTime to peak $= 13.07 \, hrs$ Time interval = 2 min Hyd. volume = 87,008 cuft Inflow hyd. No. = 11 - Post B (Total to Det. Pondly@akk.#alfe)vation = 2430.45 ft = New Det. Pond @ Kraft Reservoir name Max. Storage = 25,990 cuft



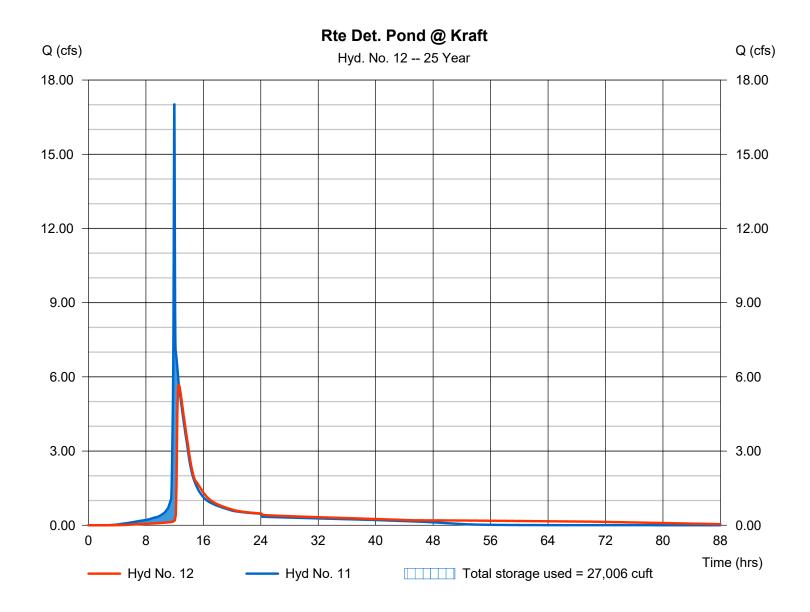
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Thursday, 02 / 1 / 2018

Hyd. No. 12

Rte Det. Pond @ Kraft

Hydrograph type = Reservoir Peak discharge = 5.676 cfsStorm frequency = 25 yrsTime to peak $= 12.57 \, hrs$ Time interval = 2 min Hyd. volume = 109,671 cuftInflow hyd. No. = 11 - Post B (Total to Det. Pondly@akk.#alfe)vation = 2430.54 ft= New Det. Pond @ Kraft Reservoir name Max. Storage = 27,006 cuft



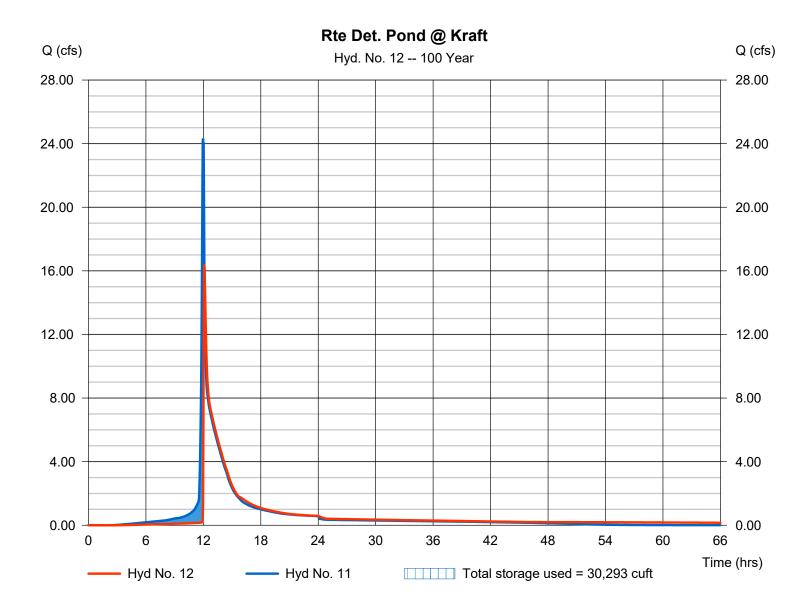
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Thursday, 02 / 1 / 2018

Hyd. No. 12

Rte Det. Pond @ Kraft

Hydrograph type = Reservoir Peak discharge $= 16.41 \, \text{cfs}$ Storm frequency Time to peak = 12.10 hrs= 100 yrsTime interval = 2 min Hyd. volume = 149,310 cuftInflow hyd. No. = 11 - Post B (Total to Det. Pondly@akk.#alfe)vation = 2430.82 ft = New Det. Pond @ Kraft Reservoir name Max. Storage = 30,293 cuft



APPENDIX F – 2018 POST DEVELOPMENT FACILITY #4 VERIFICATION

(See attached Drawings)

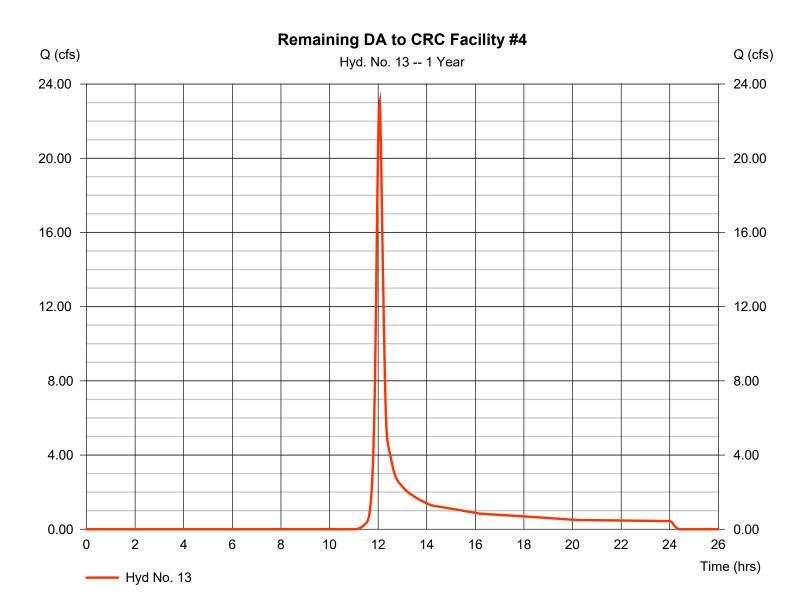
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 13

Remaining DA to CRC Facility #4

Hydrograph type = SCS Runoff Peak discharge = 23.02 cfsStorm frequency Time to peak = 12.07 hrs= 1 yrsTime interval = 2 min Hyd. volume = 68,055 cuft= 27.720 acCurve number Drainage area = 79 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = 14.00 min = User Total precip. = 2.28 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

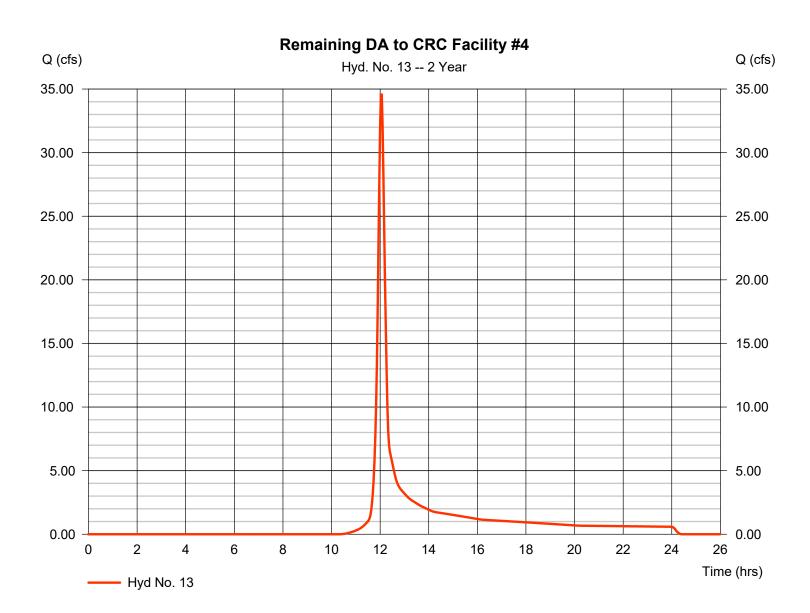
Hyd. No. 13

Remaining DA to CRC Facility #4

Hydrograph type Peak discharge = SCS Runoff = 34.57 cfsStorm frequency = 2 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 99,694 cuft Drainage area = 27.720 acCurve number = 79

Hydraulic length Basin Slope = 0.0 %= 0 ft

Tc method Time of conc. (Tc) = 14.00 min = User Total precip. = 2.76 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

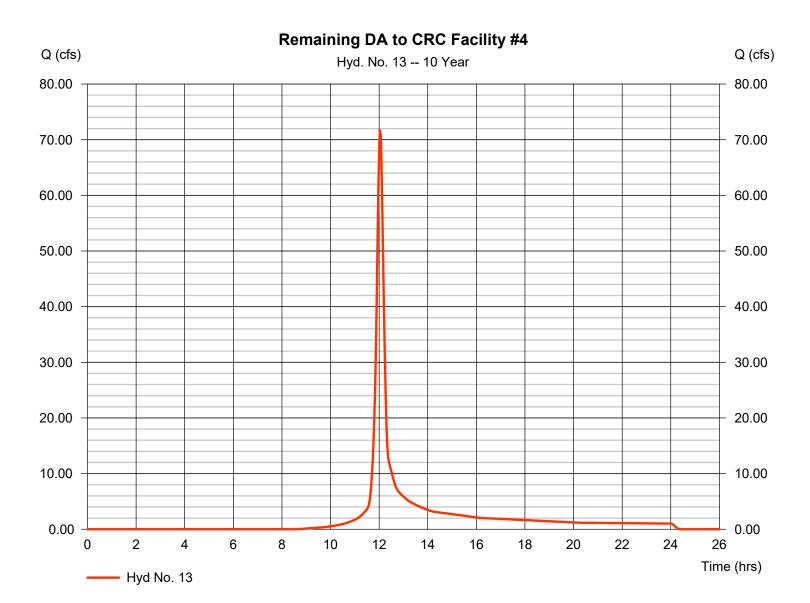
Wednesday, 01 / 31 / 2018

Hyd. No. 13

Remaining DA to CRC Facility #4

Peak discharge = 71.77 cfsHydrograph type = SCS Runoff Storm frequency = 10 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 201,430 cuft Drainage area = 27.720 acCurve number = 79

Total precip. = 4.11 in Distribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

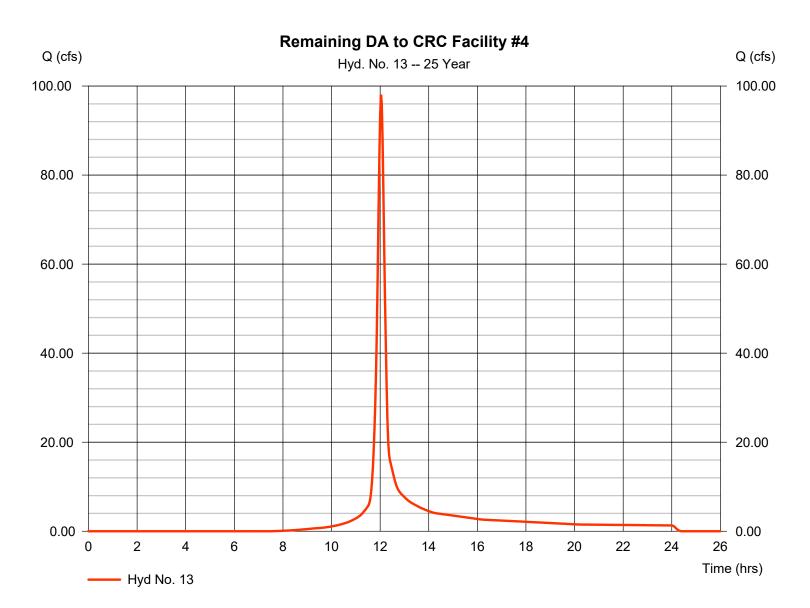
Hyd. No. 13

Remaining DA to CRC Facility #4

Hydrograph type= SCS RunoffPeak discharge= 98.04 cfsStorm frequency= 25 yrsTime to peak= 12.03 hrsTime interval= 2 minHyd. volume= 274,864 cuft

Drainage area = 27.720 ac Curve number = 79 Basin Slope = 0.0 % Hydraulic length = 0 ft

Tc method = User Time of conc. (Tc) = 14.00 min
Total precip. = 5.00 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

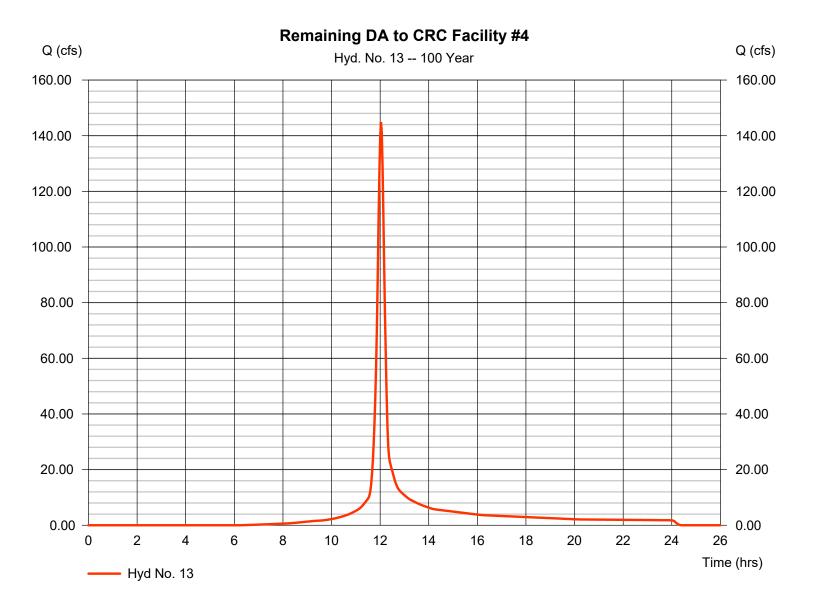
Wednesday, 01 / 31 / 2018

Hyd. No. 13

Remaining DA to CRC Facility #4

Hydrograph type = SCS Runoff Peak discharge = 144.84 cfsStorm frequency = 100 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 408,665 cuft Drainage area = 27.720 acCurve number = 79 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = 14.00 min = User

Total precip. = 6.54 in Distribution = Type II
Storm duration = 24 hrs Shape factor = 484



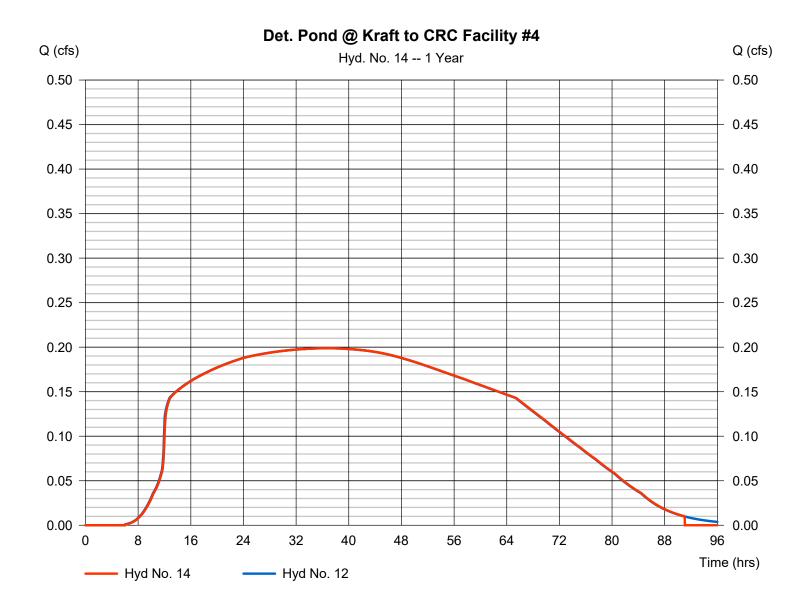
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 14

Det. Pond @ Kraft to CRC Facility #4

Hydrograph type	= Reach	Peak discharge	= 0.199 cfs
Storm frequency	= 1 yrs	Time to peak	= 36.73 hrs
Time interval	= 2 min	Hyd. volume	= 41,562 cuft
Inflow hyd. No.	= 12 - Rte Det. Pond @ Kraft	Section type	= Trapezoidal
Reach length	= 425.0 ft	Channel slope	= 2.5 %
Manning's n	= 0.025	Bottom width	= 10.0 ft
Side slope	= 4.0:1	Max. depth	= 1.5 ft
Rating curve x	= 2.029	Rating curve m	= 1.315
Ave. velocity	= 1.16 ft/s	Routing coeff.	= 0.3552



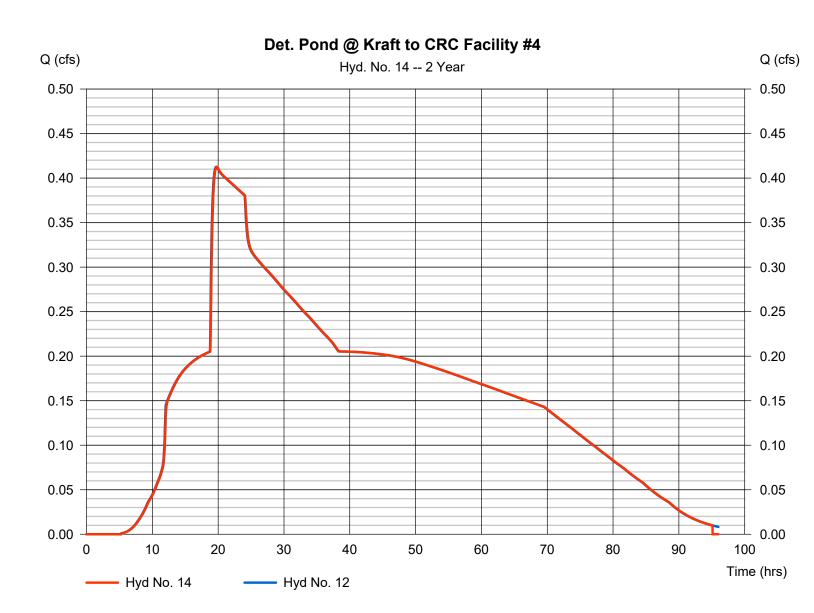
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Wednesday, 01 / 31 / 2018

Hyd. No. 14

Det. Pond @ Kraft to CRC Facility #4

Hydrograph type	= Reach	Peak discharge	= 0.412 cfs
Storm frequency	= 2 yrs	Time to peak	= 19.80 hrs
Time interval	= 2 min	Hyd. volume	= 53,228 cuft
Inflow hyd. No.	= 12 - Rte Det. Pond @ Kraft	Section type	= Trapezoidal
Reach length	= 425.0 ft	Channel slope	= 2.5 %
Manning's n	= 0.025	Bottom width	= 10.0 ft
Side slope	= 4.0:1	Max. depth	= 1.5 ft
Rating curve x	= 2.029	Rating curve m	= 1.315
Ave. velocity	= 1.39 ft/s	Routing coeff.	= 0.4092



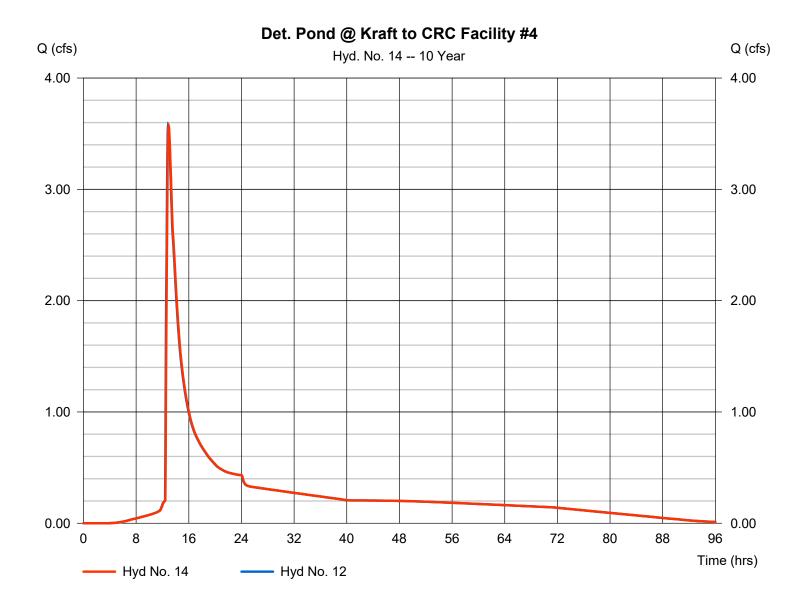
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Wednesday, 01 / 31 / 2018

Hyd. No. 14

Det. Pond @ Kraft to CRC Facility #4

Hydrograph type	= Reach	Peak discharge	= 3.570 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.93 hrs
Time interval	= 2 min	Hyd. volume	= 86,959 cuft
Inflow hyd. No.	= 12 - Rte Det. Pond @ Kraft	Section type	= Trapezoidal
Reach length	= 425.0 ft	Channel slope	= 2.5 %
Manning's n	= 0.025	Bottom width	= 10.0 ft
Side slope	= 4.0:1	Max. depth	= 1.5 ft
Rating curve x	= 2.029	Rating curve m	= 1.315
Ave. velocity	= 2.32 ft/s	Routing coeff.	= 0.6028



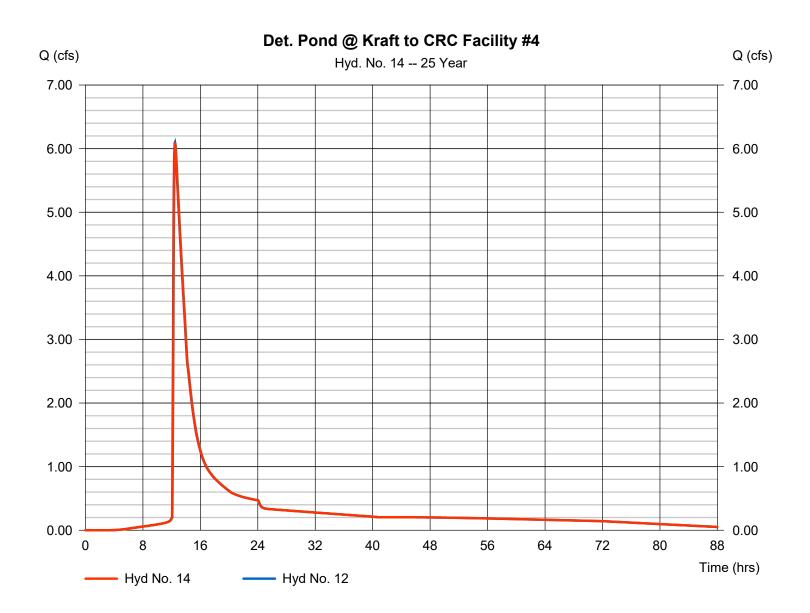
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Wednesday, 01 / 31 / 2018

Hyd. No. 14

Det. Pond @ Kraft to CRC Facility #4

Hydrograph type	= Reach	Peak discharge	= 6.080 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.50 hrs
Time interval	= 2 min	Hyd. volume	= 109,616 cuft
Inflow hyd. No.	= 12 - Rte Det. Pond @ Kraft	Section type	Trapezoidal
Reach length	= 425.0 ft	Channel slope	= 2.5 %
Manning's n	= 0.025	Bottom width	= 10.0 ft
Side slope	= 4.0:1	Max. depth	= 1.5 ft
Rating curve x	= 2.029	Rating curve m	= 1.315
Ave. velocity	= 2.64 ft/s	Routing coeff.	= 0.6577



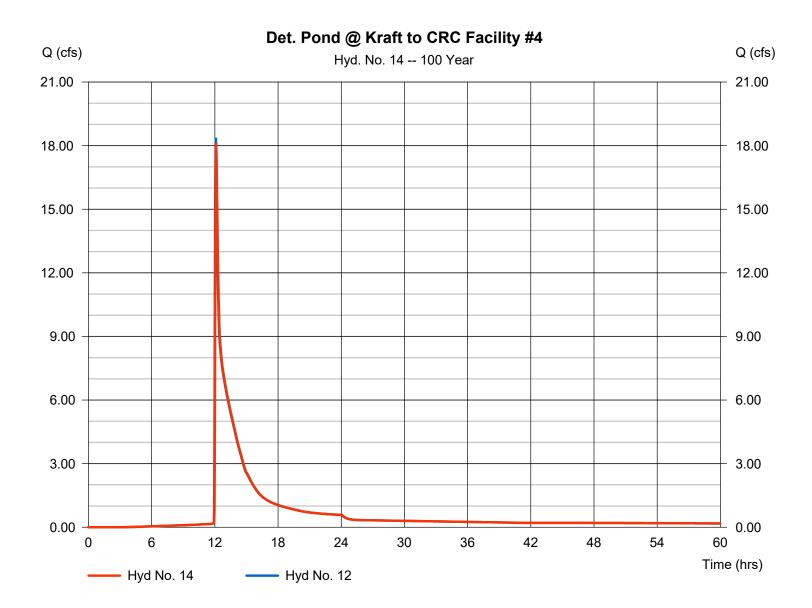
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Wednesday, 01 / 31 / 2018

Hyd. No. 14

Det. Pond @ Kraft to CRC Facility #4

Hydrograph type	= Reach	Peak discharge	= 18.13 cfs
Storm frequency	= 100 yrs	Time to peak	= 12.13 hrs
Time interval	= 2 min	Hyd. volume	= 149,247 cuft
Inflow hyd. No.	= 12 - Rte Det. Pond @ Kraft	Section type	Trapezoidal
Reach length	= 425.0 ft	Channel slope	= 2.5 %
Manning's n	= 0.025	Bottom width	= 10.0 ft
Side slope	= 4.0:1	Max. depth	= 1.5 ft
Rating curve x	= 2.029	Rating curve m	= 1.315
Ave. velocity	= 3.44 ft/s	Routing coeff.	= 0.7790



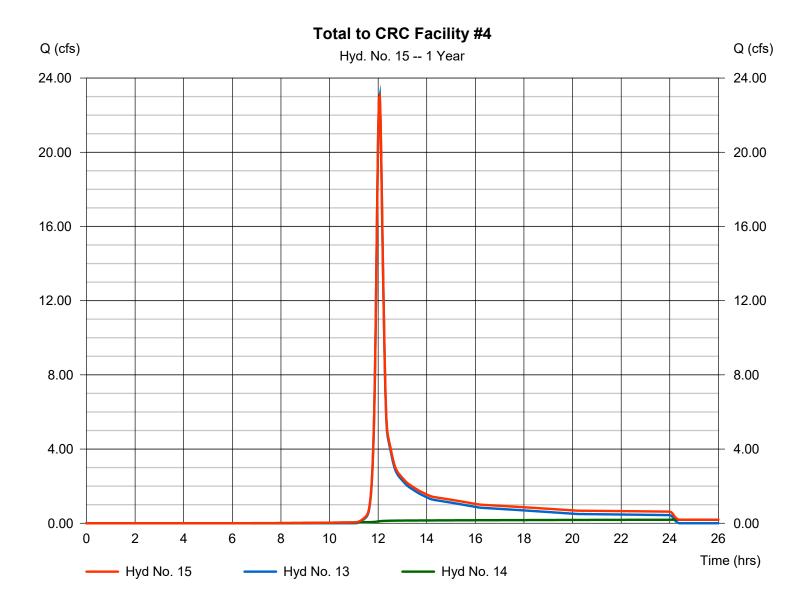
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Wednesday, 01 / 31 / 2018

Hyd. No. 15

Total to CRC Facility #4

Hydrograph type = Combine Peak discharge = 23.13 cfsTime to peak Storm frequency = 1 yrs= 12.07 hrsTime interval = 2 min Hyd. volume = 109,617 cuft Inflow hyds. Contrib. drain. area = 27.720 ac= 13, 14



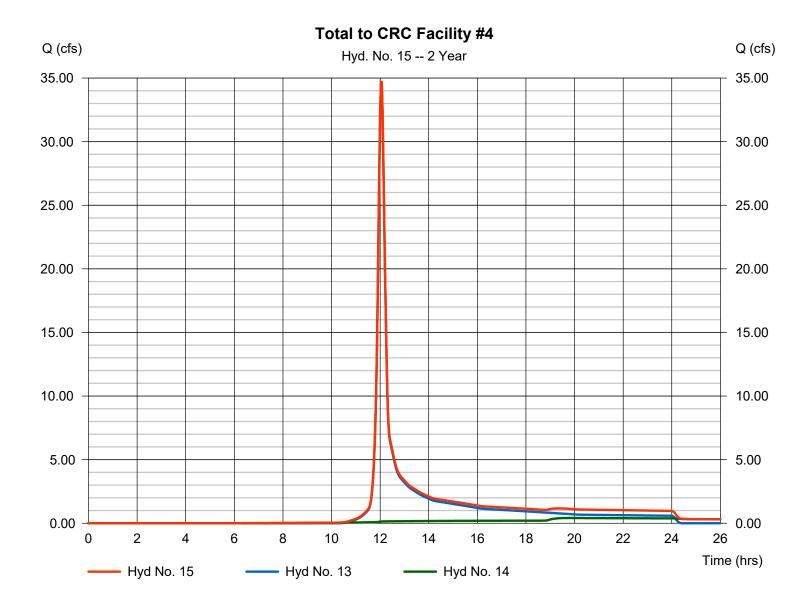
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Hyd. No. 15

Total to CRC Facility #4

Hydrograph type Peak discharge = Combine = 34.70 cfsTime to peak Storm frequency = 2 yrs $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 152,921 cuft Inflow hyds. = 13, 14 Contrib. drain. area = 27.720 ac



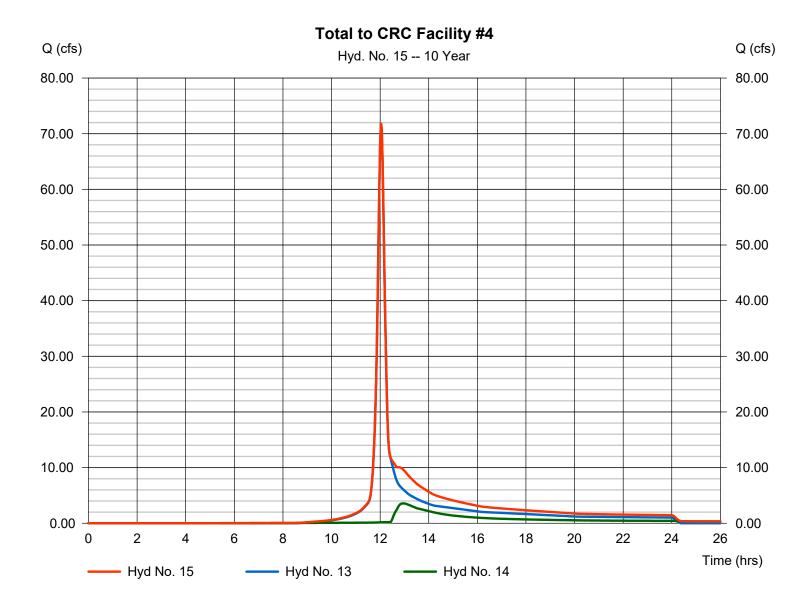
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Wednesday, 01 / 31 / 2018

Hyd. No. 15

Total to CRC Facility #4

Hydrograph type Peak discharge = 71.94 cfs= Combine Time to peak Storm frequency = 10 yrs $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 288,389 cuft Inflow hyds. = 13, 14 Contrib. drain. area = 27.720 ac



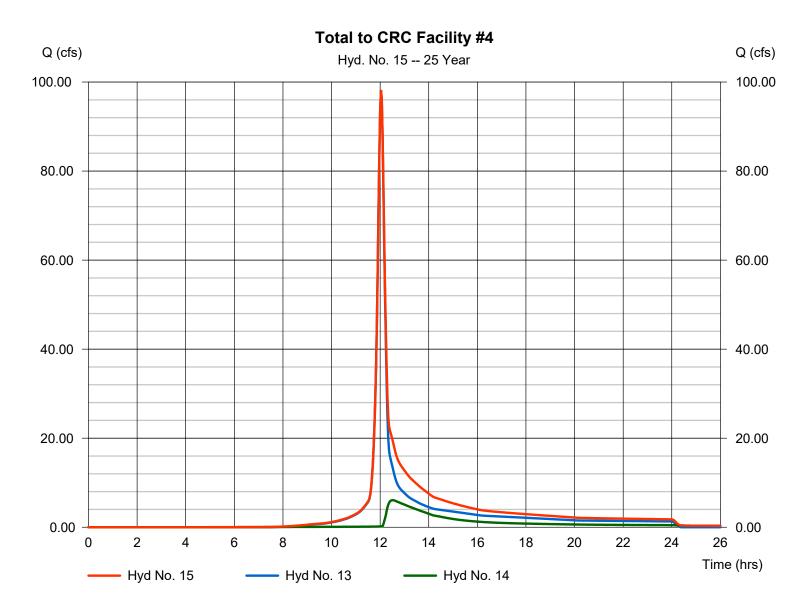
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Wednesday, 01 / 31 / 2018

Hyd. No. 15

Total to CRC Facility #4

Hydrograph type = Combine Peak discharge = 98.23 cfsStorm frequency = 25 yrsTime to peak $= 12.03 \, hrs$ Time interval = 2 min Hyd. volume = 384,481 cuft = 13, 14 Contrib. drain. area = 27.720 acInflow hyds.



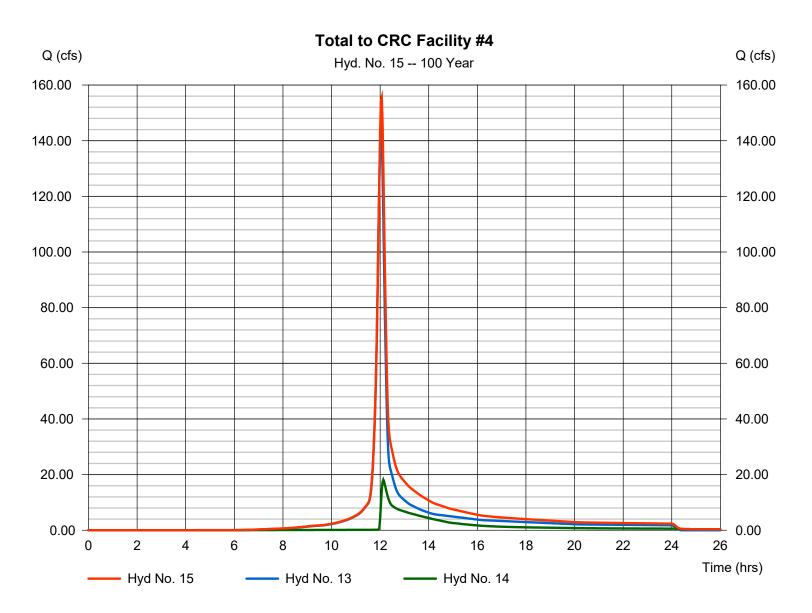
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Wednesday, 01 / 31 / 2018

Hyd. No. 15

Total to CRC Facility #4

Hydrograph type = Combine Peak discharge = 155.52 cfsTime to peak Storm frequency = 100 yrs $= 12.07 \, hrs$ Time interval = 2 min Hyd. volume = 557,913 cuft Inflow hyds. = 13, 14 Contrib. drain. area = 27.720 ac



Pond Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2018 by Autodesk, Inc. v12

Wednesday, 01 / 31 / 2018

Pond No. 1 - Facility #4 (2015)

Pond Data

Pond storage is based on user-defined values.

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (cuft)	Total storage (cuft)
0.00	2101.90	n/a	0	0
2.10	2104.00	n/a	1,873	1,873
4.10	2106.00	n/a	8,276	10,149
6.10	2108.00	n/a	18,339	28,488
8.10	2110.00	n/a	32,060	60,548
10.10	2112.00	n/a	49,963	110,511
12.10	2114.00	n/a	65,253	175,764

Culvert / Orifice Structures

Weir Structures

	[A]	[B]	[C]	[PrfRsr]		[A]	[B]	[C]	[D]
Rise (in)	= 36.00	3.00	0.00	0.00	Crest Len (ft)	= 9.00	0.00	0.00	0.00
Span (in)	= 36.00	3.00	0.00	0.00	Crest El. (ft)	= 2109.70	0.00	0.00	0.00
No. Barrels	= 1	1	0	0	Weir Coeff.	= 3.33	3.33	3.33	3.33
Invert El. (ft)	= 2101.77	2101.90	0.00	0.00	Weir Type	= 1			
Length (ft)	= 150.69	0.10	0.00	0.00	Multi-Stage	= Yes	No	No	No
Slope (%)	= 3.90	0.10	0.00	n/a					
N-Value	= .013	.013	.013	n/a					
Orifice Coeff.	= 0.60	0.60	0.60	0.60	Exfil.(in/hr)	= 0.000 (by)	Wet area)	1	
Multi-Stage	= n/a	Yes	No	No	TW Elev. (ft)	= 0.00			

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage cuft	Elevation ft	Clv A cfs	CIv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0	2101.90	0.00	0.00			0.00						0.000
0.21	187	2102.11	0.14 ic	0.07 ic			0.00						0.069
0.42	375	2102.32	0.14 ic	0.13 ic			0.00						0.128
0.63	562	2102.53	0.18 ic	0.17 ic			0.00						0.168
0.84	749	2102.74	0.20 ic	0.20 ic			0.00						0.200
1.05	937	2102.95	0.23 ic	0.23 ic			0.00						0.227
1.26	1,124	2103.16	0.26 ic	0.25 ic			0.00						0.252
1.47	1,311	2103.37	0.29 ic	0.27 ic			0.00						0.274
1.68	1,498	2103.58	0.29 ic	0.29 ic			0.00						0.295
1.89	1,686	2103.79	0.33 ic	0.31 ic			0.00						0.314
2.10	1,873	2104.00	0.33 ic	0.33 ic			0.00						0.332
2.30	2,701	2104.20	0.37 ic	0.35 ic			0.00						0.349
2.50	3,528	2104.40	0.37 ic	0.36 ic			0.00						0.364
2.70	4,356	2104.60	0.41 ic	0.38 ic			0.00						0.379
2.90	5,183	2104.80	0.41 ic	0.39 ic			0.00						0.394
3.10	6,011	2105.00	0.41 ic	0.41 ic			0.00						0.408
3.30	6,839	2105.20	0.45 ic	0.42 ic			0.00						0.421
3.50	7,666	2105.40	0.45 ic	0.43 ic			0.00						0.434
3.70	8,494	2105.60	0.45 ic	0.45 ic			0.00						0.447
3.90	9,321	2105.80	0.46 ic	0.46 ic			0.00						0.459
4.10	10,149	2106.00	0.50 ic	0.47 ic			0.00						0.471
4.30	11,983	2106.20	0.50 ic	0.48 ic			0.00						0.483
4.50	13,817	2106.40	0.50 ic	0.49 ic			0.00						0.494
4.70	15,651	2106.60	0.51 ic	0.51 ic			0.00						0.505
4.90	17,485	2106.80	0.55 ic	0.52 ic			0.00						0.516
5.10	19,319	2107.00	0.55 ic	0.53 ic			0.00						0.527
5.30	21,152	2107.20	0.55 ic	0.54 ic			0.00						0.537
5.50	22,986	2107.40	0.55 ic	0.55 ic			0.00						0.548
5.70	24,820	2107.60	0.56 ic	0.56 ic			0.00						0.558
5.90	26,654	2107.80	0.57 ic	0.57 ic			0.00						0.567
6.10	28,488	2108.00	0.61 ic	0.58 ic			0.00						0.577
6.30	31,694	2108.20	0.61 ic	0.59 ic			0.00						0.586
6.50	34,900	2108.40	0.61 ic	0.60 ic			0.00						0.596
6.70	38,106	2108.60	0.61 ic	0.61 ic			0.00						0.605
6.90	41,312	2108.80	0.61 ic	0.61 ic			0.00						0.614
7.10	44,518	2109.00	0.62 ic	0.62 ic			0.00						0.623
7.30	47,724	2109.20	0.67 ic	0.63 ic			0.00						0.632
											Continue	oc on nov	t nago

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Facility #4 (2015) Stage / Storage / Discharge Table

•	•	•											
Stage ft	Storage cuft	Elevation ft	CIv A cfs	CIv B cfs	CIv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
7.50	50,930	2109.40	0.67 ic	0.64 ic			0.00						0.640
7.70	54,136	2109.60	0.67 ic	0.65 ic			0.00						0.649
7.90	57,342	2109.80	1.64 ic	0.65 ic			0.95						1.600
8.10	60,548	2110.00	5.74 ic	0.64 ic			4.93						5.566
8.30	65,544	2110.20	11.49 ic	0.63 ic			10.60						11.23
8.50	70,541	2110.40	18.43 ic	0.62 ic			17.55						18.17
8.70	75,537	2110.60	26.28 ic	0.61 ic			25.60						26.21
8.90	80,533	2110.80	35.20 ic	0.60 ic			34.58						35.18
9.10	85,529	2111.00	34.93 ic	0.61 ic			31.04 ic						31.65
9.30	90,526	2111.20	34.66 ic	0.62 ic			33.34 ic						33.96
9.50	95,522	2111.40	36.12 ic	0.63 ic			35.49 ic						36.12
9.70	100,518	2111.60	38.15 ic	0.63 ic			37.52 ic						38.15
9.90	105,515	2111.80	40.08 ic	0.63 ic			39.45 ic						40.08
10.10	110,511	2112.00	41.92 ic	0.63 ic			41.28 ic						41.92
10.30	117,036	2112.20	43.68 ic	0.64 ic			43.04 ic						43.68
10.50	123,562	2112.40	45.37 ic	0.64 ic			44.73 ic						45.37
10.70	130,087	2112.60	47.00 ic	0.64 ic			46.35 ic						47.00
10.90	136,612	2112.80	48.57 ic	0.65 ic			47.93 ic						48.57
11.10	143,138	2113.00	50.10 ic	0.65 ic			49.45 ic						50.10
11.30	149,663	2113.20	51.58 ic	0.65 ic			50.92 ic						51.58
11.50	156,188	2113.40	53.01 ic	0.66 ic			52.36 ic						53.01
11.70	162,713	2113.60	54.42 ic	0.66 ic			53.76 ic						54.41
11.90	169,239	2113.80	55.78 ic	0.66 ic			55.12 ic						55.78
12.10	175,764	2114.00	57.11 ic	0.66 ic			56.44 ic						57.11

...End

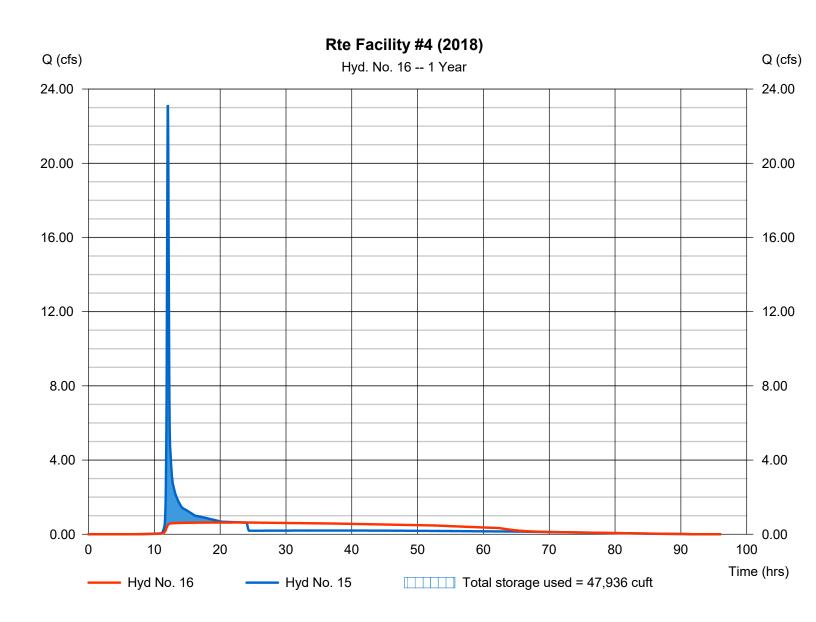
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Wednesday, 01 / 31 / 2018

Hyd. No. 16

Rte Facility #4 (2018)

Hydrograph type = Reservoir Peak discharge = 0.632 cfsStorm frequency Time to peak $= 23.53 \, hrs$ = 1 yrsTime interval = 2 min Hyd. volume = 109,614 cuftMax. Elevation Inflow hyd. No. = 15 - Total to CRC Facility #4 = 2109.21 ft= Facility #4 (2015) Reservoir name Max. Storage = 47,936 cuft



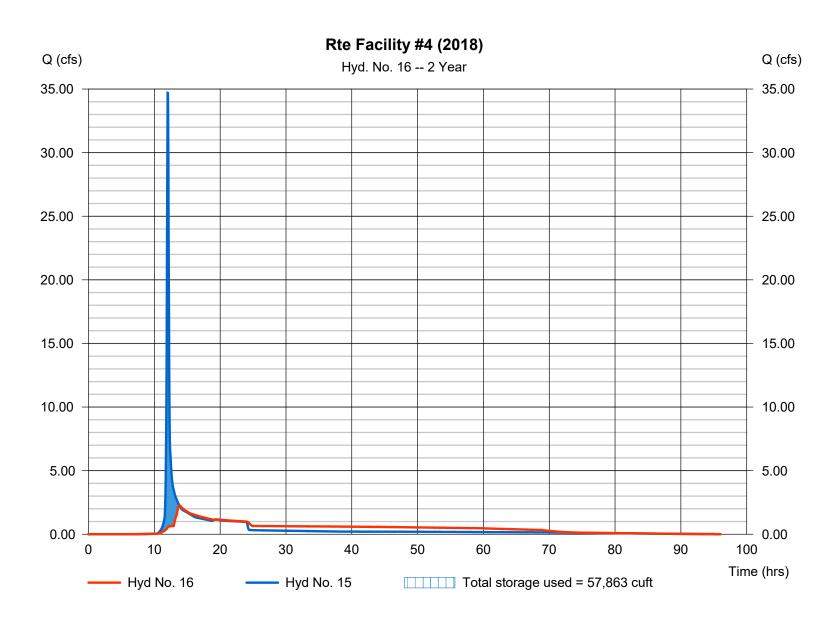
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Wednesday, 01 / 31 / 2018

Hyd. No. 16

Rte Facility #4 (2018)

Hydrograph type = Reservoir Peak discharge = 2.244 cfsStorm frequency = 2 yrsTime to peak $= 13.83 \, hrs$ Time interval = 2 min Hyd. volume = 152,912 cuft Max. Elevation Inflow hyd. No. = 15 - Total to CRC Facility #4 = 2109.83 ft= Facility #4 (2015) Reservoir name Max. Storage = 57,863 cuft



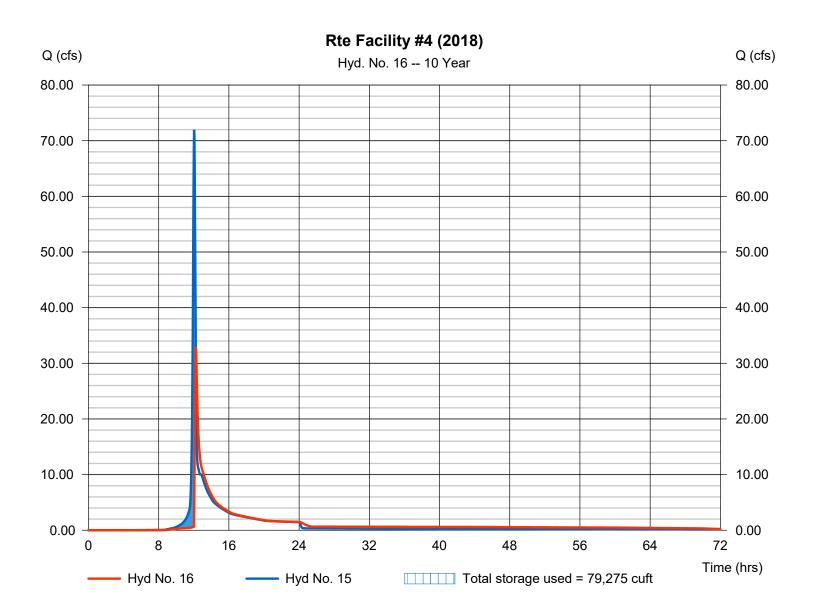
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Wednesday, 01 / 31 / 2018

Hyd. No. 16

Rte Facility #4 (2018)

Hydrograph type = Reservoir Peak discharge = 32.92 cfsStorm frequency = 10 yrsTime to peak $= 12.23 \, hrs$ Time interval = 2 min Hyd. volume = 288,352 cuft Max. Elevation Inflow hyd. No. = 15 - Total to CRC Facility #4 = 2110.75 ft= Facility #4 (2015) = 79,275 cuft Reservoir name Max. Storage



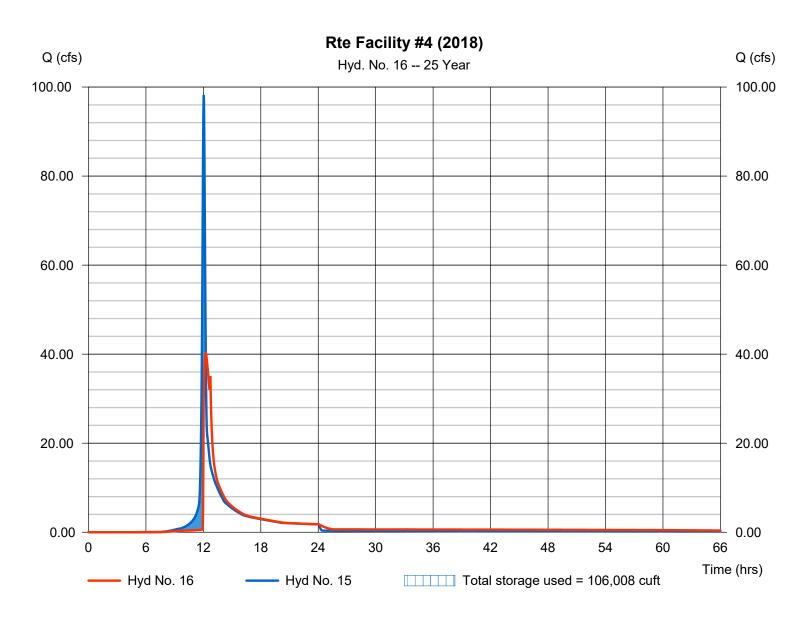
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Wednesday, 01 / 31 / 2018

Hyd. No. 16

Rte Facility #4 (2018)

Hydrograph type = Reservoir Peak discharge = 40.26 cfsStorm frequency = 25 yrsTime to peak = 12.27 hrsTime interval = 2 min Hyd. volume = 384,437 cuft Max. Elevation = 2111.83 ft Inflow hyd. No. = 15 - Total to CRC Facility #4 = Facility #4 (2015) Reservoir name Max. Storage = 106,008 cuft



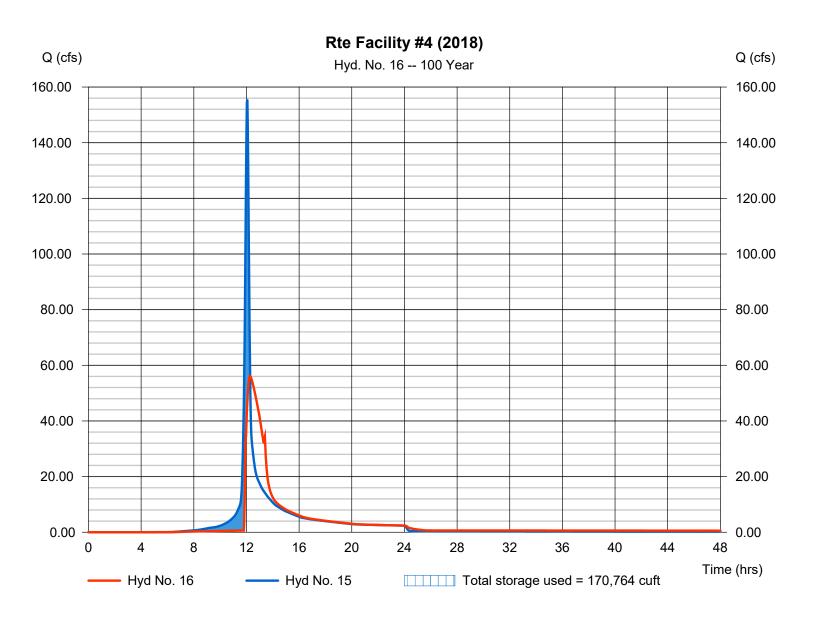
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Wednesday, 01 / 31 / 2018

Hyd. No. 16

Rte Facility #4 (2018)

Hydrograph type = Reservoir Peak discharge = 56.09 cfsStorm frequency Time to peak = 12.27 hrs= 100 yrsTime interval = 2 min Hyd. volume = 557,864 cuft Max. Elevation Inflow hyd. No. = 15 - Total to CRC Facility #4 = 2113.85 ft = Facility #4 (2015) Reservoir name Max. Storage = 170,764 cuft



APPENDIX G – VIRGINIA RUNOFF REDUCTION METHOD CALCULATIONS

(See attached Drawings)

DEQ Virginia Runoff Reduction Method New Development Compliance Spreadsheet - Version 3.0

☐ 2011 BMP Standards and Specifications ☐ 2013 ☐

2013 Draft BMP Standards and Specifications

Project Name: 30R @ CRC
Date: 8/10/2017

CLEAR ALL

data input cells
constant values
calculation cells
final results

BMP Design Specifications List: 2013 Draft Stds & Specs

Site Information

Post-Development Project (Treatment Volume and Loads)

Land Cover (acres)

	A Soils	B Soils	C Soils	D Soils	Totals
Forest/Open Space (acres) undisturbed,					0.00
protected forest/open space or reforested land					0.00
Managed Turf (acres) disturbed, graded for					4.00
yards or other turf to be mowed/managed		3.05	0.85	0.10	4.00
Impervious Cover (acres)		5.30	0.25		5.55
					9.55

Constants

Constants	
Annual Rainfall (inches)	43
Target Rainfall Event (inches)	1.00
Total Phosphorus (TP) EMC (mg/L)	0.26
Total Nitrogen (TN) EMC (mg/L)	1.86
Target TP Load (lb/acre/yr)	0.41
Pj (unitless correction factor)	0.90

Runoff Coefficients (Rv)

	A Soils	B Soils	C Soils	D Soils
Forest/Open Space	0.02	0.03	0.04	0.05
Managed Turf	0.15	0.20	0.22	0.25
Impervious Cover	0.95	0.95	0.95	0.95

Post-Development Requirement for Site Area

TP Load Reduction Required (lb/yr) 9.98

LAND COVER SUMMARY -- POST DEVELOPMENT

Land Cover Summary						
Forest/Open Space Cover (acres)	0.00					
Weighted Rv (forest)	0.00					
% Forest	0%					
Managed Turf Cover (acres)	4.00					
Weighted Rv (turf)	0.21					
% Managed Turf	42%					
Impervious Cover (acres)	5.55					
Rv (impervious)	0.95					
% Impervious	58%					
Site Area (acres)	9.55					
Site Rv	0.64					

Treatment Volume and Nutrient Loads					
Treatment Volume (acre-ft)	0.5079				
Treatment Volume (cubic feet)	22,123				
TP Load (lb/yr)	13.90				
TN Load (lb/yr) (Informational Purposes Only)	99.44				



P.O. Box 142 Penn Laird, VA 22846 · (540) 908-1679 · www.VirginiaNutrientBank.com

January 15, 2018

Simon L Rutrough, PE Parker Design Group, Inc. 2122 Carolina Avenue, SW Roanoke, VA 24014

Re: Nutrient Credit Availability – Olver Property - Blacksburg

Mr. Rutrough,

The Virginia Nutrient Bank (VNB) is pleased to preliminary reserve approximately <u>10±</u> pounds per year of phosphorus offsets (nutrient credits) for the Olver Property in Blacksburg. The project is located in the Upper New River Basin.

VNB has approval from the Virginia Department of Environmental Quality (VDEQ) for Nonpoint Source Offset Generation Certification. VNB is approved to transfer nutrient credits in accordance with the Chesapeake Bay Watershed Nutrient Credit Exchange Program (VA Code 62.1-44.19:14 et seq). These offsets are also transferable in accordance with the Virginia stormwater offset program (VA Code 62.1-44.15:35) and the Virginia Soil and Water Conservation Board's Guidance Document on Stormwater Nonpoint Nutrient Offsets approved on July 23, 2009, to those regulator entities qualifying for nutrient offsets.

VNB manages the Hiwassee Nutrient Bank located near Hiwassee in Pulaski County that will generate roughly 68 pounds of phosphorus reduction and roughly 318 pounds of nitrogen reduction per. Upon final approval and closing, VNB will retire $\underline{10+}$ pounds of phosphorus credits in accordance with the Nutrient Offset Certification regulations.

Respectfully, Virginia Nutrient Bank, LLC

Nathan W. Blackwell, PE 540-908-1679

Owner - Conservation Director